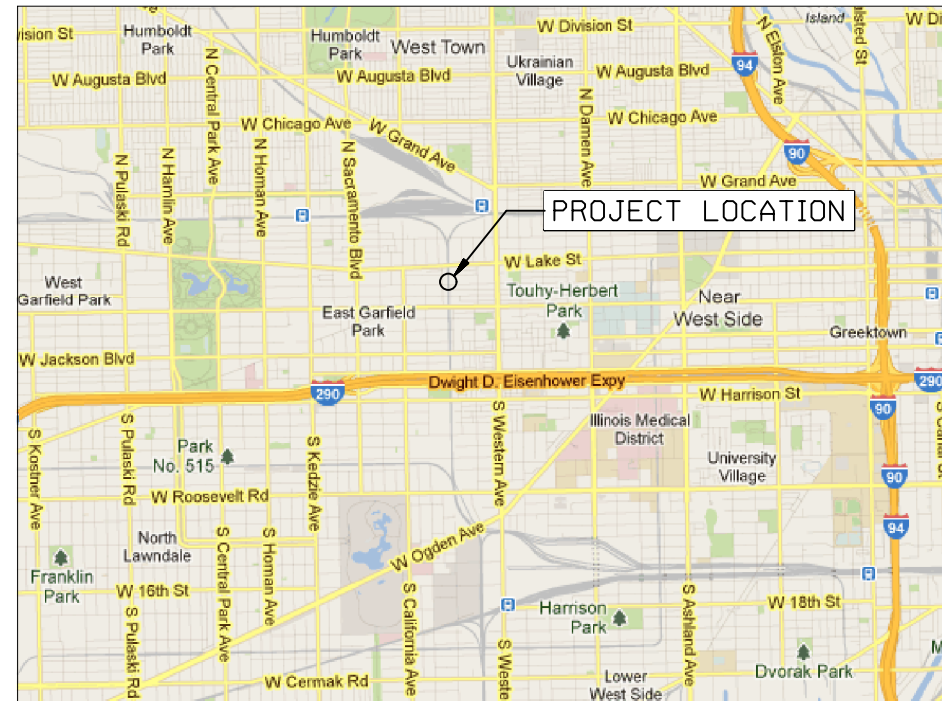
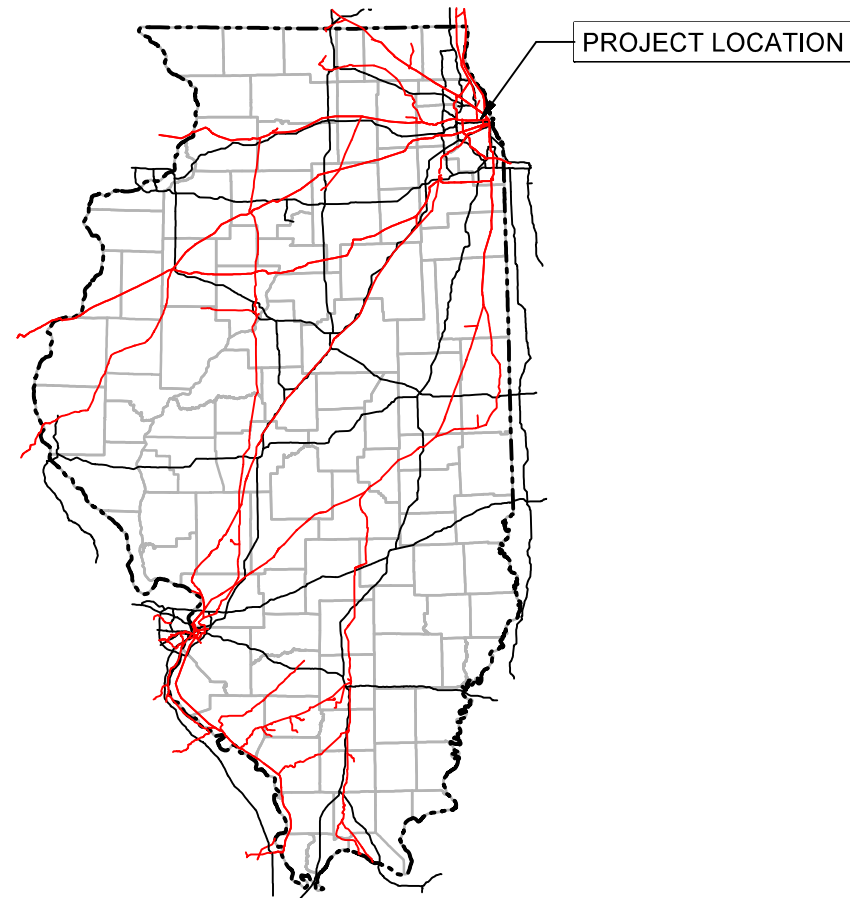




ENGINEERING DESIGN & CONSTRUCTION



2620 WEST WASHINGTON BLVD
CHICAGO, IL

Project Location Map

MP 0.93 ROCKWELL SUB. UNION PACIFIC RAILROAD WASHINGTON BLVD BRIDGE REPLACEMENT

FINAL PLANS

CREATE PROJECT #:
WA1-UP-XXB-003-B-FE

WORK ORDER: 31876
PROJECT NUMBER: N/A
BUDGET REFERENCE: N/A

LAST REVISED
May 28, 2021

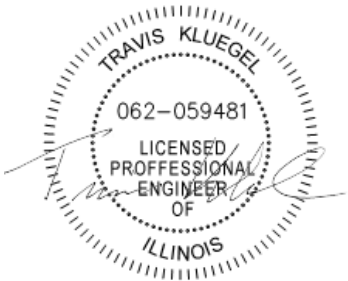
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5/26/2021

PROJECT INDEX

PROJECT DESIGN	DESCRIPTION
G-001	COVER SHEET WITH VICINITY MAP
G-002	PROJECT INDEX & REVISION SHEET
G-003	GENERAL NOTES & PROJECT CONTACTS
G-004	ABBREVIATIONS & LEGEND
R-001	ROADWAY EXISTING CONDITIONS AND REMOVAL PLAN
R-002	ROADWAY RESTORATION PLAN
R-003	TRAFFIC CONTROL GENERAL NOTES
R-004	MAINTENANCE OF TRAFFIC - WESTBOUND NORTH CONSTRUCTION W. WASHINGTON BLVD.
R-005	MAINTENANCE OF TRAFFIC - WESTBOUND SOUTH CONSTRUCTION W. WASHINGTON BLVD.
R-006	FULL CLOSURE DETOUR - W. WASHINGTON BLVD.
R-007	IDOT HIGHWAY STANDARD
R-008	IDOT HIGHWAY STANDARD
R-009	IDOT HIGHWAY STANDARD
R-010	IDOT HIGHWAY STANDARD
R-011	IDOT HIGHWAY STANDARD
R-012	IDOT HIGHWAY STANDARD
R-013	IDOT HIGHWAY STANDARD
R-014	IDOT HIGHWAY STANDARD
R-015	IDOT HIGHWAY STANDARD
L-001	ELECTRICAL GENERAL NOTES
L-002	PROPOSED UNDERDECK LIGHTING
L-003	LIGHTING DETAILS
L-004	LIGHTING DETAILS
L-005	LIGHTING DETAILS
E1 TO E43	BRIDGE 0.93 STRUCTURAL PLANS
UPRR 531100 Sht. T2	UPPR H-PILE FOUNDATION DETAILS
UPRR 531100 Sht. T3	UPPR GENERAL NOTES
UPRR 531110 Sht. H1	UPPR PIECE FABRICATION NOTES

BILL OF MATERIAL TABLE INDEX

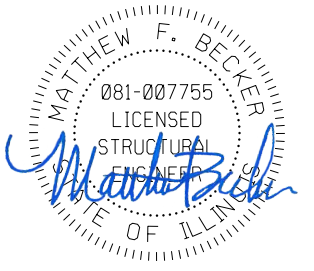
BILL OF MATERIAL	LOCATION
REMOVAL	R-001
RESTORATION	R-002
TRAFFIC CONTROL	R-003
LIGHTING	L-002
STRUCTURES	E4



EXPIRATION DATE: 11-30-2021
DATE: 05-28-2021
(CIVIL SHEETS)



EXPIRATION DATE: 11-30-2021
DATE: 05-28-2021
(ELECTRICAL SHEETS)



EXPIRATION DATE: 11-30-2022
DATE: 05-28-2021
(STRUCTURAL SHEETS)

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CONSTRUCTION

REVISION	BY	DATE	DESCRIPTION



Alfred Benesch & Company
35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



DRAWN BY: KP	WORK ORDER: 31876
CHECKED BY: TK	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: N.T.S	SHEET NUMBER G-002



Director Structures Design

LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE:
PROJECT INDEX & REVISION SHEET

GENERAL NOTES

1. UPRR forces will perform all track work, with the exception of removal of existing tracks. UPRR will cut the existing rails and the contractor shall remove and dispose of all track material with the bridge removal work. The Contractor will be responsible for constructing proposed track bed up to the top of the sub-ballast layer. UPRR will furnish & install all ballast, ties, rail, and other track materials.
2. Existing and new track within the project limits will be surfaced and lined by UPRR forces once all other work is complete.
3. Contractors shall notify Service Alert, (800) 642-2444, UPRR Fiber Optics Hotline (800) 336-9193, and the Chicago Utility Alert Network (312) 744-7000 48 hours prior to any excavation. The USA Authorization Numbers shall be kept at the job site.
4. No work whatsoever shall be commenced without first notifying the UPRR Engineer.
5. The Contractor shall comply with all Federal, State, County, and City Laws and Ordinances and Regulations of the Department of Industrial Relations, OSHA, NPDES and Industrial Accident Commission related to the safety and character of the work, equipment and labor personnel.
6. Contractor shall be responsible for coordinating with all Utility agencies.
7. Contractor shall protect in place (by any means necessary) all existing utilities to remain unless otherwise specified herein, contractor shall be responsible for the complete repair at his expense, for any damage to existing utilities, structures, or other site features, as a result of his work.
8. Prior to placing curbs, pavements, base, subbase, track, etc., all underground utilities shall be installed, backfill completed, and the Engineer notified by each of the utility companies having facilities within the work area, that the utility installation has satisfactorily passed acceptance tests.
9. All existing underground utilities within the UPRR ROW, that are not to be re-used shall be abandoned in place. All existing pipelines to be abandoned in place shall be cement slurry filled and capped at least 3'-0" below top of proposed subgrade.
10. Contractor shall verify locations and elevations of existing utilities whether known or unknown prior to beginning construction.
11. Any underground structures such as cesspools, cisterns, mining shafts, tunnels, septic tanks, wells, and pipelines not located prior to construction shall be brought to the attention of the engineer for determination of appropriate action such as removal or treatment in a manner judged suitable to the engineer.
12. Contractor shall coordinate location of all proposed utilities with UPRR to assure accuracy of utility connections and compliance with local codes.
13. Any existing conditions found to be a variance with these drawings must be immediately reported to the Engineer.
14. Contractor shall maintain and clean to the satisfaction of the Engineer, all access and service roads used during construction.
15. Contractor shall perform all construction in such a manner as to protect adjacent existing buildings, and other site elements which are to remain in service.
16. Contractor shall provide As-built Drawings for all improvements.
17. No field changes will be permitted without direct written authorization from the UPRR Engineer or his representative.
18. Contractor shall coordinate work which affects adjacent property owners. Any questions or agreements between adjacent property owners and contractor shall be made in writing. A copy of such agreement shall be provided to the UPRR Engineer or his representative.
19. The contractor is responsible for preparing a Stormwater Pollution Prevention Plan (SWPPP) to comply with State regulations.
20. Right-of-way lines shown on the plans were taken from existing UPRR right-of-way map and are approximate.
21. Match lines for sheets are based on the existing Main Line stationing unless otherwise specified.
22. Track laying, ballasting, and installation of road crossing panels will be done by UPRR unless otherwise stated.
23. The contractor is responsible for the removal of all pavement markings that will be in conflict with the proposed work.
24. Contractor shall comply with all IDOT specifications for construction of public improvements requirements.
25. Contractor shall maintain at least one access to all affected business. If necessary, multiphase construction shall be utilized.
26. All work must be coordinated with the UPRR to minimize track outage time and disruption of train service. The contractor shall submit for approval his proposed sequence of operations prior to the start of construction.
27. Removal of existing viaduct lighting equipment will be performed by the City of Chicago, Department of Streets and Sanitation, Bureau of Electric. The contractor must schedule and coordinate this work with the city. All expenses or charges by the city related to this work will be incidental to this contract.
28. The contractor shall take special care to avoid damage to the existing utilities under the public streets from excessive surface loads during construction activities. Video inspection of the existing sewer shall be performed before and after construction in accordance with the requirements of the City of Chicago Department of Water Management. Any damage to the existing utilities caused by work under this contract must be repaired or replaced at the contractor's expense.
29. All frames and lids removed from abandoned sewers and appurtenances must be returned to the Chicago Department of Water Management, Sewer Section.
30. In case of damage to the City of Chicago sewers, private and public drains, sewer structures and/or bench monuments, the contractor shall immediately contact the Department of Water Management at (312) 747-7892 or (312) 747-7893.
31. Stockpiling of removed materials and/or construction debris on the job site will not be permitted and shall be removed from the job site each and every day and disposed of in accordance with article 202.03 of the Standard Specifications. Failure to comply with this requirement shall be considered a traffic control deficiency and will be subject to charges in accordance with the item Traffic Control Complete.

DESIGN CRITERIA

1. UPRR Standard Plans and Specifications
2. Illinois Department of Transportation (IDOT) Standard Specifications for Road and Bridge Construction
3. Chicago Department of Transportation (CDOT) Regulations for Openings, Construction and Repair in the Public Way

	DATUM
HORIZONTAL	Illinois East State Plane (1201) North American Datum of 1983 (NAD83)
VERTICAL	Chicago City Datum (CCD)

SURVEY NOTES

1. Railroad stationing for project profiles and alignments is based on stations established for chord definition spiraled curves at the centerline of the existing UPRR Main Line unless otherwise noted.
2. The contractor is responsible for the preservation of all survey control monuments. In the event monuments are damaged or destroyed by the contractor, the Engineer will replace the monument solely at the contractor's expense.

TRAFFIC NOTES

1. All barricades, warning signs, lights, devices, etc. for the guidance of vehicle traffic and pedestrians must conform to the Manual on Uniform Traffic Control Devices (MUTCD), current edition, IDOT and CDOT standards.
2. The contractor will ensure that all barricades, signs, lights, and other devices installed by him are operational every day, including Sundays and holidays. The contractor shall make twice daily inspections of barricades, signs, lights and other devices installed by him to ensure proper placement and functioning of warning devices. In the event of severe weather conditions, the contractor must furnish any additional personnel required to properly maintain all traffic control devices. The contractor shall provide a manned 24-hour / 7 day a week contact number to respond to requests/urgencies and emergencies related to the placement and maintenance of the traffic control devices throughout the project duration.
3. The contractor is responsible for the prompt replacement and/or repair of all traffic control devices and appurtenances damaged or disturbed due to construction.

PROJECT CONTACTS

<u>CONTACT</u>	<u>PHONE NUMBER</u>	<u>UPRR</u>
Curt Nystron	(515) 298-1131	Construction Field Manager
Adam Studts	(402) 544-3541	Structures Design Sr. Manager
Paul Pino	(402) 544-3582	Information Technology - Fiber
Stan Dulinski	(402) 544-0353	Real Estate - Utilities

PHONE NUMBER

(800) 336-9193
(888) 258-0808
(888) 877-7267

GENERAL

CALL UPRR BEFORE YOU DIG
CALL BEFORE YOU DIG (NATIONAL DIRECTORY)
UPRR Response Management Communications Center (RMCC)

BENCHMARKS

City of Chicago Benchmark: BM #276
6.4 feet north of south line of W Congress Parkway and 47.1 feet east of east
line of S Washtenaw Avenue. Elev. = 14.66 (CCD)

Adams Benchmark: BM "ADAMS"
Cut square near SW corner of concrete raised beam seat on south abutment
of Adams Street Overpass at track level (at SW corner). Elev. = 26.979 (CCD)

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CONSTRUCTION

[illegible]

DRAWN BY: KP	WORK ORDER: 31876
CHECKED BY: TK	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: N.T.S	SHEET NUMBER G-003

UNION PACIFIC RAILROAD		Director Structures Design
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT		
SHEET TITLE:		GENERAL NOTES & PROJECT CONTACTS

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5/26/2021

ABBREVIATIONS

MISCELLANEOUS

Ac.	Acres
Ave.	Avenue
Blvd.	Boulevard
Bldg.	Building
BNSF	BNSF Railway
C.Y.	Cubic Yards
Conc.	Concrete
Const.	Construct
°	Degree (s)
Dia.	Diameter
Dr.	Drive
Dwg.	Drawing
E	East
Elev.	Elevation
Exist.	Existing
'	Foot, Feet or Minute (s)
F.S.	Finished Surface
Horiz.	Horizontal
"	Inch, Inches or Second (s)
Inst.	Install
Inv.	Invert
Lt.	Left
L	Length
L.F.	Lineal Feet
Max.	Maximum
Min.	Minimum
N	North
NTS	Not to Scale
No.	Number
OH	Overhead
OHP	Overhead Power Line
PGL	Profile Grade Line
Prop.	Proposed
RR	Railroad
Rwy	Railway
R/W	Right of Way
Rt.	Right
S	South
S.F.	Square Feet
Sta.	Station
Std.	Standard
St.	Street
TT	Timetable
Twp.	Township
Typ.	Typical
UG	Underground
UPRR	Union Pacific Railroad
V	Velocity
Wt.	Weight
W	West
X-ing	Crossing

SIGNAL

ABS	Automatic Block Signal
ATC	Automatic Train Control
CTC	Centralized Traffic Control
DED	Dragging Equipment Detector
DTC	Direct Traffic Control
ELTO	Electric Lock Turnout
HBD	Hot Box Detector
HTTO	Hand Throw Turnout
HWD	High Wide Detector
POTO	Power Operated Turnout
TWC	Track Warrant Control
WILD	Wheel Impact Load Dectector

STRUCTURES

Bldg.	Building
Br.	Bridge
CB	Catch Basin
CPT	Concrete Pile Trestle - Ballast Deck
CIP	Cast Iron Pipe
CMP	Corrugated Metal Pipe
CMPA	Corrugated Metal Pipe Arch
CSP	Corrugated Steel Pipe
Culv.	Culvert
DI	Drop Inlet
DPGBD	Deck Plate Girder - Ballast Deck
DPGOD	Deck Plate Girder - Open Deck
EBW	East Backwall
F.L.	Flowline
F.F.	Finished Floor
GIP	Galvanized Iron Pipe
Hdwl	Headwall
NBW	North Backwall
PSCT	Prestressed Concrete Trestle
RCA	Reinforced Concrete Arch
RCB	Reinforced Concrete Box
RCP	Reinforced Concrete Pipe
SBW	South Backwall
SSP	Smooth Steel Pipe
SPTBD	Steel Pile Trestle - Ballast Deck
SPTOD	Steel Pile Trestle - Open Deck
SPP	Structural Plate Pipe
TPGBD	Through Plate Girder - Ballast Deck
TPGOD	Through Plate Girder - Open Deck
TPTBD	Timber Pile Trestle - Ballast Deck
TPTOD	Timber Pile Trestle - Open Deck
TTBD	Through Truss - Ballast Deck
TTOD	Through Truss - Open Deck
TWB	Treated Wood Box
VCP	Vitrified Clay Pipe
Viad.	Viaduct
WBW	West Backwall
WIP	Wrought Iron Pipe

TRACK

ATR	Above Top of Rail
Align.	Alignment
BBR	Below Base of Rail
Cntrs.	Centers
CWR	Continuous Welded Rail
DSPD	Double Switch Point Derail
EOT	End of Track
HH	Head Hardened
Jtd.	Jointed Rail
LH	Left Hand
ML	Main Line
MM	Mile Marker
MP	Mile Post
NSC	Not Sufficient Clearance
OTM	Other Track Material
PCC	Point of Compound Curve
PC	Point of Curve
PCS	Point of Curve to Spiral
POC	Point on Curve
PF	1/2" Point of Frog
PI	Point of Intersection
PITO	Point of Intersection of Turnout
PS	Point of Spiral
PSC	Point of Spiral to Curve
POS	Point on Spiral
PT	Point of Tangent
POT	Point on Tangent
Pt. Sw.	Point of Switch
PVC	Point of Vertical Curve
PVI	Point of Vertical Intersection
PVT	Point of Vertical Tangent
RH	Right Hand
SH	Second Hand
SSPD	Single Switch Point Derail
TC	Track Centers
T.F.	Track Feet
Trk.	Track
UXO	Universal Cross-Over
X-Over	Cross-Over

UTILITIES

AIR	AIR - Compressed Air
F/O	Fiber Optic Cable
G	Gas Pipeline
OHP	Overhead Power Line
SS	Sanitary Sewer
	Overhead Signal Line
UGS	Underground Signal Line
	Steam Line
S	Storm Sewer
T	Telephone
UGE	Underground Electric
W	Water Main
	Underground Wire
UD	Under Drain
	Water Valve
	Gas Buffalo Box
	Manhole
	Catch Basin
	Fire Hydrant
	Junction Box Electric
	Junction Box Telephone
	Junction Box Water
	Power Pole
	Generator

TRACK

	Existing Mainline
	Existing Siding or Spur
	Proposed
	Remove
	Shift
	Relay
	Future
	Foreign Railroad or Industry
	In Buildings or Under Structures
	Turnout
	Wheel Stop
	Bumping Post
	Earthen Bumper
	Inert Retarder
	Dowty Retarder
	Derail
	Switch Point Derail or Double Switch Point Derail

PROPERTY

	Section Line
	Center Section Line
	Parcel or Easement Line
	Right of Way
	Former Right of Way
	Right of Way to be Acquired
	Foreign Right of Way

SYMBOLS

ROAD CROSSING WARNING DEVICES

⊗	Crossbuck Sign
⊗	Flashing Light Warning Device
⊗	Flashing Light Warning Device with Gate
⊗	Cantilever Flashing Light Warning Device
⊗	Cantilever Flashing Light Signal with Gate

SIGNAL

	Absolute Signal
	Signal Bridge
	Cantilever Signal
	ACS or CTC Signal
	Dwarf Signal
	Begin CTC
	Microwave Tower
AEI	AEI
	Battery Box
	Dragging Equipment Detector
GEN	Generator
	Hot Box Detector
	Hot Air Blower
	Plastibeton

STRUCTURES

	Culvert
	Culvert with Headwalls
	Double Culvert
	Railroad Bridge
	Highway Overpass
	Highway Underpass
	Tunnel
	Retaining Wall
	Building
	Flag Pole

LIGHTING

	Light Pole
	Light Tower

SIGNS

	Sign
	Yard Limit
	1 Mile to Yard Limit
	Whistle Post
	Flanger
	Station
	Reduce Speed
	Resume Speed

FENCES

x x x	Barbed Wire
//	Chain Link
o o o o	Ornamental Fence
	Snow / Sand

ROADS

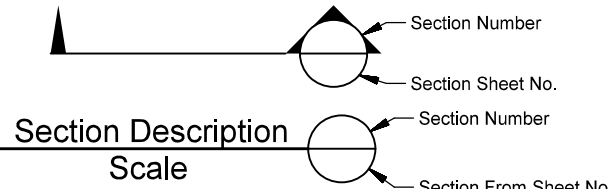
	Paved Road
	Unimproved Road
	Interstate Highway
	Federal Highway
	State Highway
	County Highway

OTHER

	Wetlands		Tree
	River or Lake		Bush
	Embankment		Stump
	Flow Line		Traffic Signal
	Milepost		E-T-01 Soil Boring
MM	Milemarker		CP-01 Control Point
	Revision Number		
	Revision Cloud		

CONSTRUCTION

Note	Proposed Note (Work by Contractor)	Note	Removal Note (Work by Contractor)
Note	Proposed Note (Work by Others)	Note	Removal Note (Work by Others)
	Cut Lines		
	Fill Lines	Note	Shift Note (Work by Contractor)
PGL	Profile Grade Line	Note	Shift Note (Work by Others)



 Alfred Benesch & Company 35 W. Wacker Drive Suite 3300 Chicago, Illinois 60601 312-565-0450 Job No. 210070.11		DRAWN BY: KP	WORK ORDER: 31876	UNION PACIFIC RAILROAD Director Structures Design
		CHECKED BY: TK	PID:	
		DATE: 05/28/21	BUDGET REF:	
		SCALE: N.T.S	SHEET NUMBER G-004	
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT				SHEET TITLE: ABBREVIATIONS & LEGEND

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5/26/2021

TRAFFIC CONTROL GENERAL NOTES:

1. All signing must be in accordance with the latest applicable provisions of the State of Illinois "Standard Specifications for Road and Bridge Construction", the details in these plans must be in accordance with the latest edition of the IDOT Bureau of Design and Environment highway standards and the latest edition of the "Manual on Uniform Traffic Control Devices", in effect on the date of invitation for bids.
2. Longitudinal dimensions shown on these plans may be adjusted to fit field conditions as directed by the commissioner.
3. The contractor must be responsible for ensuring that all barricades, signs, lights and other devices installed by him/her are in place and operating 24 hours each day including Sundays and holidays during the time this construction is in effect.
4. All existing signing that is not applicable while the construction is in effect must be completely covered by the contractor.
5. The sizes of all signs not specified in these plans must be as required by the Manual on Uniform Traffic Control Devices.
6. As a minimum, all amber flashing lights that are required must meet the requirements for Type A - low intensity flashing lights in article 702.04 of the standard specifications. All lights shall operate during hours of darkness. Only lights that have been approved by the Illinois Department of Transportation must be used.
7. The contractor must maintain access to all private and commercial driveways during construction.
8. Sidewalk access must be maintained on one side of the street during all stages, except when there is a full closure. Any closed sidewalks must be appropriately barricaded. Use Standard 701801.
9. All walkways must be clearly identified and adequately protected from motor vehicle traffic and free of any obstructions and hazards such as holes, debris, mud, construction equipment, stored materials, etc.
10. Proposed maintenance of traffic signing must be covered or removed when not required during a specific stage of construction.

11. Changeable message signs to be provided at locations shown on plans or determined by the commissioner.
12. The contractor must conduct his/her work in such a manner that emergency vehicles will have access to the work area at all times.
13. The contractor will be responsible for the proper location, installation, maintenance, relocation, and removal of all traffic control devices.
14. The contractor is responsible for recording the existing pavement marking patterns and limits prior to existing striping removal.
15. All existing pavement markings removed or impacted within the project limits must be replaced in-kind and to CDOT rules and regulations.
16. Unwanted temporary pavement markings must be removed by the contractor as ordered by the commissioner.
17. The contractor must notify CDOT 72 hours before commencing construction.
18. The contractor is responsible for all no parking notifications required by traffic shifts.

BILL OF MATERIAL				
REQD.	UNIT	DESCRIPTION	STORE ITEM NO.	ORDERED BY
* 868.6	FOOT	TEMPORARY PAVEMENT MARKING - LINE 4"		CONSTRUCTOR ↓
137.5	FOOT	TEMPORARY CONCRETE BARRIER		
137.5	FOOT	RELOCATE TEMPORARY CONCRETE BARRIER		
1	EACH	IMPACT ATTENUATORS, TEMPORARY (NON- REDIRECTIVE), TEST LEVEL 2		
1	EACH	IMPACT ATTENUATORS, RELOCATE (NON- REDIRECTIVE), TEST LEVEL 2		
4320	HOUR	ARROWBOARD (TRAILER MOUNTED)		
2	EACH	BARRICADES, TYPE III WITH WARNING LIGHT		
* 289.5	SQ FT	PAVEMENT MARKING REMOVAL - WATER BLASTING		
11	EACH	DRUM WITH WARNING LIGHT or BARRICADES, TYPE I WITH EXTENDED LEGS AND WARNING LIGHT		
1	L SUM	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)		
6	EACH	BARRICADES, TYPE III		

* Nominal Quantity to be used as needed and approved by the Commissioner.

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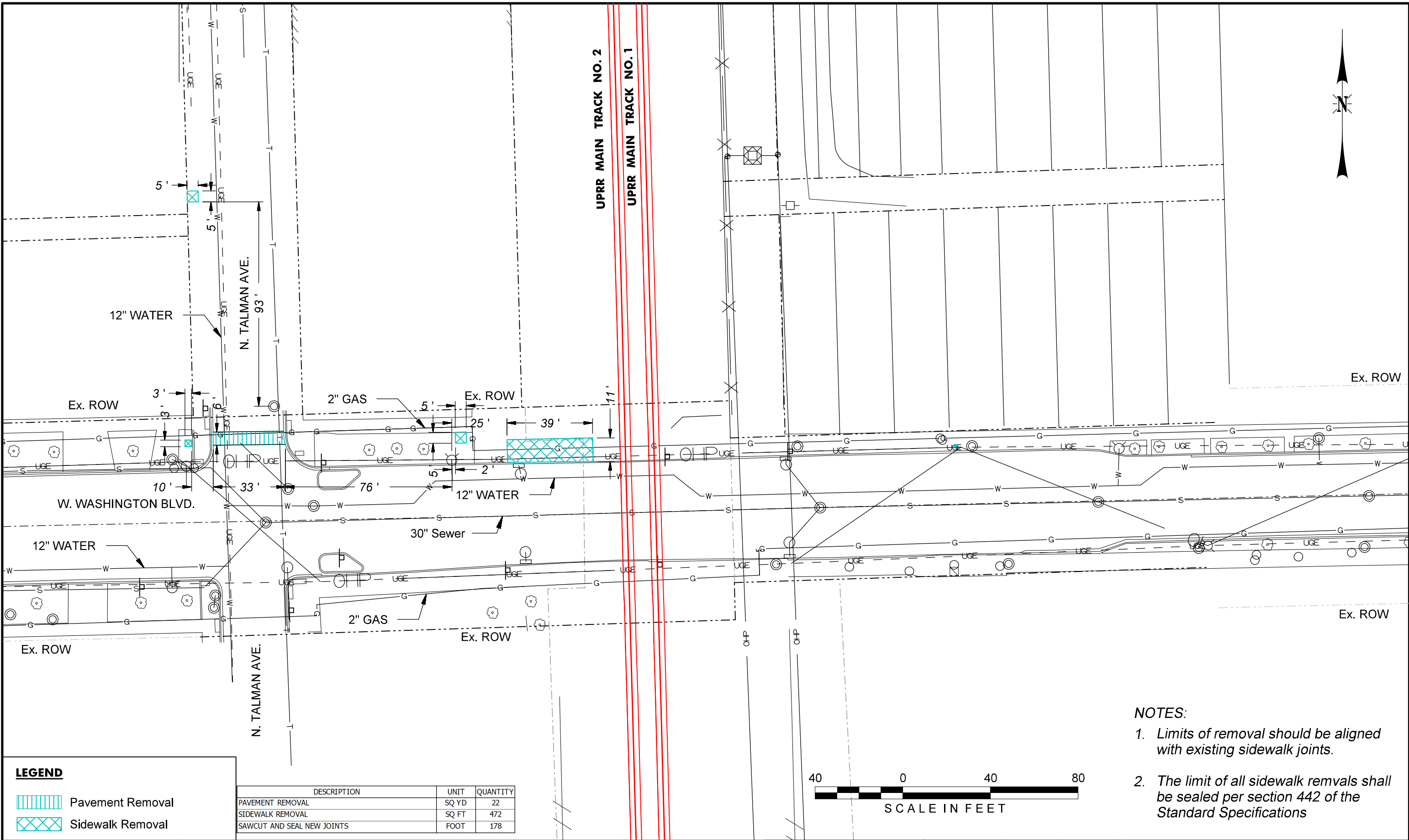
REVISION	BY	DATE	DESCRIPTION



DRAWN BY: KP	WORK ORDER: 31876
CHECKED BY: TK	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: N.T.S	SHEET NUMBER R-003

UNION PACIFIC RAILROAD	Director Structures Design
	LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT
	SHEET TITLE: TRAFFIC CONTROL GENERAL NOTES

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5/21/2021



LEGEND

Pavement Removal

Sidewalk Removal

DESCRIPTION	UNIT	QUANTITY
PAVEMENT REMOVAL	SQ YD	22
SIDEWALK REMOVAL	SQ FT	472
SAWCUT AND SEAL NEW JOINTS	FOOT	178



- NOTES:**
- Limits of removal should be aligned with existing sidewalk joints.
 - The limit of all sidewalk removals shall be sealed per section 442 of the Standard Specifications

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REVISION	BY	DATE	DESCRIPTION

Alfred Benesch & Company
35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11

DRAWN BY: KP	WORK ORDER: 31876
CHECKED BY: TK	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: 1" = 40'	SHEET NUMBER R-001

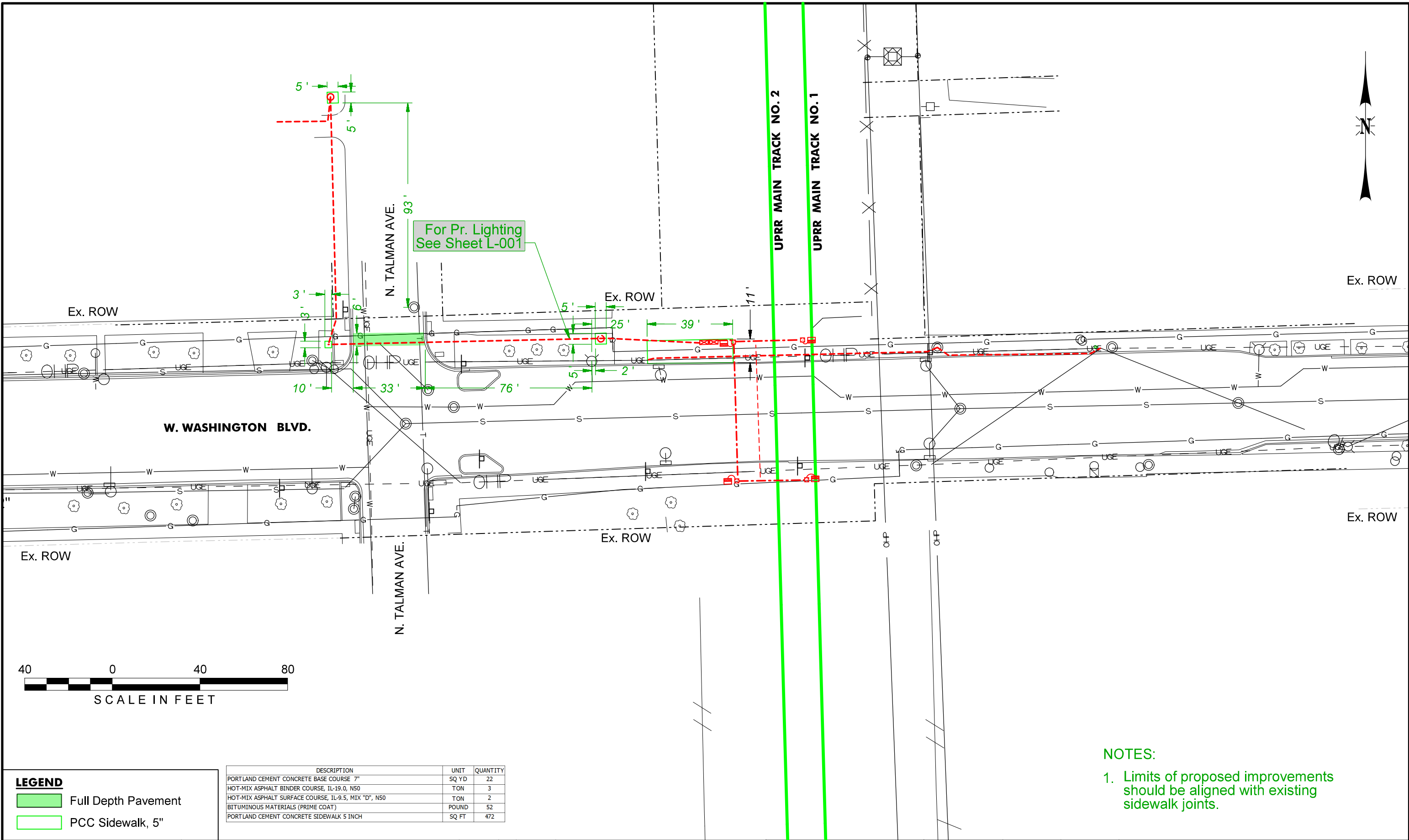
UNION PACIFIC RAILROAD

Director Structures Design

LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE:
ROADWAY EX. CONDITIONS & REMOVAL PLAN

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5/21/2021



LEGEND

- Full Depth Pavement
- PCC Sidewalk, 5"

DESCRIPTION	UNIT	QUANTITY
PORTLAND CEMENT CONCRETE BASE COURSE 7"	SQ YD	22
HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	3
HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50	TON	2
BITUMINOUS MATERIALS (PRIME COAT)	POUND	52
PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	472

- NOTES:**
- Limits of proposed improvements should be aligned with existing sidewalk joints.

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REVISION	BY	DATE	DESCRIPTION



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35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



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DATE: 05/28/21	BUDGET REF:
SCALE: 1" = 40'	SHEET NUMBER R-002

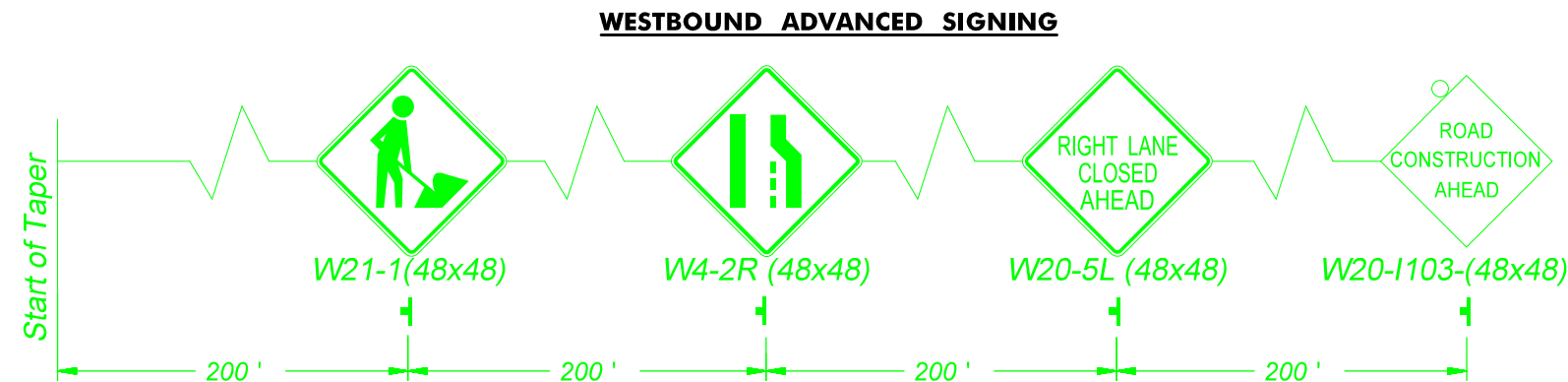
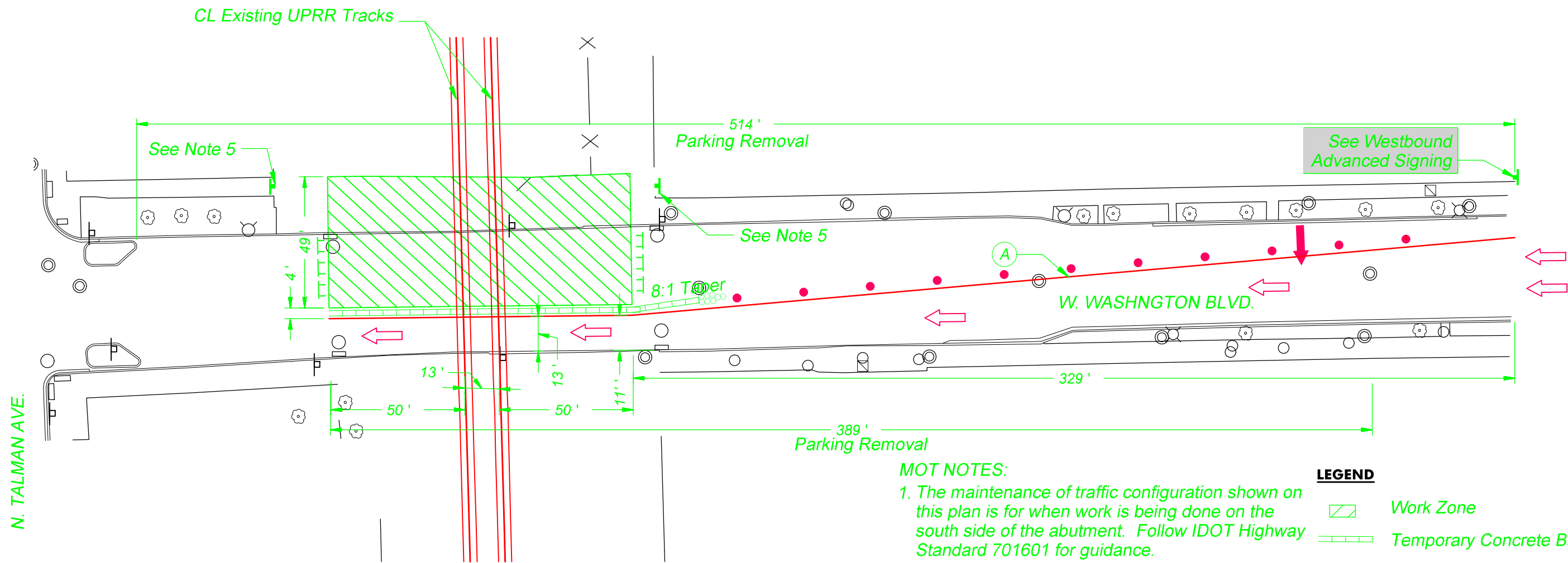


Director Structures Design

LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE:
ROADWAY RESTORATION PLAN

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5/26/2021



MOT NOTES:

1. The maintenance of traffic configuration shown on this plan is for when work is being done on the south side of the abutment. Follow IDOT Highway Standard 701601 for guidance.
2. Parking removal shall be as shown and as approved by the Commissioner.
3. The Contractor shall coordinate length of on-street parking removal with the Commissioner 20 days in advance of each stage of construction. To inform public, City personnel will install advance notice of no parking on street.
4. Contractor to maintain all crosswalks opened at the intersection of Talman Ave and Washington Blvd during construction.
5. Contractor shall close north sidewalk within the work zone limits. Contractor shall place local and advance signing to inform pedestrians of sidewalk closure. Use IDOT Highway Standard 801801 for proper signing.

LEGEND

- Work Zone
- Temporary Concrete Barrier
- Proposed Temporary Pavement Marking
- Drum Or Type I Barricade W/ Flashing Amber Light @ 20' C-C
- Type III Barricade
- Impact Attenuator Test Level 2
- Traffic Direction
- Arrow Board
- Temporary Traffic Sign
- Temporary Pavement Marking, 4" White Line



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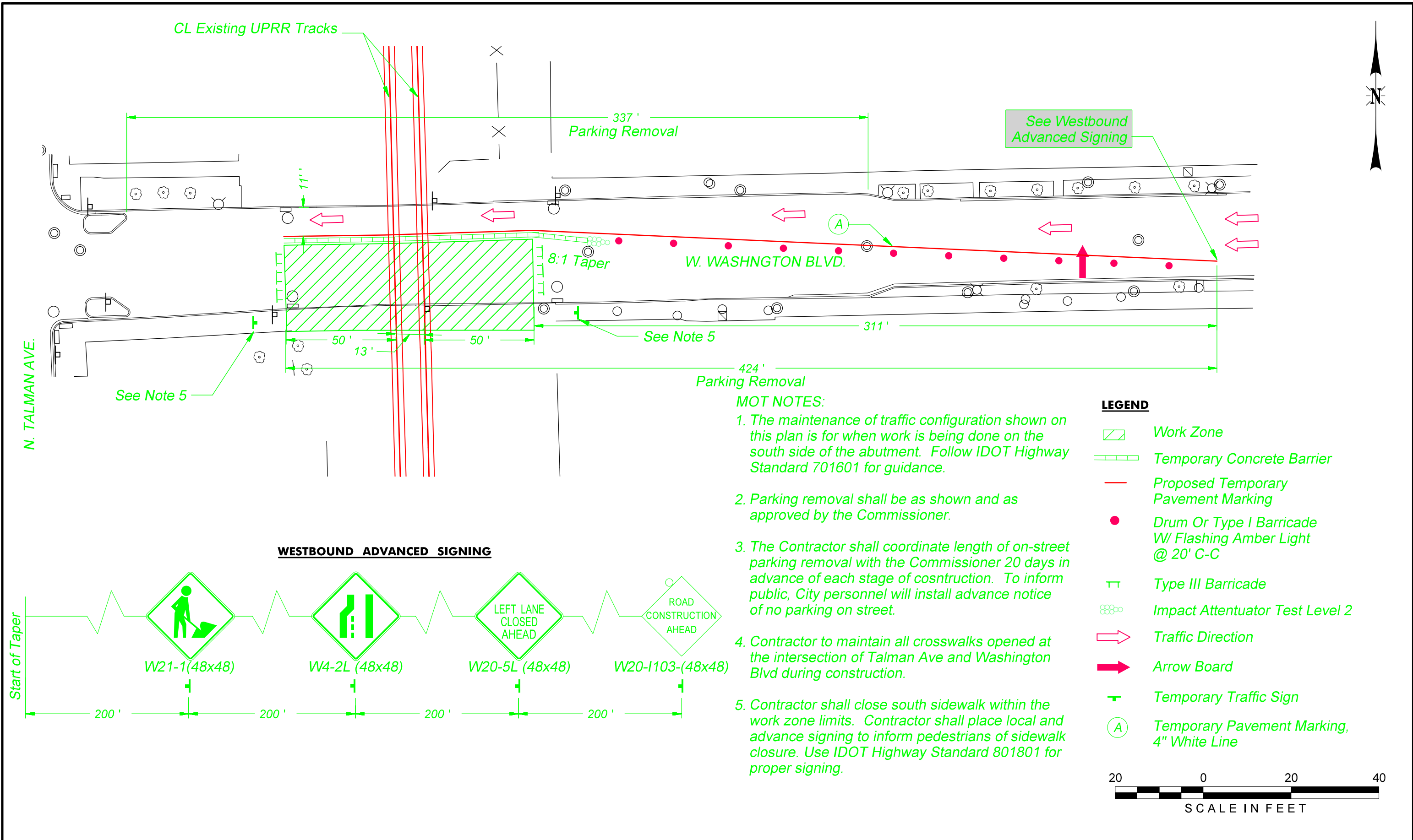
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SCALE: 1" = 20'	SHEET NUMBER R-004

UNION PACIFIC RAILROAD	Director Structures Design
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
SHEET TITLE: MOT - WB NORTH CONST. W. WASHINGTON BLVD.	

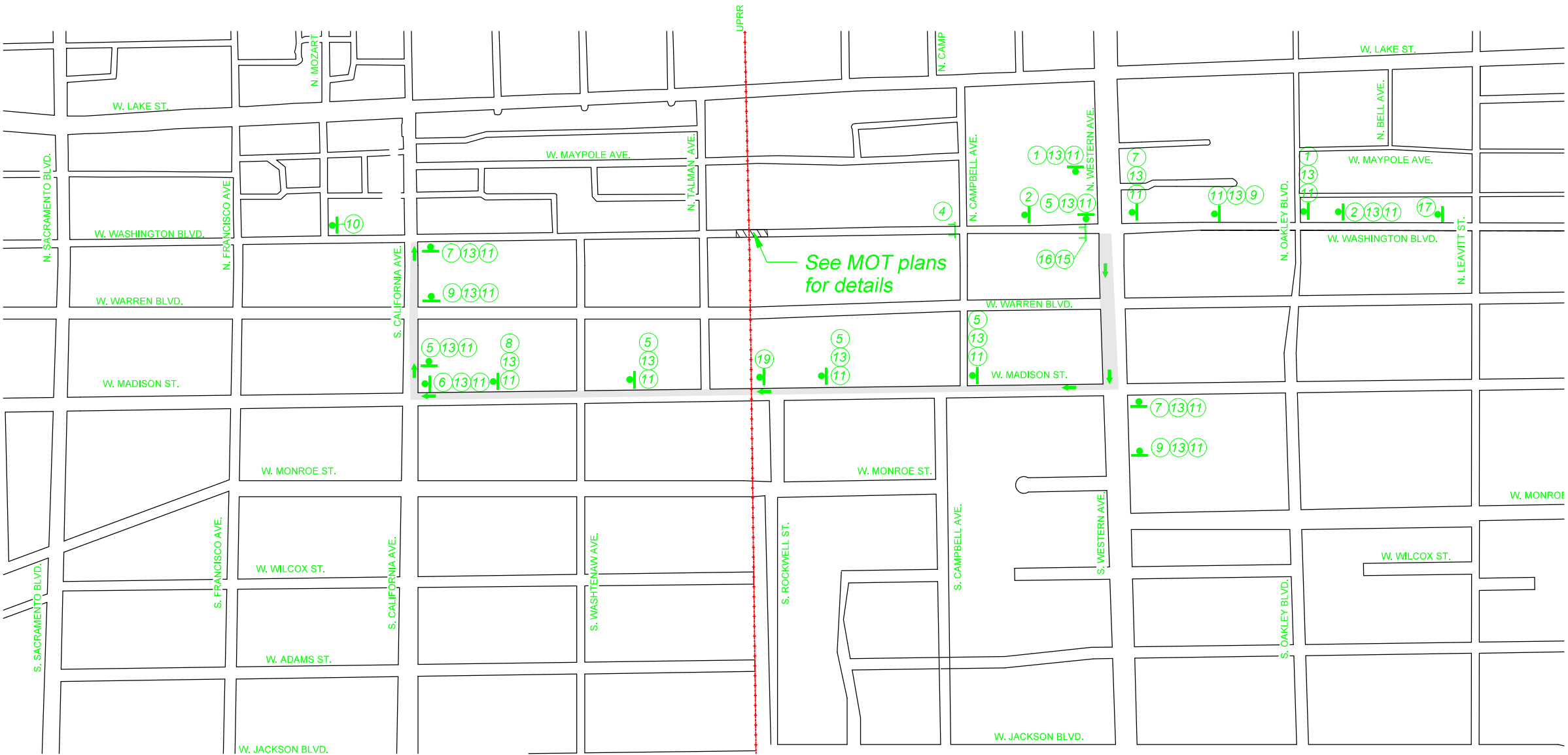
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		CHECKED BY:	PID:		
		DATE:	BUDGET REF:	LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
		SCALE:	SHEET NUMBER		
		1" = 20'	R-005	SHEET TITLE: MOT - WB SOUTH CONST. W. WASHINGTON BLVD.	

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5/26/2021

- DETOUR NOTES:**
1. Full closure of Washington Blvd will take place when the railroad superstructure will be erected. Closure shall take place between the hours of 7 PM to 5 AM.
 2. The Contractor shall coordinate with the Commissioner 20 days in advance of the start of full roadway closure. During the full roadway closure period, parking will be removed on both sides of Washington Blvd, from east of the work zone to Campbell Ave. To inform the public, City personnel will install advance notice of no parking on street.



SIGN LEGEND

1

W20-2 (48x48)

2

W20-3 (48x48)

3

W20-3 (48x48)

4

R11-2 (48x30)

5

M4-9 (24x30)

6

M4-9R (24x30)

7

M4-9L (24X30)

8

M4-9 R (24x30)

9

M4-9L (24x30)

10

M4-8A (24x18)

11

SPECIAL (48x10)

12

M3-2 (24x12)

13

M3-4 (24x12)

14

M4-10L (48x18)

15

M4-10R (48X18)

16

R11-4 (60x30)

17

SPECIAL (72x36)

18

SPECIAL (72x36)

19

W12-2 (48x48)

Work Area

Detour Route

Traffic Sign Key
(Combination Of Signs)

Type III Barricade With Amber
Flashing Lights

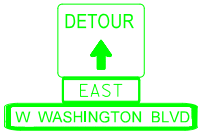
Sign On Portable Or
Permanent Support

48"x48" Construction
Warning Sign, With Amber
Flashing Light

Detoured Traffic Direction

LEGEND

TYPICAL LAYOUT OF SIGNS



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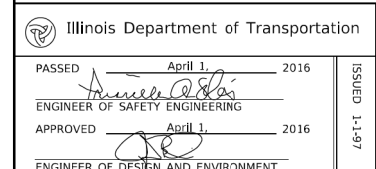
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**UNION PACIFIC
RAILROAD**

Director Structures Design

LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE:
FULL CLOSURE DETOUR - W. WASHINGTON BLVD.



DATE	REVISIONS
4-1-16	Omitted orange safety fence from standard as this is covered in the std. spec.
1-1-12	Added SIDEWALK DIVERSION. Modified appearance of plan views. Renamed Std.

SIDEWALK, CORNER OR CROSSWALK CLOSURE

(Sheet 1 of 2)

STANDARD 701801-06

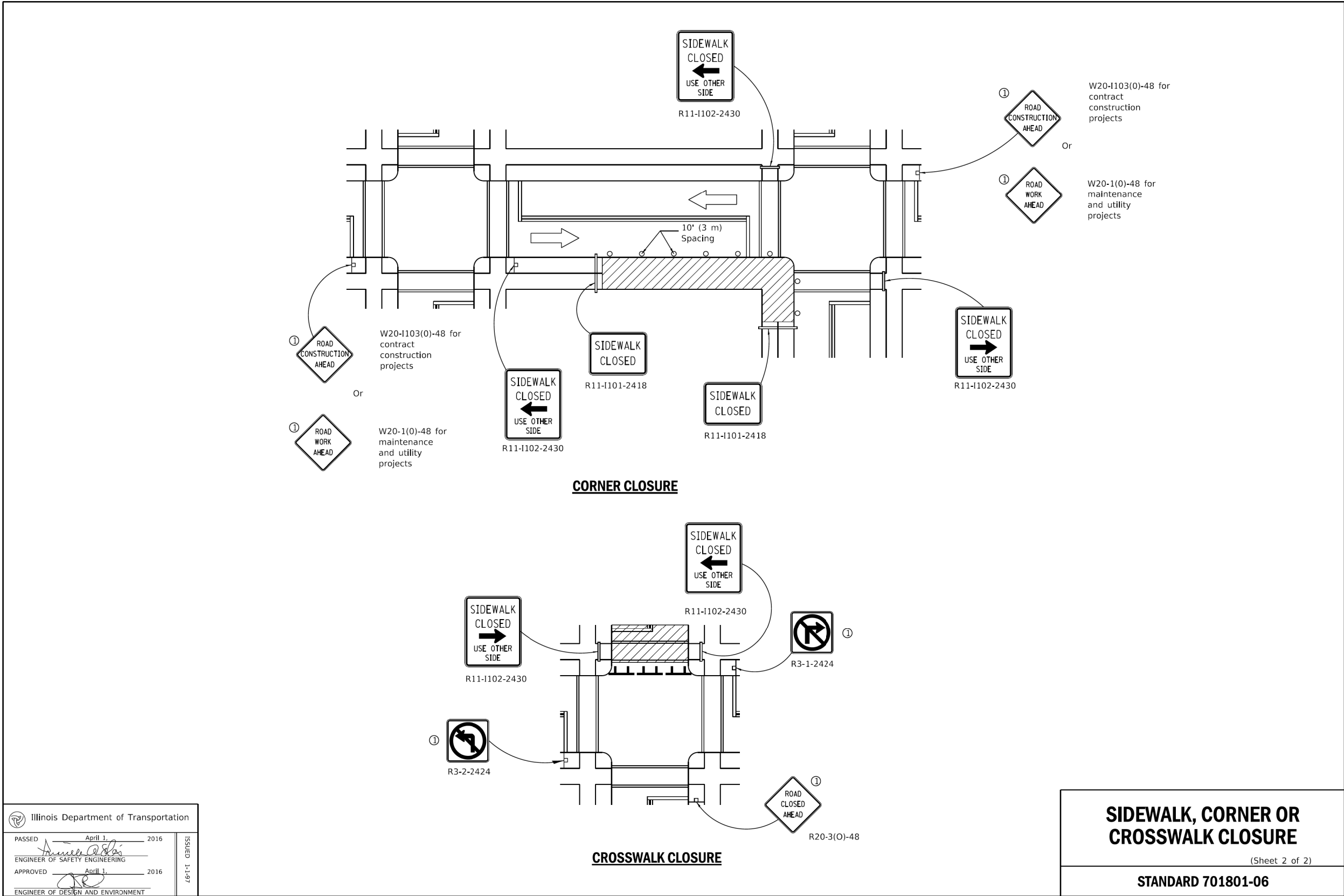
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UNION PACIFIC RAILROAD Director Structures Design	
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
SHEET TITLE: IDOT HIGHWAY STANDARD	

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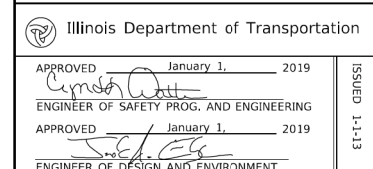
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UNION PACIFIC RAILROAD	Director Structures Design
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
SHEET TITLE: IDOT HIGHWAY STANDARD	



DATE	REVISIONS
1-1-19	Revised cone usage and added cones >36" (900 m) height.
1-1-18	Revised END WORK ZONE SPEED LIMIT sign from orange to white background.

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SHEET TITLE:	IDOT HIGHWAY STANDARD

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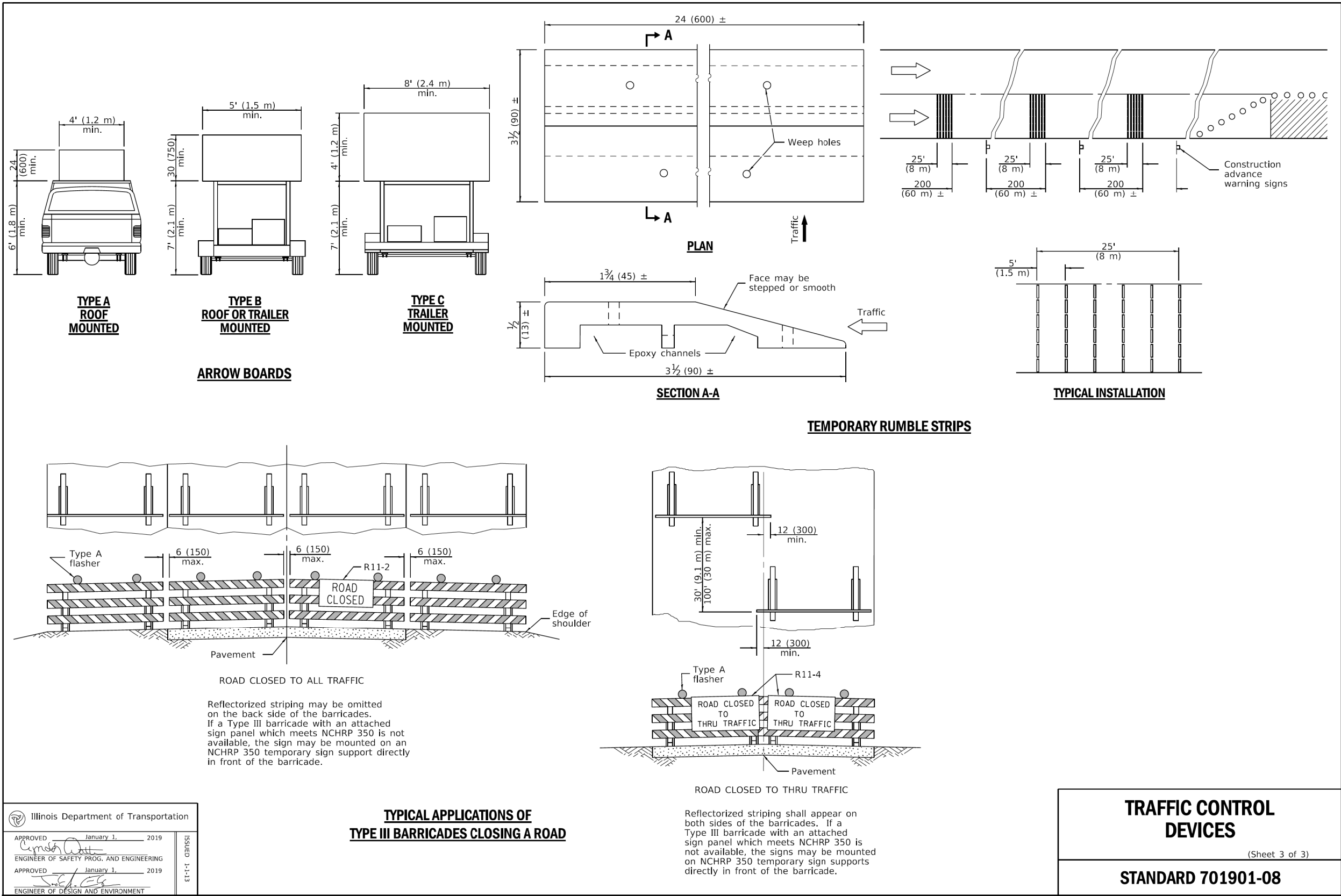
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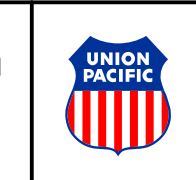
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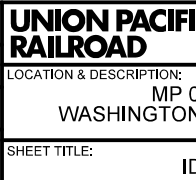
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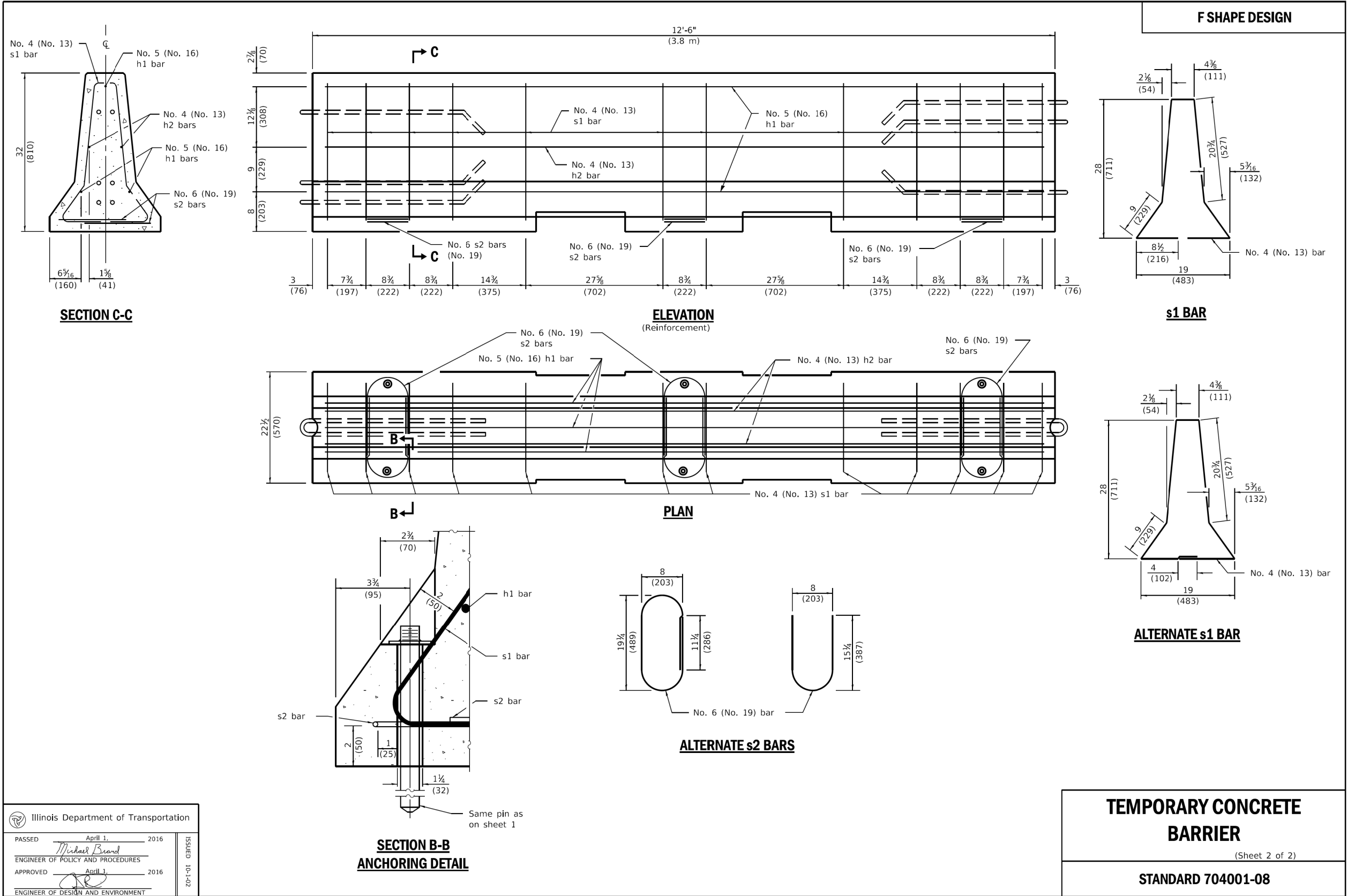


UNION PACIFIC RAILROAD
LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT
SHEET TITLE:
IDOT HIGHWAY STANDARD

Director Structures Design

UNION PACIFIC RAILROAD Director Structures Design	LOCATION & DESCRIPTION:
	MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT
SHEET TITLE:	IDOT HIGHWAY STANDARD

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SHEET TITLE: IDOT HIGHWAY STANDARD	

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	SHEET TITLE: IDOT HIGHWAY STANDARD

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5/26/2021

GENERAL UTILITY NOTES :

COMED :
USE EXTREME CAUTION NEAR ComEd FACILITIES. HAND DIG WHILE CROSSING 69/138/345 kv TRANSMISSION LINE. ComEd TRANSMISSION SHALL BE NOTIFIED 2 BUSINESS DAYS PRIOR TO THE START OF THE WORK. TO SCHEDULE AN ON SITE INSPECTOR DURING CONSTRUCTION, CONTACT LESLIE PASCHAL AT (630) 437-4767.

NO DIRECTIONAL BORING WITHIN 5 FEET OF COMED CONDUITS. CITY CONTRACTORS ARE REQUESTED TO TAKE EXTRA PRECAUTIONS WHEN WORKING NEAR COMED OVERHEAD WIRES. THESE WIRES ARE NOT INSULATED. COMED REQUESTS THAT THE CITY DIRECTIONAL BORE CONTRACTORS PROTECT ALL EXISTING COMED MANHOLES & CONDUITS WHEN CROSSING, ADJACENT TO OR IN THE NEAR VICINITY, BY DIGGING TEST HOLES TO ASSURE COMED FACILITIES ARE NOT DAMAGED, DURING THE DIRECTIONAL BORE PROCESS, & HAND DIG WHEN WITHIN 5 FT OR CROSSING FACILITIES. IF NEEDED CONTACT MICHELLE HO, 331-481-9108 WITH A 6-WEEK NOTICE.

CHICAGO DEPARTMENT OF WATER MANAGEMENT :
WATER MAINS AND SERVICES:

THE MINIMUM VERTICAL CLEARANCE (EDGE-TO-EDGE) FROM WATER MAINS AND SERVICES IS 18- INCHES. FOR GRID MAINS (WATER MAINS LESS THAN 16- INCHES) AND WATER STRUCTURES (VALVE BASINS, SERVICE CONTROL VALVES, ETC.) THE MINIMUM HORIZONTAL CLEARANCE (EDGE-TO-EDGE) IS THREE (3) FEET. FOR FEEDER MAINS (WATER MAINS 16- INCHES AND LARGER), THE MINIMUM HORIZONTAL CLEARANCE (EDGE-TO-EDGE) IS FIVE (5) FEET.ALL LIGHT POLE FOUNDATIONS MUST MAINTAIN A MINIMUM OF FIVE (5) FEET HORIZONTAL CLEARANCE (EDGE-TO-EDGE) TO ALL EXISTING WATER FACILITIES.

FIRE HYDRANTS :

IN NO CASE SHALL THE INSTALLATION OF ANY PROPOSED FACILITY BE CLOSER THAN FIVE (5) FEET FROM A FIRE HYDRANT OR FIRE HYDRANT LEAD. EXTREME CAUTION IS TO BE TAKEN TO ENSURE THAT NO FACILITY OWNED AND MAINTAINED BY THIS DEPARTMENT (DWM) IS DAMAGED DURING CONSTRUCTION. IF DAMAGE OCCURS TO ANY FACILITIES, THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR THE COST OF REPAIRING OR REPLACING THE FACILITIES.

DEPARTMENT OF WATER MANAGEMENT (DWM) SEWER REQUIREMENTS FOR EXISTING FACILITIES PROTECTION, AUGUST 2016.
FOR ALL GENERAL CLEARANCE REQUIREMENTS (NON-WATER):
UPON PROJECT COMPLETION, RESIDENT ENGINEERS MUST CONTACT THE DWM SEWER EVALUATION SECTION AT 312-747-4680 TO SCHEDULE A FIELD MEETING FOR PROJECT ACCEPTANCE.
DWM REQUIRES A MINIMUM HORIZONTAL CLEARANCE OF 1D (INNER DIAMETER OF THE SEWER) + 4’ CENTER-TO-CENTER OR 4 FEET EDGE-TO-EDGE, WHICHEVER IS LARGER, AND A MINIMUM VERTICAL CLEARANCE OF 18" EDGE-TO-EDGE. IF EITHER OF THESE CONDITIONS ARE NOT MET, THE SEWER MUST BE REPLACED OR LINED.

ONLY FOR PARALLEL UTILITY INSTALLATIONS IN THE PARKWAY:
UPON PROJECT COMPLETION, RESIDENT ENGINEERS MUST CONTACT THE DWM SEWER EVALUATION SECTION AT 312-747-4680 TO SCHEDULE A FIELD MEETING FOR PROJECT ACCEPTANCE.
PRIVATE SEWER DRAINS (W/BASEMENTS) TYPICALLY HAVE APPROXIMATELY 5 FEET OF COVER. DUE TO POTENTIAL VERTICAL CONFLICTS IN THE PARKWAY, THE CONTRACTOR MUST TELEVIEW ALL PRIVATE DRAINS AFTER CONSTRUCTION. A COPY OF THE DVD MUST BE PRESENTED TO THE DWM SEWER INSPECTOR FOR PROJECT ACCEPTANCE.

THE FOLLOWING NOTE MUST BE DISPLAYED ON THE CONSTRUCTION PLANS:
THE PROPOSED WORK ENCROACHES UPON MINIMUM CITY SEWER CLEARANCE REQUIREMENTS. EXTREME CAUTION IS REQUIRED DURING CONSTRUCTION. THE CITY SEWER MUST BE TELEVIEWED BEFORE AND AFTER CONSTRUCTION TO ASSESS ITS CONDITION. CONTACT THE DWM SEWER EVALUATION SECTION TWO BUSINESS DAYS PRIOR TO CONSTRUCTION AT (312) 747-4680 TO COORDINATE THE WORK.
IF SEWER DAMAGE OCCURS, THE CONTRACTOR AND/OR OWNER WILL BE RESPONSIBLE FOR THE COST OF REPAIRING, LINING OR REPLACING THE DAMAGED FACILITIES.

PEOPLES GAS :
PEOPLES GAS FACILITIES ARE PRESENT WITHIN CONSTRUCTION. USE EXTREME CAUTION NEAR ALL GAS FACILITIES DURING CONSTRUCTION AND RELATED EXCAVATION ACTIVITIES. HAND EXCAVATION IS REQUIRED TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF GAS MAIN(S) PRIOR TO CROSSING AND WORKING WITHIN 3’ OF ALL GAS FACILITIES. A MINIMUM OF 3’ HORIZONTAL EDGE-TO-EDGE CLEARANCE IS REQUIRED FOR GAS MAINS WITH DIAMETERS OF 16” OR SMALLER, & 5’ EDGE-TO-EDGE CLEARANCE FOR GAS MAINS WITH DIAMETERS 18” AND LARGER. MAINTAIN A MINIMUM OF 1.5’ EDGE-TO-EDGE VERTICAL CLEARANCE ON GAS MAINS 16” OR LESS IN DIAMETER, AND 2’ EDGE-TO-EDGE VERTICAL CLEARANCE ON 18” AND LARGER DIAMETER GAS MAINS. CONTACT DIGGER 312-744-7000 FOR LOCATES 48 HOURS PRIOR TO START OF CONSTRUCTION.

THE USE OF CONCRETE, FLOW FILL, OR THE LIKE IS PROHIBITED WITHIN 24” OF ALL GAS FACILITIES, NOR SHALL IT ENCASE ANY GAS FACILITY. A BUFFER OF 24” SAND IS TO USED BETWEEN FLOW FILL AND ALL GAS FACILITIES. A MINIMUM OF 6” FA-02 OR FM-02 SAND SHALL BE USED WHEN BACKFILLING OTHER MATERIALS AROUND ANY EXPOSED GAS FACILITY. CONTRACTOR EXPOSING GAS FACILITY IS RESPONSIBLE FOR PROVIDING THE SAND. ANY DAMAGES TO PEOPLES GAS FACILITIES SHALL BE THE RESPONSIBILITY OF THE INSTALLING UTILITY AND THEIR CONTRACTOR(S).

SERVICE MAY BE EFFECTED. USE EXTREME CAUTION NEAR ALL GAS FACILITIES
HAND EXCAVATION IS REQUIRED TO LOCATE AND EXPOSE GAS FACILITIES PRIOR TO CROSSING AND WORKING WITHIN 3’ OF ALL GAS FACILITIES. CONTACT DIGGER FOR LOCATES 48 HOURS PRIOR TO START OF CONSTRUCTION. PEOPLES GAS FACILITIES ARE PRESENT WITHIN AREA OF CONSTRUCTION. DIRECTIONAL DRILLING IS CONSIDERED A HIGH RISK EXCAVATION AS DETERMINED BY PHMSA AND NTSB; AS SUCH SPECIAL CONSIDERATIONS SHALL TAKE PLACE NEAR NATURAL GAS DISTRIBUTION LINES. USE EXTREME CAUTION NEAR ALL GAS FACILITIES DURING CONSTRUCTION AND RELATED EXCAVATION ACTIVITIES. HAND DIG OR NON-INVASIVE EXCAVATION IS REQUIRED TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF GAS FACILITIES PRIOR TO CROSSING AND WORKING WITHIN 3’ OF ALL GAS FACILITIES. A MINIMUM OF 3’ HORIZONTAL EDGE-TO-EDGE CLEARANCE IS REQUIRED FOR GAS MAINS WITH DIAMETERS OF 16” OR SMALLER, & 5FT EDGE-TO-EDGE CLEARANCE FOR GAS MAINS WITH DIAMETERS 18” AND LARGER.MAINTAIN A MINIMUM OF 1.5’ EDGE-TO-EDGE VERTICAL CLEARANCE ON GAS MAINS 16” OR LESS IN DIAMETER, AND 2’ EDGE-TO-EDGE VERTICAL CLEARANCE ON 18” AND LARGER DIAMETER GAS MAINS. CONTRACTOR MUST OBSERVE BORE HEAD WHILE CROSSING GAS FACILITIES AND VERIFY THAT MINIMUM VERTICAL CLEARANCE IS OBTAINED. LOCATIONS OF EXISTING GAS FACILITIES MUST BE VERIFIED AT SUFFICIENT INTERVALS, OR A MAX OF DISTANCE OF 50’, WHEN PARALLELING TO ENSURE SEPARATION IS MAINTAINED. CONTACT SYSTEM INTEGRITY OPERATIONS SUPERVISOR, A MINIMUM OF 5 BUSINESS DAYS PRIOR TO EXCAVATION TO SET UP ON-SITE INSPECTION.CONTACT DIGGER 312-744-7000 FOR LOCATES 48 HOURS PRIOR TO START OF CONSTRUCTION AND SPECIFY EXCAVATION TYPE.

North Shop: North of Chicago Ave/Ashland Ave /North Ave
Robert Goral, 773 647-0537
Central Shop: South of Chicago Ave/ Ashland Ave. / North Ave to
North of 87th St / Ashland Ave / Chicago River / Cermak Rd
Dave Marquez, 773-858-1825
South Shop: South of 87th St / Ashland Ave / Chicago River / Cermak Rd
Jacob Weber, 312-806-0559

THE USE OF CONCRETE, FLOW FILL, OR THE LIKE IS PROHIBITED WITHIN 24” OF ALL GAS FACILITIES, NOR SHALL IT ENCASE ANY GAS FACILITY. A BUFFER OF 24” SAND IS TO USED BETWEEN FLOW FILL AND ALL GAS FACILITIES. A MINIMUM OF 6” FA-02 OR FM-02 SAND SHALL BE USED WHEN BACKFILLING OTHER MATERIALS AROUND ANY EXPOSED GAS FACILITY. CONTRACTOR EXPOSING GAS FACILITY IS RESPONSIBLE FOR PROVIDING THE SAND. ANY DAMAGES TO PEOPLES GAS FACILITIES SHALL BE THE RESPONSIBILITY OF THE INSTALLING UTILITY AND THEIR CONTRACTOR(S). CALL 866-556-6002 IMMEDIATELY FOR ANY DAMAGES TO THE GAS FACILITIES. ALL GAS FACILITIES ARE TO BE MAINTAINED. THIS PROJECT WILL BE PLACED ON PEOPLES GAS WATCH & PROTECT.

BUREAU OF FORESTRY :
TREE PROTECTION MEASURES:
AT A MINIMUM, NEED TO INCLUDE ORANGE SNOW FENCE TO BE INSTALLED ALONG BACK OF THE CURB-LINE, 10 FEET FROM THE BACK OF THE CURB-LINE AND AT A DISTANCE OF 5 FEET FROM EITHER SIDE OF THE TREE. IF TRIMMING IS REQUIRED FOR LINE INSTALLATION, IT WILL BE DONE BY AN INSURED TREE SERVICE COMPANY. TRIMMING WILL BE DONE IN ACCORDANCE TO AMERICAN NATIONAL STANDARDS INSTITUTE GUIDELINES, LEAVING THE FORM AND STRUCTURE OF THE TREES INTACT.

TUNNELING SPECIFICATIONS (EACH SIDE OF TREE)
TREE SIZE* TUNNEL/TRENCH DISTANCE
< 10" 5 FEET
10" TO 19" 10 FEET
> 19" 15 FEET
* DBH = DIAMETER AT BREAST HEIGHT (4.5’)

• STAY OUT OF THE DRIP LINE
• STOP TRENCHING WHEN ROOTS OF (2) INCHES OR LARGER ARE ENCOUNTERED.

CTA BUS ROUTES :
PLEASE NOTIFY CTA AT LEAST (2) TWO WEEKS PRIOR TO ANY SIDEWALK, LANE, OR STREET CLOSURES, OR THE REMOVAL OF ANY BUS STOP SIGNS SO THAT CTA CAN FACILITATE ANY NECESSARY DETOURS OR BUS STOP RELOCATIONS.

WARNING !
FIBER OPTIC CABLE
ON RAILROAD R-O-W
CALL BEFORE YOU DIG
1-800-336-9193

ISSUED FOR
CONSTRUCTION

REVISION	BY	DATE	DESCRIPTION

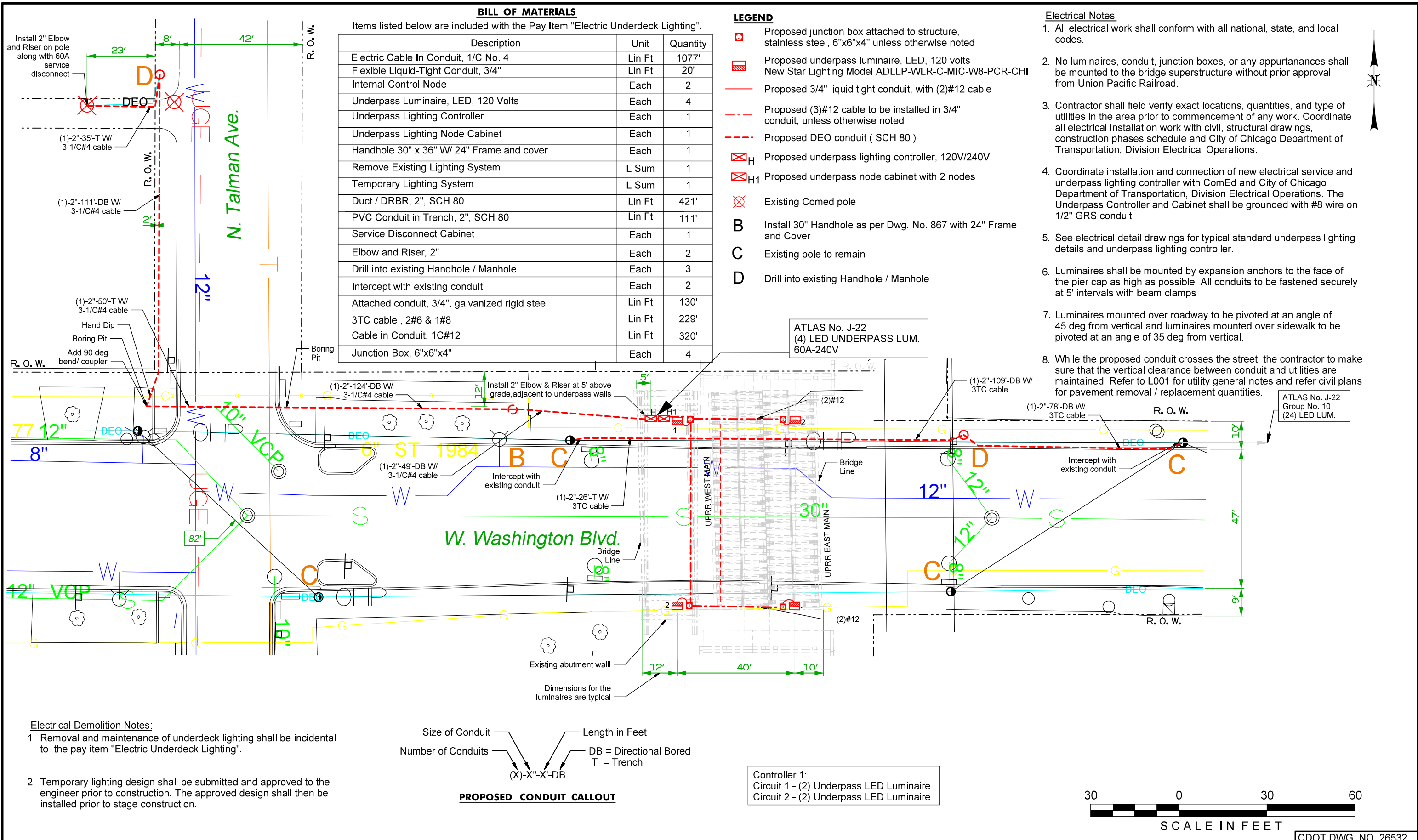


Alfred Benesch & Company
35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



DRAWN BY: RS	WORK ORDER: 31876	UNION PACIFIC RAILROAD Director Structures Design
CHECKED BY: GT	PID:	
DATE: 05/28/21	BUDGET REF:	
SCALE: N.T.S	SHEET NUMBER L-001	LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT
		SHEET TITLE: ELECTRICAL GENERAL NOTES

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5/26/2021



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REVISION	BY	DATE	DESCRIPTION

benesch

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Chicago, Illinois 60601

312-565-0450 Job No. 210070.11

UNION PACIFIC

DRAWN BY: RS	WORK ORDER: 31876	UNION PACIFIC RAILROAD Director Structures Design
CHECKED BY: GT	PID:	
DATE: 05/28/21	BUDGET REF:	
SCALE: 1" = 30'	SHEET NUMBER L-002	

LOCATION & DESCRIPTION:

MP 0.93 ROCKWELL SUBDIVISION

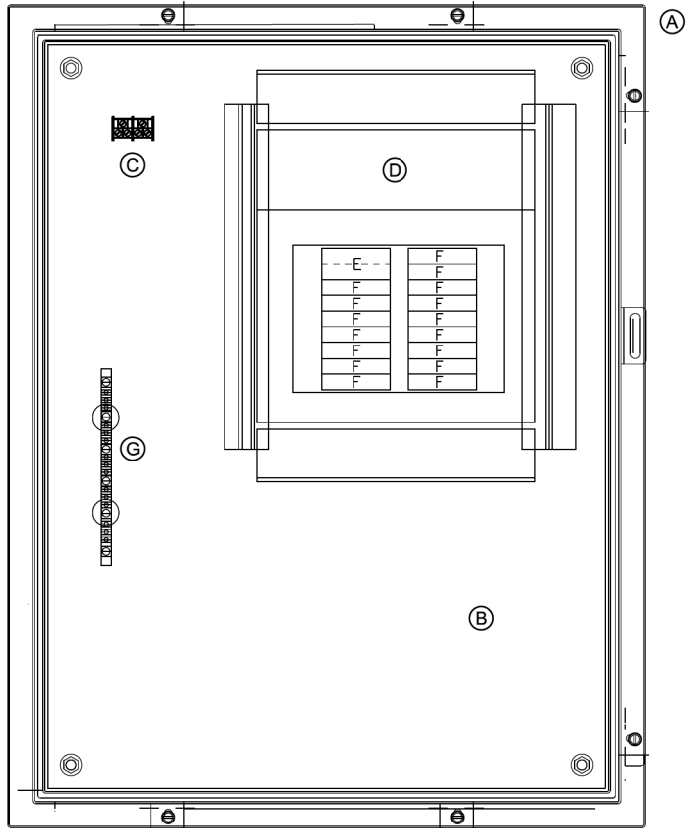
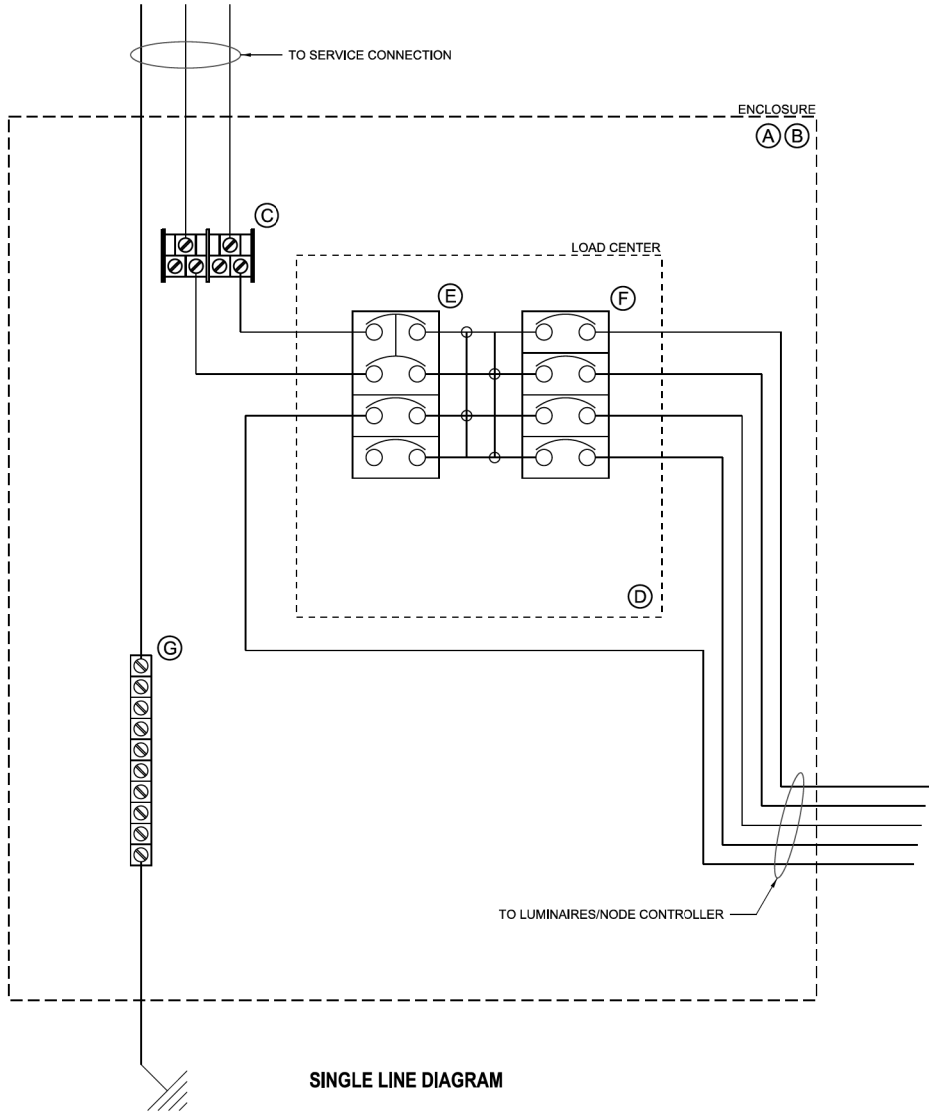
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE: PROPOSED UNDERDECK LIGHTING

CDOT DWG. NO. 26532

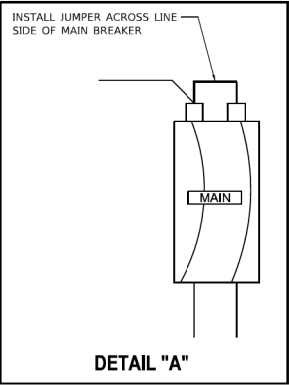
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Default: 5/26/2021

DATE PLOTTED: 10/30/2019
FILE NAME: NSPRC\020231.00\020231.01\05-New Specs & Standards\XXX\Underpass Lighting Controller\Underpass Lighting Controller Detail.dgn



- NOTES:
- ENCLOSURE SHALL HAVE A PIANO HINGE, DOOR CLAMPS AND HASP/STAPLE FOR PADLOCK.
 - TERMINAL BLOCK SHALL HAVE A MINIMUM CURRENT RATING OF 200A. LINE SIDE SHALL ACCEPT #2 THROUGH #3/0 CABLE. LOAD SIDE SHALL ACCEPT (1) #2 AND (1) #2/0 CABLE.
 - LOAD CENTER SHALL INCLUDE PROVISIONS FOR MAIN AND BRANCH BREAKERS. LIVE BUS SHALL BE PROTECTED IN A MANNER WHICH ALLOWS FUTURE INSTALLATION OF INDIVIDUAL CIRCUIT BREAKERS WITHOUT AFFECTING PROTECTION OF REMAINING BUS.
 - MAIN BREAKER SHALL BE SIZED AS NOTED ON THE PLANS.
 - THE NUMBER OF 15A SINGLE POLE CIRCUIT BREAKERS SHALL BE AS NOTED ON THE PLANS. A MINIMUM OF 4 BREAKERS SHALL BE PROVIDED.
 - FOR 2-WIRE 120V APPLICATION, SEE DETAIL "A".

DEVICE SCHEDULE		
ITEM	QUANTITY	DESCRIPTION
A	1	ENCLOSURE, NEMA 4X, TYPE 304 STAINLESS STEEL, 30" X 24" X 8" (SEE NOTE 1)
B	1	BACK PANEL, PAINTED STEEL, REMOVABLE
C	2	TERMINAL BLOCK, 600V (SEE NOTE 2)
D	1	18-SPACE LOAD CENTER, 120/240V, 1 PHASE, 3 WIRE, 100A, 10K AIC FULLY RATED (SEE NOTE 3)
E	1	CIRCUIT BREAKER, 60A OR 100A, 2PST, THERMAL MAGNETIC, 10KAIC@240V (SEE NOTE 4)
F	NOTE 5	CIRCUIT BREAKER, 15A, SPST, THERMAL MAGNETIC, 10KAIC@240V
G	1	NEUTRAL BUS TERMINAL LUGS



A		
REV	DATE	DESCRIPTION
UNDERPASS LIGHTING CONTROLLER, CONSTANT POWER, 120/240 VOLT		
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING		
DRAFTSMAN:	M. STANGEL	DWG. NO. 861A
ENGINEER:	J. VONDRA	
ENGINEER OF ELECTRICITY:		
DEPUTY COMMISSIONER:		
SIZE:	22" 34"	SCALE: NONE
		DATE: 10/30/2019

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ISSUED FOR
CONSTRUCTION

REVISION	BY	DATE	DESCRIPTION



Alfred Benesch & Company
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Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



DRAWN BY: RS	WORK ORDER: 31876
CHECKED BY: GT	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: N.T.S	SHEET NUMBER L-003

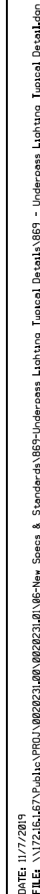


Director Structures Design

LOCATION & DESCRIPTION:
MP 0.93 ROCKWELL SUBDIVISION
WASHINGTON BOULEVARD BRIDGE REPLACEMENT

SHEET TITLE:
LIGHTING DETAILS

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5/26/2021



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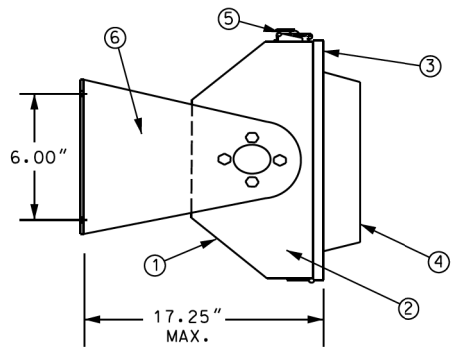
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CONSTRUCTION

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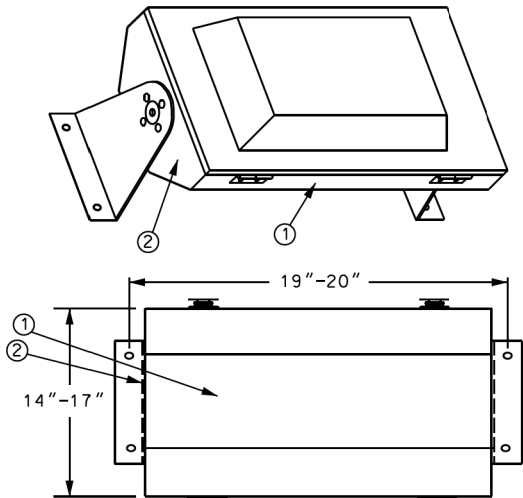
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CHECKED BY: GT		PID:			
DATE: 05/28/21		BUDGET REF:		LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
SCALE: N.T.S		SHEET NUMBER L-004		SHEET TITLE: LIGHTING DETAILS	

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5/26/2021

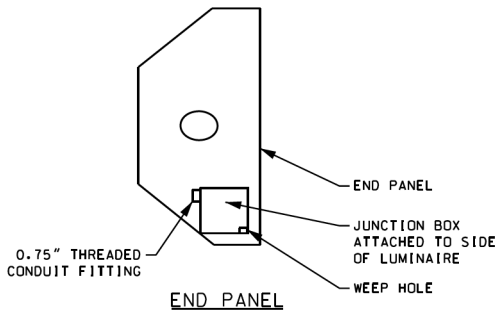
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FILE: N:\Spec\Locations\MISC\New Drawings\981_03302016.dgn



SIDE VIEW



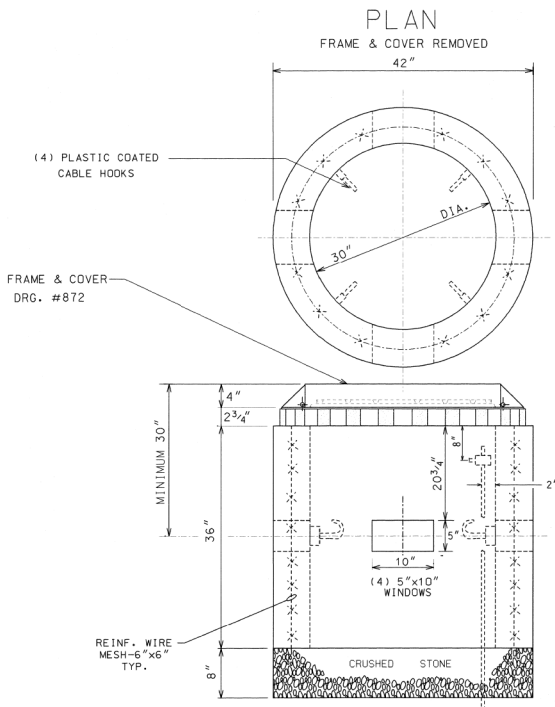
BACK VIEW



END PANEL

- 1) HOUSING
2) END PANEL
3) LENS DOOR/COVER ASSEMBLY
4) LENS
5) LATCHES
6) END MOUNT TRUNNION BRACKET

VIADUCT / EL STRUCTURE LUMINAIRE		
CITY OF CHICAGO DEPT. OF STREETS AND SANITATION BUREAU OF ELECTRICITY DIVISION OF ELECTRICAL ENGINEERING		
DRAWN BY B. CARTER	CHECKED BY E. KARAGIANES	ENGINEER
SUPERVISING ENGINEER ELEC. DESIGN ENGINEER	DWG. NO 981	
GEN'L. APP' OF ELECTRICAL ELECTRICAL ENGINEER		
DEPUTY COMMISSIONER ELECTRICAL ENGINEER		
SCALE: NONE	DATE: 4/12/04	MAT. SPEC: 1532



COMPLETE COMMODITY CODE NO. 05-6610-5310M			
CODE NO.	MATERIALS	SIZE	QUAN.
(1) 05-6610-5310	PRE-CAST HANDHOLE	30"X36"	1
(2) 05-9075-5470	STONE 3/4" CRUSHED	BAG	5
(2) 05-5082-5330	SOND TUBE	30"	1
(2) 05-5082-5342	SOND TUBE	42"	1
(2) 05-3267-2940	CONC. RED1-MIX	CU. YD.	1/2
(2) 57-0770-0000	6" X 6" MESH	36"X10'	1
05-1452-9720	BRICK		24
02-4299-5524	FRAME MANHOLE	24"	1
02-4574-5040	COVER, MANHOLE	24"	1
09-7796-9312	GROUND ROD	3/4"X12'	1
09-2630-3240	GROUND CLAMP		1

- (1) PRE-CAST HANDHOLE SHALL INCLUDE CABLE HOOKS AND CONDUIT KNOCKOUTS.
(2) THESE ITEMS ARE FOR POURED-IN-PLACE HANDHOLES ONLY.

CONSTRUCTION NOTES:

1. 8" BED OF STONE FOR DRAINAGE.
2. ALL METALLIC CONDUITS ENTERING HANDHOLE SHALL EXTEND MINIMUM 1" & MAXIMUM 3" INSIDE INNER WALL AND BE EQUIPPED WITH AN APPROVED TYPE OF THREADED GROUNDING BUSHING.

81-23-00	ADDED CABLE HOOKS PER COMMISSIONER MURPHY	05/28/21
12-0-00	REVISION (1)	MP
30" DIA. CONCRETE HANDHOLE		
CITY OF CHICAGO		
DEPT. OF STREETS AND SANITATION	DEPT. OF STREETS AND SANITATION	DEPT. OF STREETS AND SANITATION
DRAWN BY B. CARTER	CHECKED BY E. KARAGIANES	ENGINEER
SUPERVISING ENGINEER ELEC. DESIGN ENGINEER	DWG. NO 867	
GEN'L. APP' OF ELECTRICAL ELECTRICAL ENGINEER		
DEPUTY COMMISSIONER ELECTRICAL ENGINEER		
SCALE: 11"	DATE: 12-8-95	MAT. SPEC: 1532

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**ISSUED FOR
CONSTRUCTION**

REVISION	BY	DATE	DESCRIPTION

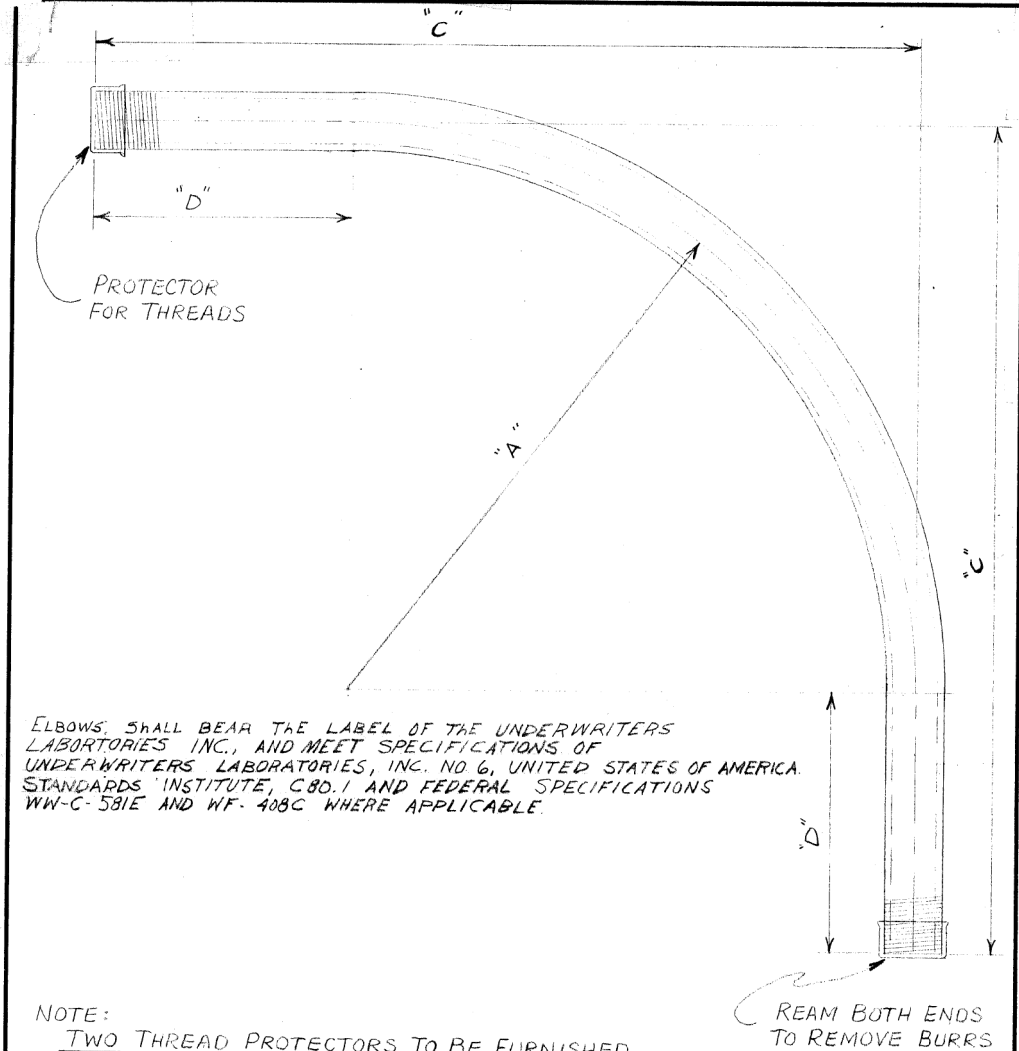
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35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



DRAWN BY: RS	WORK ORDER: 31876
CHECKED BY: GT	PID:
DATE: 05/28/21	BUDGET REF:
SCALE: N.T.S	SHEET NUMBER L-005

UNION PACIFIC RAILROAD	Director Structures Design
LOCATION & DESCRIPTION: MP 0.93 ROCKWELL SUBDIVISION WASHINGTON BOULEVARD BRIDGE REPLACEMENT	
SHEET TITLE: LIGHTING DETAILS	

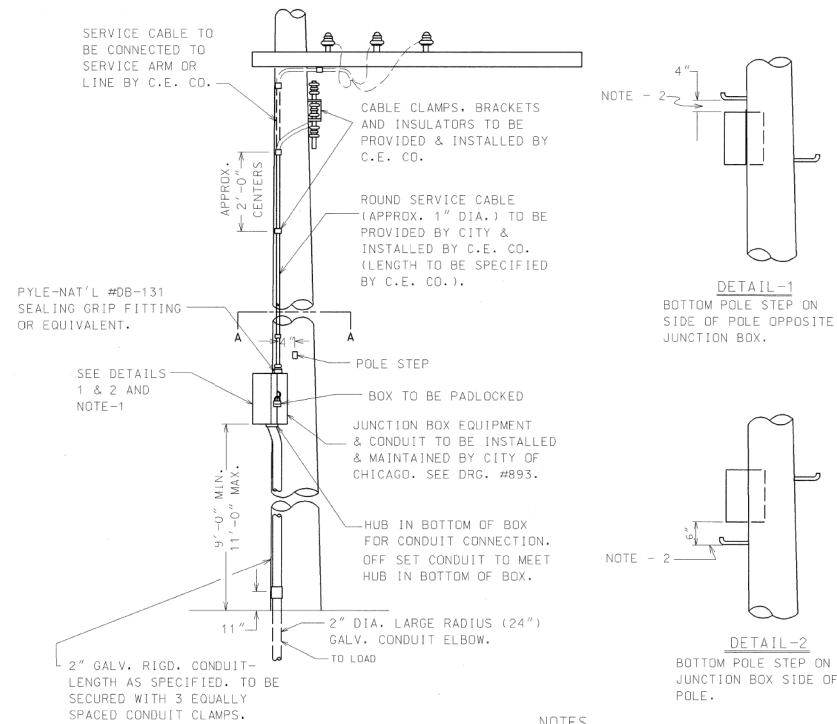
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5/26/2021



NOTE:
TWO THREAD PROTECTORS TO BE FURNISHED ON EACH ELBOW, PROTECTOR TO COVER A MINIMUM OF TEN THREADS.

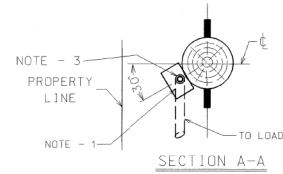
TABLE OF DIMENSIONS				
CONDUIT SIZE	DIMENSIONS			COMMODITY CODE
	"A"	"C"	"D"	
1 1/4"	24"	35"	11"	09-4001-0510
1 1/2"	24"	35"	11"	09-4001-0520
2"	24"	35"	11"	09-4001-4126
2 1/2"	24"	35"	11"	09-4001-4128
3"	24"	35"	11"	09-4001-4230
4"	24"	35"	11"	09-4001-0000

SPECIFICATIONS REVISED			
A REVISED DIMENSIONS ON 3" & 4" CONDUIT L.P.			
ELBOW, CONDUIT, RIGID GALVANIZED STEEL, LARGE RADIUS			
CITY OF CHICAGO			
DEPT. OF STREETS AND SANITATION			
BUREAU OF ELECTRICITY			
DIVISION OF ELECTRICAL ENGINEERING			
REVISED			
A 7-22-71			
B 4-3-79			
C			
D			
E			
F			
G			
DRAWN	CHECKED	ENGINEER	
LON PURDY	M.S.	M. SHINE	
		DRG. NO.	
		11825	
		DEPT. COMM.	
		SCALE: 3/16"	
		DATE 6-2-71	



NOTES

- 1 - WHERE POSSIBLE THE JUNCTION BOX SHALL BE LOCATED FACING THE PROPERTY LINE.
- 2 - BOX SHALL HAVE A MINIMUM CLEARANCE OF 4" BELOW POLE STEP, DETAIL-1, OR 6" ABOVE STEP, DETAIL-2.
- 3 - SERVICE CABLE TO ENTER BOX THROUGH SEALING GRIP FITTING IN TOP.



A	9-3-96	REDRAW	MP.
REVISION			
DATE			
INSTALLATION OF SERVICE EQUIPMENT ON C.E. CO. WOOD POLES			
CITY OF CHICAGO			
DEPT. OF STREETS AND SANITATION			
BUREAU OF ELECTRICITY			
DIVISION OF ELECTRICAL ENGINEERING			
DRAFTSMAN:	CHIEF DRAFTSMAN:	ENGINEER:	
E. LEMASTER	J. BORE	J. BORE	
SUPERVISING ENGINEER:	ELEC. DESIGN ENGR.	DWG. NO.	
S.W. BERTRAM		11925	
ENGINEER OF ELECTRICITY:		DATE: 12-26-56	
GEN'L. SUPT. OF ELECTRICITY:			
DEPUTY COMMISSIONER:			
SIZE:			

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ISSUED FOR
CONSTRUCTION

REVISION	BY	DATE	DESCRIPTION

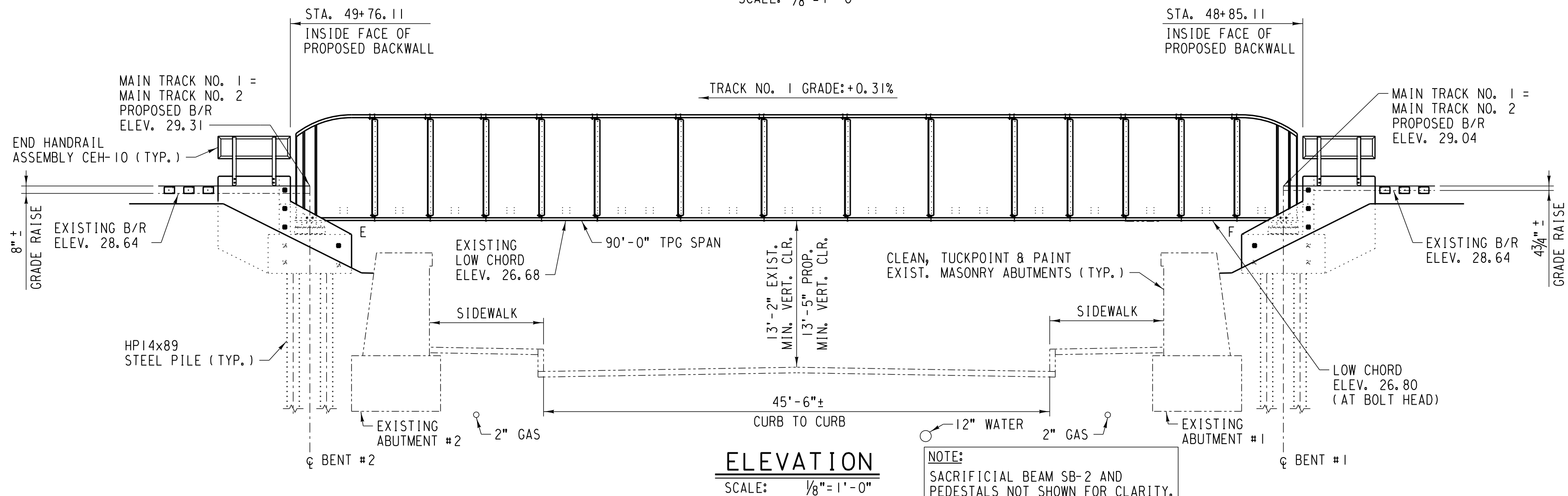
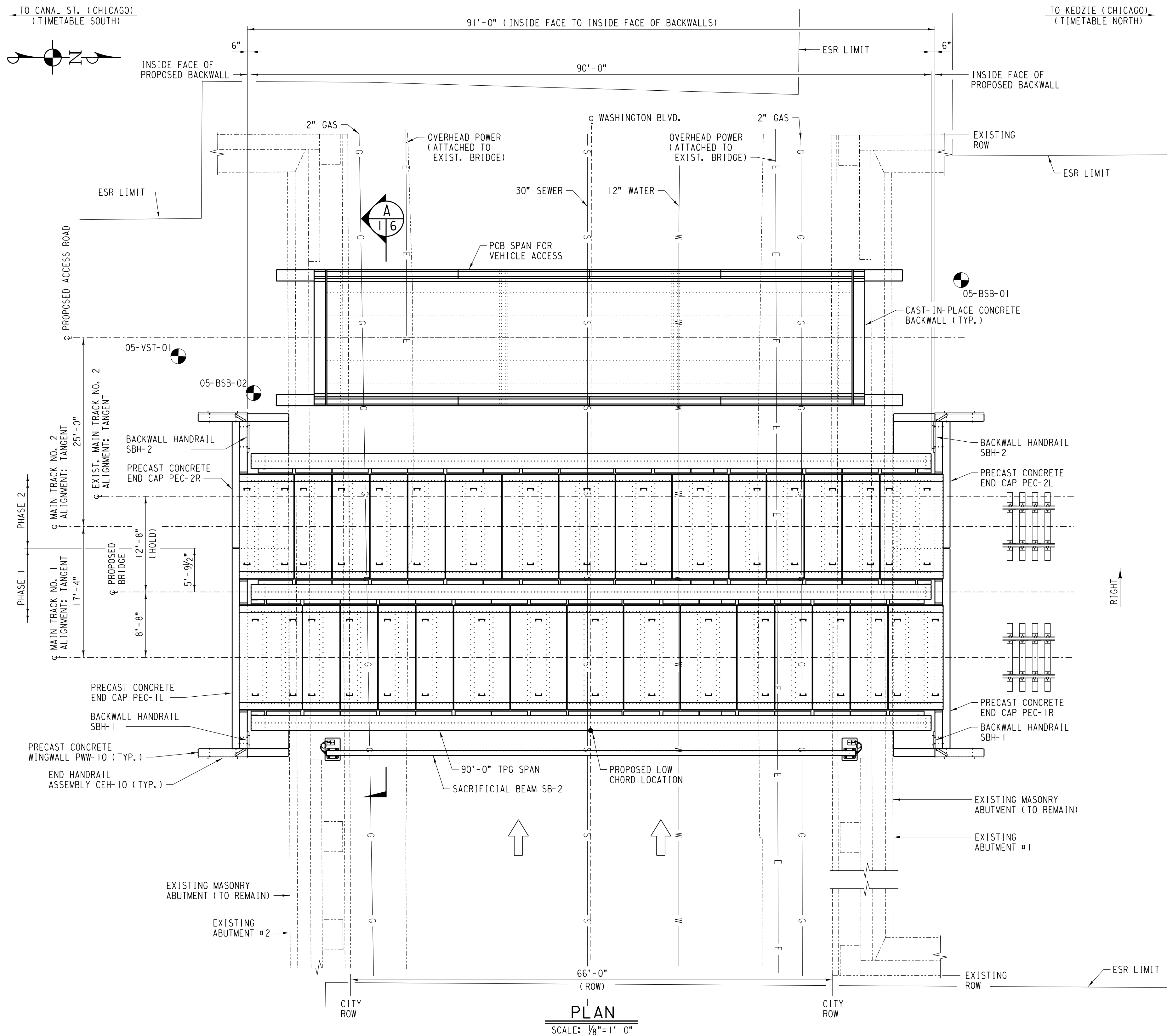
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35 W. Wacker Drive Suite 3300
Chicago, Illinois 60601
312-565-0450 Job No. 210070.11



DRAWN BY:	WORK ORDER:
RS	31876
CHECKED BY:	PID:
GT	
DATE:	BUDGET REF:
05/28/21	
SCALE:	SHEET NUMBER
N.T.S	L-006

UNION PACIFIC RAILROAD		Director Structures Design
LOCATION & DESCRIPTION:		
MP 0.93 ROCKWELL SUBDIVISION		
WASHINGTON BOULEVARD BRIDGE REPLACEMENT		
SHEET TITLE:		
LIGHTING DETAILS		

FILE NAME: C:\Users\mfranklin\OneDrive\Documents\Rockwell\020931.dgn



- NOTES:
- FOR PILE LAYOUT AND PILE DRIVING DIAGRAM, SEE SHEET NO. 2.
 - LOCATION OF KNOWN UTILITIES IS APPROXIMATE. LOCATION SHALL BE VERIFIED PRIOR TO CONSTRUCTION. NOTIFY DIGGER/811 CHICAGO AT 312-744-7000 TO HAVE THE LOCATION OF EXISTING UTILITIES STAKED TWO WORKING DAYS, 48 HOURS, BEFORE STARTING EXCAVATION.
 - E = EXPANSION
F = FIXED

LETTER SERIES

SHEET NO. CUT ON

SHEET NO. SHOWN ON

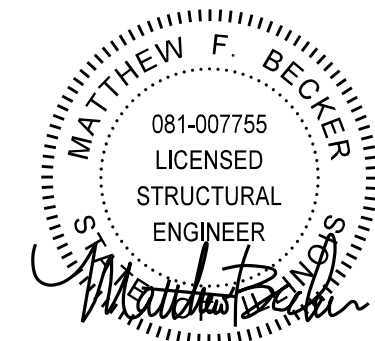
SECTION DESIGNATION

POSTCONSTRUCTION COMPLIANCE

Contractor or UPRR Manager in charge of construction to provide to the office of the Director Structures Design as-built drawings confirming that the project was constructed in compliance with the plans and indicating any construction variances.

IN CHARGE OF CONSTRUCTION

DATE



EXPIRATION DATE: 11-30-2022
DATE: 02-12-2021

DRAWING SCHEDULE

SHEET NO.	DESCRIPTION
E1	GENERAL ARRANGEMENT
E2	PILE LAYOUT AND PILE DRIVING DIAGRAM
E3	SUBSTRUCTURE LAYOUT
E4	GENERAL NOTES AND BILL OF MATERIAL
E5	REMOVAL DETAILS
E6	TYPICAL SECTIONS
E7	SACRIFICIAL BEAM AND MISCELLANEOUS DETAILS
E8	BORING LOGS
E9	PRECAST CONCRETE END CAP 1R DETAILS
E10	PRECAST CONCRETE END CAP 1L DETAILS
E11	PRECAST CONCRETE END CAP 2R DETAILS
E12	PRECAST CONCRETE END CAP 2L DETAILS
E13	PRECAST CONCRETE WINGWALL PW-10 DETAILS
E14	ACCESS BRIDGE CAST-IN-PLACE ABUTMENTS
E15	ACCESS BRIDGE PLAN AND CROSS SECTION
E16	ACCESS BRIDGE PARAPET DETAILS
E17	ACCESS BRIDGE EXPANSION JOINT DETAILS
E18	ACCESS BRIDGE REINFORCEMENT DETAILS (SHEET 1 OF 2)
E19	ACCESS BRIDGE REINFORCEMENT DETAILS (SHEET 2 OF 2)
E20	ACCESS BRIDGE MISCELLANEOUS DETAILS
E21	TPG FRAMING PLAN
E22	TPG BALLAST PAN AND GRATING PLAN
E23	TPG GIRDER ELEVATIONS AND DETAILS
E24	TPG TYPICAL SECTIONS (SHEET 1 OF 2)
E25	TPG TYPICAL SECTIONS AND NOTES (SHEET 2 OF 2)
E26	TPG ASSEMBLY DETAILS
E27	TPG KNEE BRACE DETAILS (SHEET 1 OF 2)
E28	TPG KNEE BRACE DETAILS (SHEET 2 OF 2)
E29	TPG FLOORBEAM CONNECTION PIECE DETAILS
E30	TPG MISCELLANEOUS PIECE DETAILS AND GRATING PANELS
E31	TPG FLOORBEAM DETAILS (SHEET 1 OF 2)
E32	TPG FLOORBEAM DETAILS (SHEET 2 OF 2)
E33	TPG STIFFENER ANGLE DETAILS
E34	TPG BEARING STIFFENER DETAILS
E35	TPG BALLAST PAN DETAILS
E36	TPG BALLAST PLATE DETAILS
E37	TPG STRESS, MATERIAL LIST AND LIFTING WEIGHT TABLES
E38	TPG BEARING LAYOUT
E39	TPG BEARING DETAILS - TYPE A
E40	TPG BEARING DETAILS - TYPE B
E41	TPG BEARING DETAILS - TYPE C
E42	TPG BEARING DETAILS - TYPE D
E43	NONSTANDARD MISCELLANEOUS STEEL DETAILS

REF.	NO.	DWG. NO.	SHEET NO.	REV. NO.	DESCRIPTION
	1	531100	T2 & T3	-	GENERAL NOTES
	2	531110	H1	-	PILE INSTALLATION DETAILS AND NOTES

EST. WT. OF EXISTING SPAN

70'-0" TPGOD (STEEL ONLY) = 310,000 LB. (155.0 TON)

EST. WT. OF STEEL SPAN

90' TPG (PHASE 1, ASSEMBLED) = 335,680 LB. (167.8 TON)

90' COMMON GIRDER = 82,655 LB. EA. (41.3 TON)

90' SIDE GIRDER = 83,080 LB. EA. (41.5 TON)

EST. WT. OF PRECAST CONCRETE

END CAP PEC-1L = 121,455 LB. EA. (60.8 TON)

END CAP PEC-1R = 121,455 LB. EA. (60.8 TON)

END CAP PEC-2L = 77,910 LB. EA. (39.0 TON)

END CAP PEC-2R = 77,910 LB. EA. (39.0 TON)

WINGWALL PW-10 = 11,955 LB. EA. (6.0 TON)

NO.	DATE	REVISIONS
COMPLETION STATUS:	05/28/2021	DATE
FINAL	STATUS	
APPROVED FOR UNION PACIFIC RAILROAD BY:	MATTHEW BECKER	05/28/2021
CONSULTANT ENGINEER	DATE	
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION	LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W



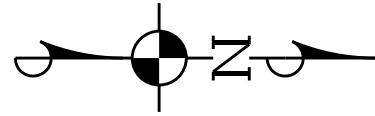
APPROVED FOR UNION PACIFIC RAILROAD BY:

MATTHEW BECKER
CONSULTANT ENGINEER

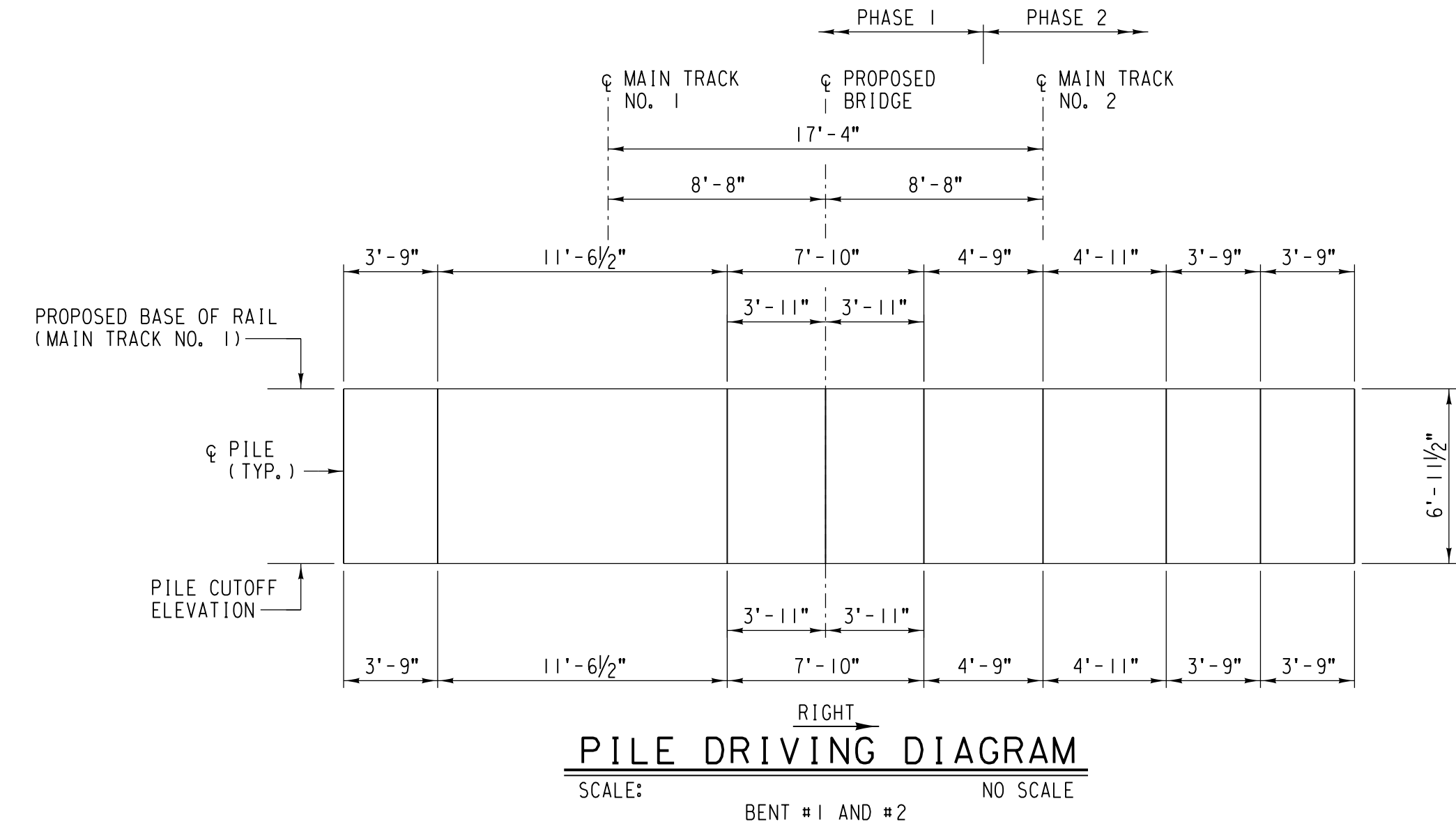
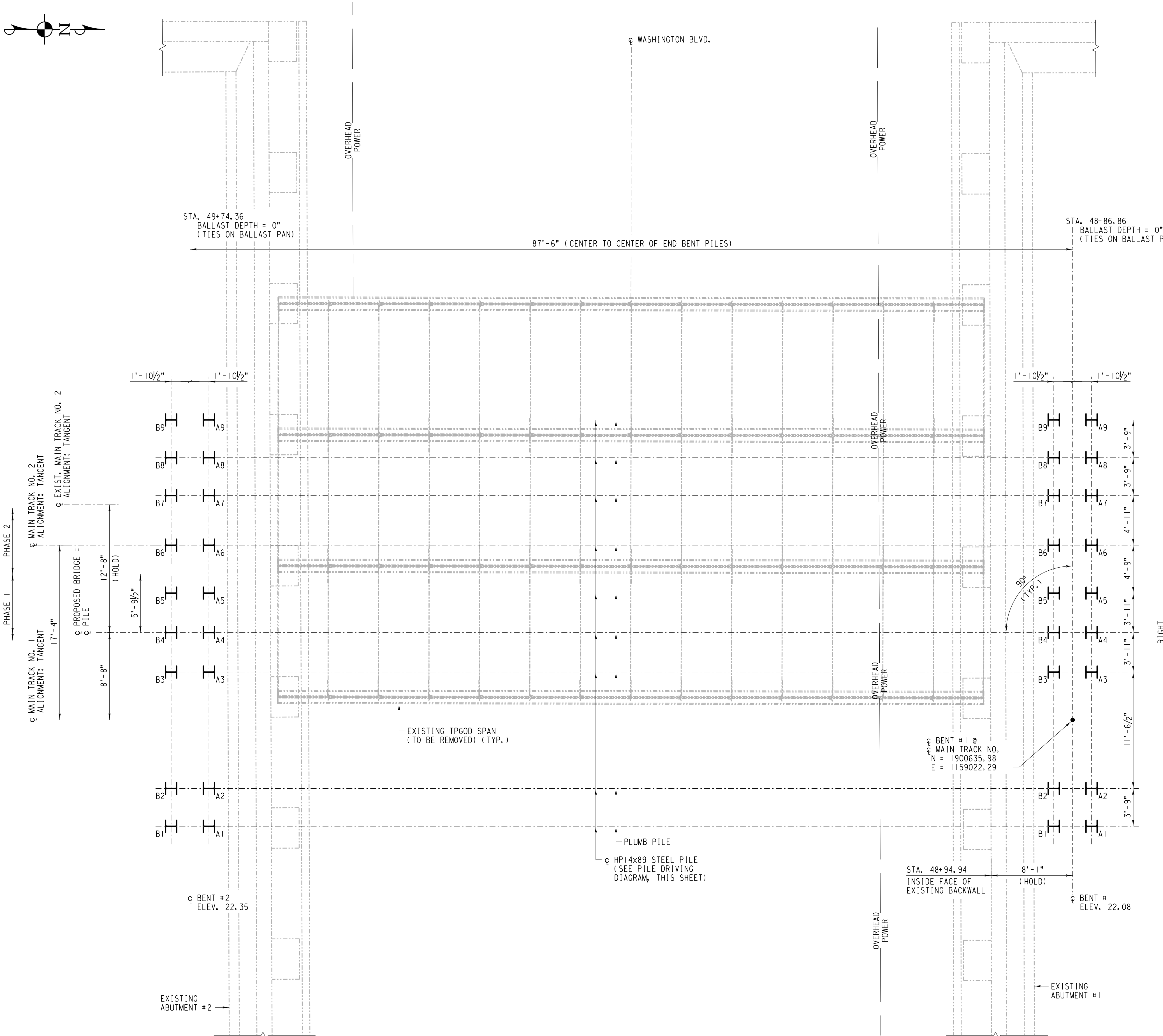
PROJECT ID: WORK ORDER: 31876 C/E NUMBER: 122525

UNION PACIFIC	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design
	DRAWN/CHK BY: ACB/MFB	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION
	UPRR ENGINEER: DEH/ADS	1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)
	SHT NO.: E1 of E43	SHEET TITLE: GENERAL ARRANGEMENT

TO CANAL ST. (CHICAGO)
(TIMETABLE SOUTH)



TO KEDZIE (CHICAGO)
(TIMETABLE NORTH)



PILE LAYOUT
SCALE: 3/8" = 1'-0"
AT PILE CUTOFF

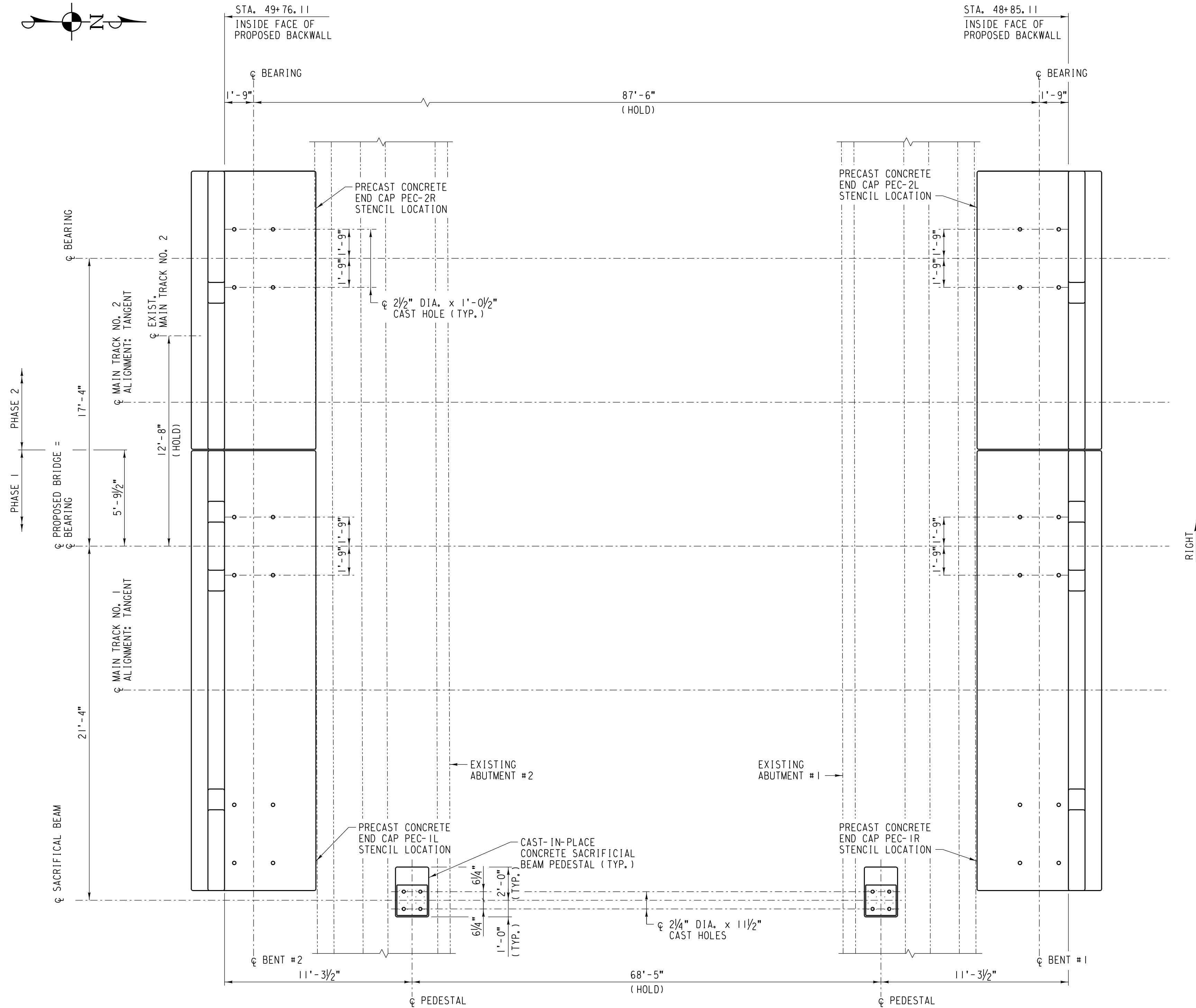
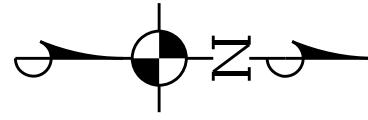
EST. PILE TIP DATA	
BENT	ELEV.
#1	-52.00 CCD
#2	-41.00 CCD

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DESIGNED BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHECK BY:	Office of Director Structures Design	
	ACB/MFB		
UPRR ENGINEER:		LOCATION & DESCRIPTION:	
DEH/ADS		BRIDGE 0.93 ROCKWELL SUBDIVISION	
SHT NO:		1 SPAN TPG x 90'	
E2 of E43		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
		SHEET TITLE:	
		PILE LAYOUT AND PILE DRIVING DIAGRAM	

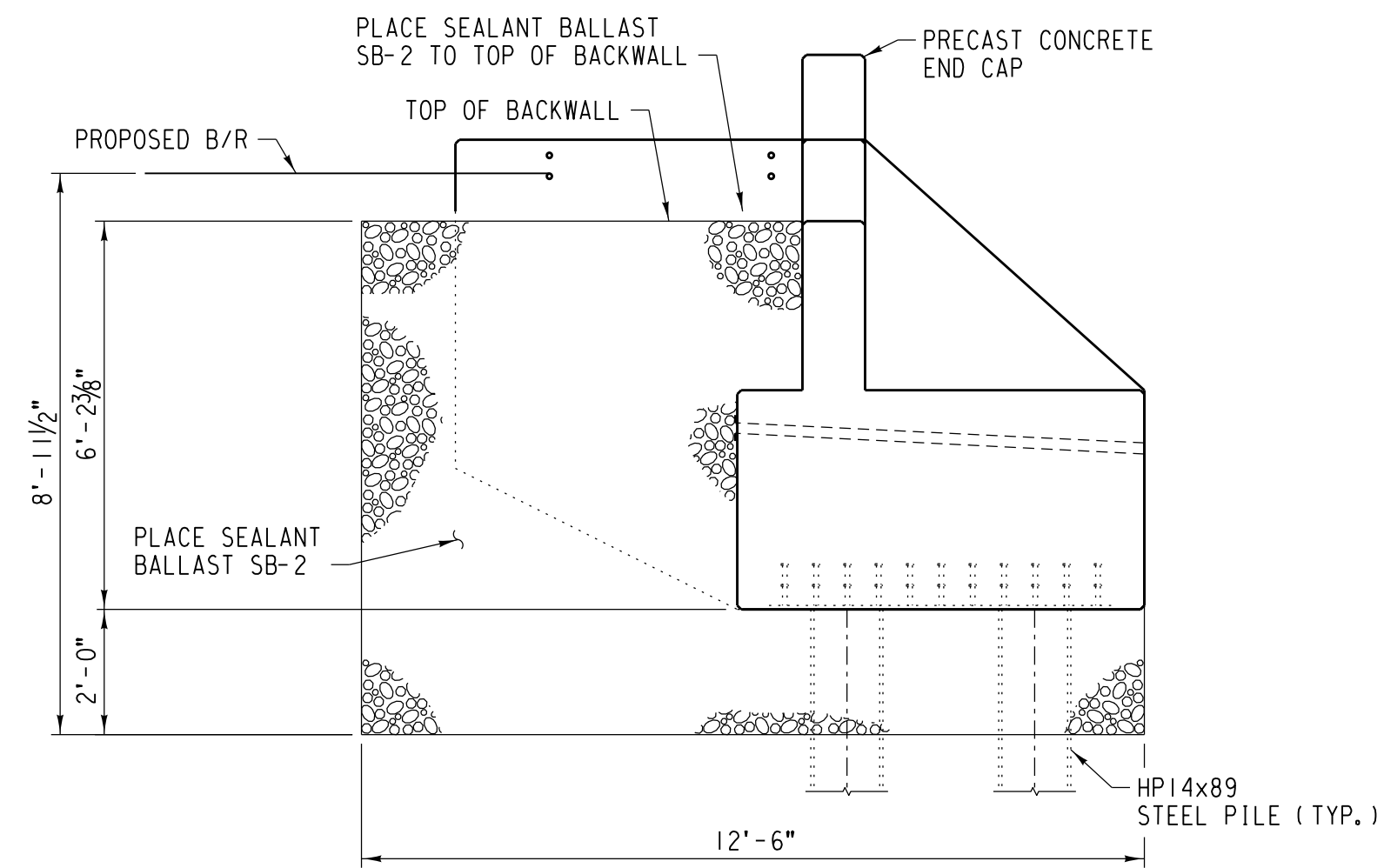
TO CANAL ST. (CHICAGO)
(TIMETABLE SOUTH)

TO KEDZIE (CHICAGO)
(TIMETABLE NORTH)



SUBSTRUCTURE LAYOUT

SCALE: 1/4"=1'-0"



END BENT BACKFILL DETAIL

SCALE: 3/8"=1'-0"
END HANDRAIL ASSEMBLY AND TPG
SPAN NOT SHOWN FOR CLARITY

NOTE:
FOR CONSTRUCTION SEQUENCE, SEE SHEET NO. 4.

NO.	DATE	REVISIONS
1	05/28/2021	FINAL
2	05/28/2021	FINAL
3	05/28/2021	FINAL
4	05/28/2021	FINAL
5	05/28/2021	FINAL
6	05/28/2021	FINAL
7	05/28/2021	FINAL
8	05/28/2021	FINAL
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97	05/28/2021	FINAL
98	05/28/2021	FINAL
99	05/28/2021	FINAL
100	05/28/2021	FINAL

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION

DESIGNED BY:
FNF/MFB

DRAWN/CHK BY:
ACB/MFB

UPRR ENGINEER:
DEH/ADS

SHT NO.:
E3 of E43

UNION PACIFIC RAILROAD

Office of Director Structures Design

LOCATION & DESCRIPTION:
BRIDGE 0.93 ROCKWELL SUBDIVISION
1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

SHEET TITLE:
SUBSTRUCTURE LAYOUT

LATITUDE: 41.88670°N
LONGITUDE: 87.69144°W

PROJECT ID:
31876

WORK ORDER:
122525

C/E NUMBER:
122525

DESIGN NOTES

- 1. In the event of a conflict between the design plans and the standards, the design plans shall control.
- 2. Location of known utilities is approximate. Location shall be verified prior to construction. Notify DIGGER/811 CHICAGO, 1-312-744-7000, at least 48 hours prior to construction.

RIGHT-OF-WAY

- 1. Approx. 60'-0" east of existing Main Track No. 1 centerline.
- 2. Approx. 47'-0" west of existing Main Track No. 2 centerline.

LAYOUT

- 1. Stationing: Sta. 48+94.94, south face of north abutment backwall of existing Bridge No. 0.93.
- 2. Elevation Datum: Chicago City Datum (CCD)
- 3. Benchmark: BM #276, Elev. 14.66 (CCD), 6.4 feet north of south line of W Congress Parkway and 47.1 feet east of east line of S Washtenaw Avenue.
- 4. Profile: Increase in rail elevation varies, 5" to 8". Rail Size: 136# (Main Track No. 1), 136# (Main Track No. 2)
- 5. Alignment: Tangent.
- 6. Information used to prepare this drawing in addition to reference drawings:
 - Survey conducted by AMERICAN SURVEYING & ENGINEERING, from 2012 to 2017.
 - Geotechnical Design Report prepared by WANG ENGINEERING, INC., dated 06/26/2020.

PILE DRIVING

- 1. All piles shall be driven to 123 ton service capacity.
- 2. Constructor shall assume one pile load test will be required in accordance with City of Chicago Department of Buildings. If pile load test is required, Constructor shall be performed in accordance with ASTM D1143 using Procedure A.

DESIGN

- 1. The proposed superstructure and substructure have been designed in accordance with the AREMA Manual for Railway Engineering, Chapter 8: Concrete Structures and Foundations and Chapter 15: Steel Structures, except longitudinal load, which is designed per the 1996 AREA Manual for Railway Engineering.
- 2. This superstructure was designed for Cooper E80 Live Load or alternate loading, where applicable, plus impact with a 30" maximum total depth of ballast. The substructure was similarly designed, but without impact (AREMA Chapter 8: Article 2.2.3.d.3).
- 3. This drawing was prepared using 0" (min.) of ballast under timber ties.

EXISTING MASONRY ABUTMENT RESTORATION NOTES

Existing Masonry Abutment Restoration consists of tuckpointing the joints and cleaning and painting the exposed surface of the existing masonry abutments. Limits include the width of railroad ROW, both abutments. All surfaces exposed to the roadway below should receive restoration treatment.

TUCKPOINTING

- 1. Loose, unsound or disintegrated mortar pointing between the masonry joints shall be removed and the joints cleaned and washed before pointing with new mortar. The exposed beds and joints shall be thoroughly cleaned by chipping with chisels or similar tools, scraping and brushing out the mortar to a minimum depth of 1-1/2 inches, and washing thoroughly with clean water.
- 2. Areas to be tuckpointed should be wet thoroughly to prevent absorption of water from the mortar.
- 3. Mortar shall be tuckd into the joints and finished with a smooth face struck flush with masonry face. All excess material shall be removed and the joint tooled to a neat workmanlike finish.
- 4. Tuckpointing mortar shall be FX 262 Repair Mortar as manufactured by Fox Industries or Commercial Grade FastSet Repair Mortar as manufactured Quikrete, or an approved equal. The mortar shall be applied according to the manufacturer's specifications.

CLEANING

- 1. Constructor shall use water cleaning to clean the existing abutment masonry surfaces of paint, heavy soiling, staining, organic and inorganic contaminants, and all loose, foreign material to teh satisfaction of the Engineer. Cleaning operations shall consist of pressure spraying, hand scrubbing and rinsing. Cleaning operations shall be performed after all tuckpointing repairs are performed to the existing abutments.
- 2. Constructor shall provide water cleaning equipment including a trailer-mounted water tank, pumps, high-pressure hose, wand with safety release cutoff control, nozzle, and auxiliary water re-supply equipment. Water used for cleaning shall be potable and obtained from a local source. Water shall also be filtered to remove minerals resulting in a neutral pH prior to its use.
- 3. Hand scrubbing of masonry shall be performed with natural- or nylon-bristled, hand held brushes. Wire brushes shall not be used.
- 4. Protective covers and barriers shall be provided as required to prevent over-spray onto adjacent surfaces or new steel superstructure and bearings.
- 5. Pressure spraying and rinsing equipment shall be capable of a discharge capacity of 55 to 400 psi and 2.5 to 3.0 gallons per minute for general surface cleaning operations. The water tank and auxiliary re-supply equipment shall be of sufficient capacity to permit continuous operations. Equipment shall not be operated at a pressure which will cause etching or other damage to the masonry surface or mortar joints.
- 6. Cleaning operations shall commence when the air temperature is above, and expected to remain above, 40 degrees F for not less than 7 consecutive days.
- 7. Adjust cleaning operations as required to ensure masonry and mortar joints are not damaged and an accesptable process is obtained.

PAINTING

- 1. Apply paint to all existing and new abutment surfaces in accordance with paint manufacturer's specifications.
- 2. Constructor shall coordinate paint system type, color and sheen with the CDOT - Division of Engineering.

PROPOSED CONSTRUCTION SEQUENCE

- 1. All work to be performed by Contractor, except where noted in parentheses. Both tracks shall remain in service except for natural windows and/or prearranged work blocks. Contractor and Railroad shall define Existing Main Track No. 1 and Track No. 2 extended work block date and duration. Extended work block shall be prearranged during one weekend in each construction phase. Work shall generally include, but is not limited to:

PHASE 1

- 1. Existing Main Track No. 1 and No. 2 to remain in service except during extended work block.
- 2. During work windows, install temporary shoring between Phase 1 and Phase 2 and drive pile for Phase 1 construction (immediately remove pile to B/R).
- 3. Close traffic service on Existing Main Track No. 1 for extended work block (Railroad).
- 4. Remove existing rail, ties, ballast and OTM (Railroad).
- 5. Remove existing steel span (eastern girder with floor system), bearings, riser blocks and excavate as required to remove existing backwall on both abutments for installation of new bridge for Proposed Main Track No. 1.
- 6. Excavate as required and remove driven pile to final cutoff elevations.
- 7. Install precast concrete end caps and wingwalls.
- 8. Install Phase 1 portion of TPG span.
- 9. Install portion of abutment backfill and subballast.
- 10. Place ties, rail and OTM (Railroad).
- 11. Place Main Track No. 1 back in service on proposed alignment (Railroad).
- 12. Construct cast-in-place pedestals for sacrificial beam.
- 13. Install sacrificial beam.

PHASE 2

- 1. Main Track No. 1 and Existing Main Track No. 2 to remain in service except during extended work block.
- 2. During work windows, drive pile for Phase 2 construction (immediately remove pile to B/R).
- 3. Close traffic service on Existing Main Track No. 2 for extended work block (Railroad).
- 4. Remove existing rail, ties, ballast and OTM (Railroad).
- 5. Remove remaining existing riser blocks and backwall on both abutments to remove existing backwall on both abutments as required for installation of new bridge for Proposed Main Track No. 2.
- 6. Excavate as required and remove driven pile to final cutoff elevations.
- 7. Install remaining precast concrete end caps and wingwalls.
- 8. Install Phase 2 portion of TPG span for Proposed Main Track No. 2.
- 9. Install remaining portion of abutment backfill and subballast.
- 10. Remove temporary shoring.
- 11. Place ties, rail and OTM (Railroad).
- 12. Place Main Track No. 2 back in service on proposed alignment (Railroad).
- 13. Remove remaining existing riser blocks and backwall on both abutments and excavate as required for installation of new bridge for Access Road.
- 14. Construct cast-in-place concrete bridge seat for proposed Access Road Bridge.
- 15. Install 33" x 36" PPC Deck Beams.
- 16. Construct cast-in-place backwalls and wingwalls for proposed Access Road Bridge.
- 17. Install remaining portion of abutment backfill as required.
- 18. Install cast-in-place concrete parapet on Access Road Bridge.
- 19. Install waterproofing and aggregate surface course on Access Road Bridge.
- 20. Restore area to original condition or better.

BILL OF MATERIAL						
TOTAL	PHASE 1	PHASE 2	UNIT	DESCRIPTION	STORE ITEM NO.	ORDERED BY
1	1	-	EA.	STRUCTURAL STEEL AND FASTENERS, INCLUDING BEARING MATERIAL, FOR ONE 90'-0" TPG STEEL SPAN, COMPLETE (PER NOTES, SHEET NO. E25, DETAILS, SHEET NOS. E21 THRU E36, BEARINGS, SHEET NOS. E38 THRU E42 AND MATERIAL LISTS, SHEET NO. E37)	122525-1	MANAGER TRACK PROJECTS
1	1	-	EA.	PRECAST CONCRETE END CAP PEC-1R (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E9)	122525-2	
1	1	-	EA.	PRECAST CONCRETE END CAP PEC-1L (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E10)	122525-3	
1	-	1	EA.	PRECAST CONCRETE END CAP PEC-2R (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E11)	122525-4	
1	-	1	EA.	PRECAST CONCRETE END CAP PEC-2L (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E12)	122525-5	
4	2	2	EA.	PRECAST WINGWALL PWM-10 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E13)	122525-6	
1	1	-	LOT	NON-STANDARD MISCELLANEOUS STEEL (PER SCHEDULE, SHEET E7)	122525-7	
4	2	2	EA.	END HANDRAIL ASSEMBLY CEH-10 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E43)	122525-8	
2	2	-	EA.	BACKWALL HANDRAIL SBH-1 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E43)	122525-9	
2	-	2	EA.	BACKWALL HANDRAIL SBH-2 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E43)	122525-10	
1	1	-	LOT	4,000 PSI CONCRETE FOR SACRIFICIAL BEAM PEDESTALS (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E7)	122525-11	
1	1	-	LOT	REINFORCING STEEL FOR SACRIFICIAL BEAM PEDESTALS (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E7)	122525-12	
1	1	-	LOT	EMBED PLATE EB-10 FOR SACRIFICIAL BEAM PEDESTALS (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAILS, SHEET NO. E7)	122525-13	
54	30	24	EA.	HP14x89x 40'-0" STEEL PILE (ASTM A572 GRADE 50, PLAIN)	510-7557	
18	10	8	EA.	HP14x89x 50'-0" STEEL PILE (ASTM A572 GRADE 50, PLAIN)	510-7565	
36	20	16	EA.	HP14 x 89 PILE POINTS (PER NOTES & DETAIL, STD. DWG. 531110 SHT. H1)	510-8063	
36	20	16	EA.	HP14 x 89 PILE SPLICER (PER NOTES & DETAIL, STD. DWG. 531110 SHT. H1)	510-8065	
1	-	1	EA.	C10x15.3x 20'-0" STEEL BRACING (ASTM A572 GR. 50, PLAIN)	247-6649	
2	1	1	QT.	ZRC COLD GALVANIZING COMPOUND OR APPROVED ALTERNATE	513-3960	
1	-	1	EA.	BRIDGE MARKER AND NO TRESPASSING SIGNS, MOUNTING POST AND MOUNTING HARDWARE (PER STD. DWG. 0502, 0507, 0538 AND 0599)	P00-2616	
54.4	-	54.4	CU. YD.	4000 PSI CONCRETE, 28.3 CU.YD. FOR ACCESS BRIDGE ABUTMENTS, 25.5 CU.YD. FOR ACCESS BRIDGE PARAPETS AND 0.6 CU.YD. FOR ACCESS BRIDGE EXPANSION JOINTS (PER PROJECT SPECIFICATIONS AND PER DETAILS, SHEET NOS. E14, E16 AND E17)		CONSTRUCTOR
1	-	1	LOT	REINFORCING STEEL FOR ACCESS BRIDGE ABUTMENTS, ACCESS BRIDGE PARAPETS AND ACCESS BRIDGE EXPANSION JOINTS (PER PROJECT SPECIFICATIONS AND PER NOTES AND SCHEDULES, SHEET NOS. E14, E16 AND E17)		
1	-	1	LOT	DRILL AND EPOXY GROUT BARS FOR ACCESS BRIDGE ABUTMENTS (PER PROJECT SPECIFICATIONS AND PER NOTES AND DETAILS, SHEET NO. E14)		
1	-	1	LOT	LIQUID SPRAY-ON-WATERPROOFING FOR ACCESS BRIDGE SPAN (PER PROJECT SPECIFICATIONS AND PER NOTES AND SCHEDULE, SHEET NO. E15)		
1	-	1	LOT	AGGREGATE SURFACE COURSE FOR ACCESS BRIDGE SPAN (PER PROJECT SPECIFICATIONS AND PER NOTES AND SCHEDULE, SHEET NO. E15)		
1	-	1	LOT	PREFORMED JOINT STRIP SEAL FOR ACCESS BRIDGE SPAN (PER PROJECT SPECIFICATIONS AND PER NOTES AND SCHEDULE, SHEET NO. E17)		
1	-	1	LOT	PRECAST PRESTRESSED CONCRETE DECK BEAMS FOR ACCESS BRIDGE SPAN (PER PROJECT SPECIFICATIONS AND PER NOTES, DETAILS AND SCHEDULE SHEET NOS. E18 AND E19)		
1	-	1	LOT	STRUCTURAL STEEL FOR APRON PLATE AND SIDE PLATES (PER PROJECT SPECIFICATIONS AND PER NOTES, DETAILS AND SCHEDULE SHEET NO. E20)		
1	-	1	LOT	EXISTING MASONRY ABUTMENT RESTORATION (PER NOTES, THIS SHEET)		
28	14	14	GAL	CARLISLE CCW-201 POLYURETHANE SEALANT (PER NOTES, SHEET NO. E22)		
2	1	1	LOT	NON-SHRINK CEMENTITIOUS GROUT FOR TPG BEARING ANCHOR RODS, SACRIFICIAL BEAM ANCHOR RODS AND ACCESS BRIDGE DOWEL RODS AND ANCHOR RODS		
368	187	181	TON	SEALANT BALLAST (PER STANDARD DRAWING 0010E AND DETAIL, PER SHEET NOS. E3 AND E14)	562-5428	
1	1	-	LOT	TEMPORARY SHORING AS REQUIRED (PER NOTES, STD. DWG. 531100 SHT. T2)		

EST. WT. OF STRUCTURAL STEEL (NOT INCL. BOLTS) = 626,000 LB.
EST. WT. OF NON-STANDARD STRUCTURAL STEEL (SACRIFICIAL BEAM) = 7,963 LB.
EST. WT. OF NON-STANDARD STRUCTURAL STEEL (HANDRAILS) = 1,176 LB.
EST. WT. OF STEEL PILING = 272,340 LB.

BULK MATERIAL QUANTITIES ARE ESTIMATED.

NOTE:
CONSTRUCTOR DESIGNATES MANAGER BRIDGE CONSTRUCTION IF CONSTRUCTION BY UPRR AND CONTRACTOR IF CONSTRUCTION BY CONTRACTOR.

NOTE:
ALL NOTES NOT PROVIDED IN THESE PLANS SHALL BE PER STD. DWG. 531100 SHT. T2.

/ /		
/ /		
NO.	DATE	
REVISIONS		
COMPLETION STATUS:		
FINAL	05/28/2021	
STATUS	DATE	
		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER	05/28/2021	
CONSULTANT ENGINEER	DATE	
PROJECT ID:	WORK ORDER:	C NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design	
	DRAWN/CHK BY: ACB/MFB		
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION	
	SHT NO.: E4 of E43	SHEET TITLE: GENERAL NOTES AND BILL OF MATERIAL	

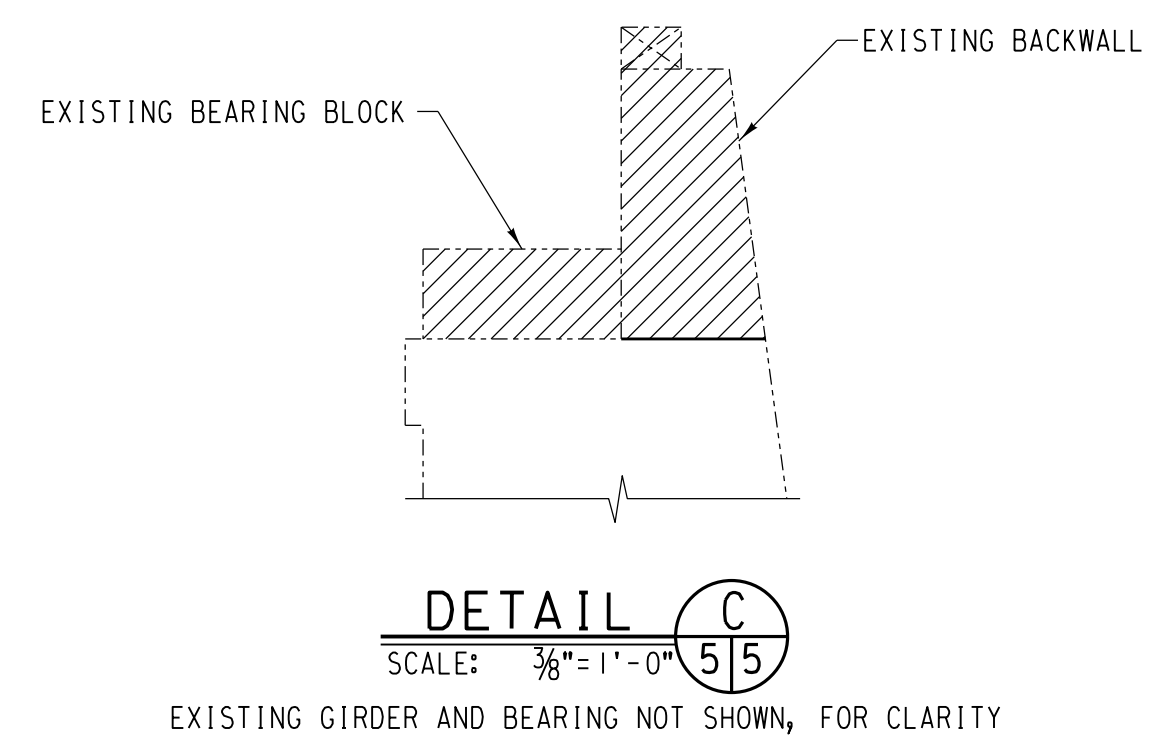
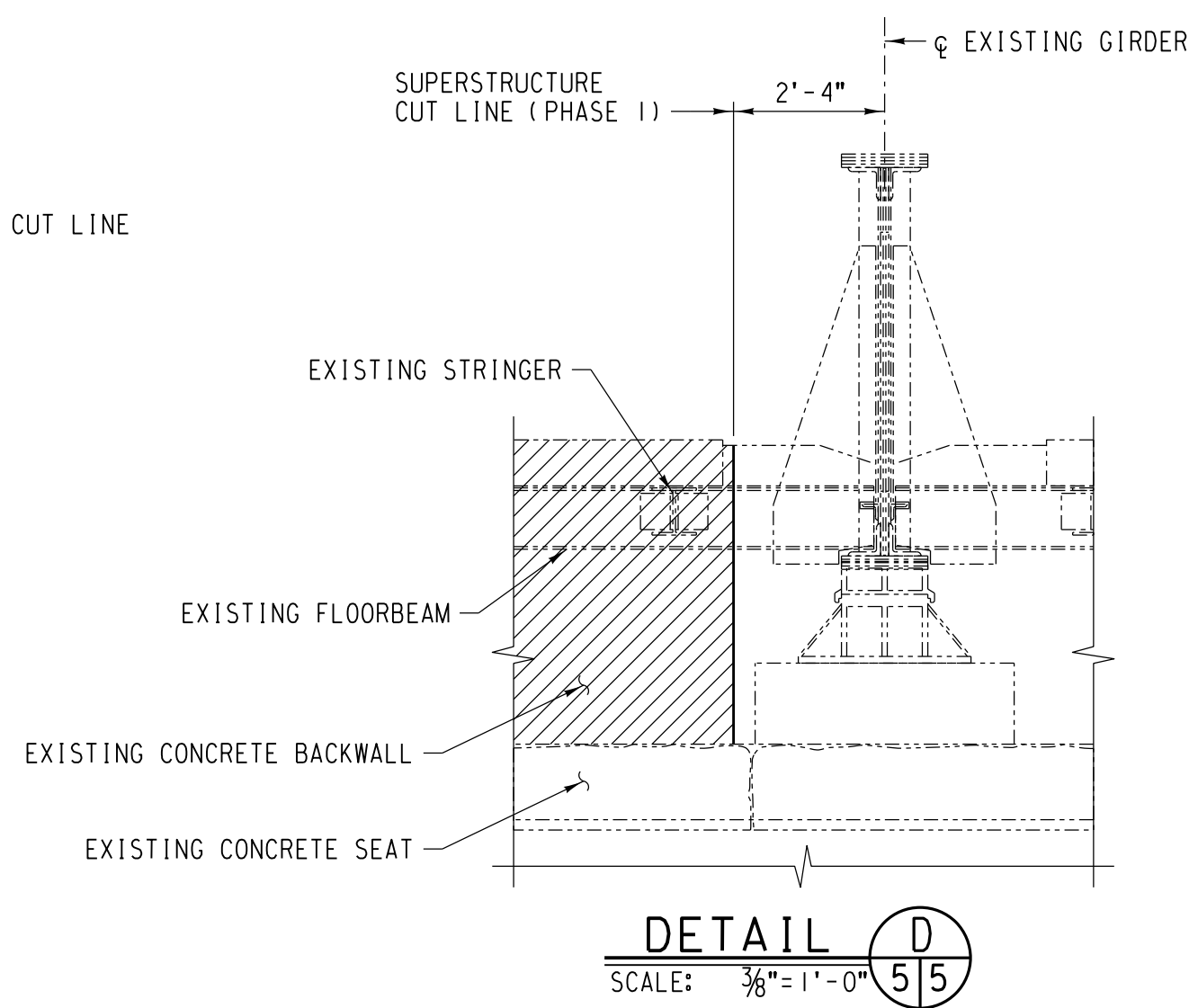
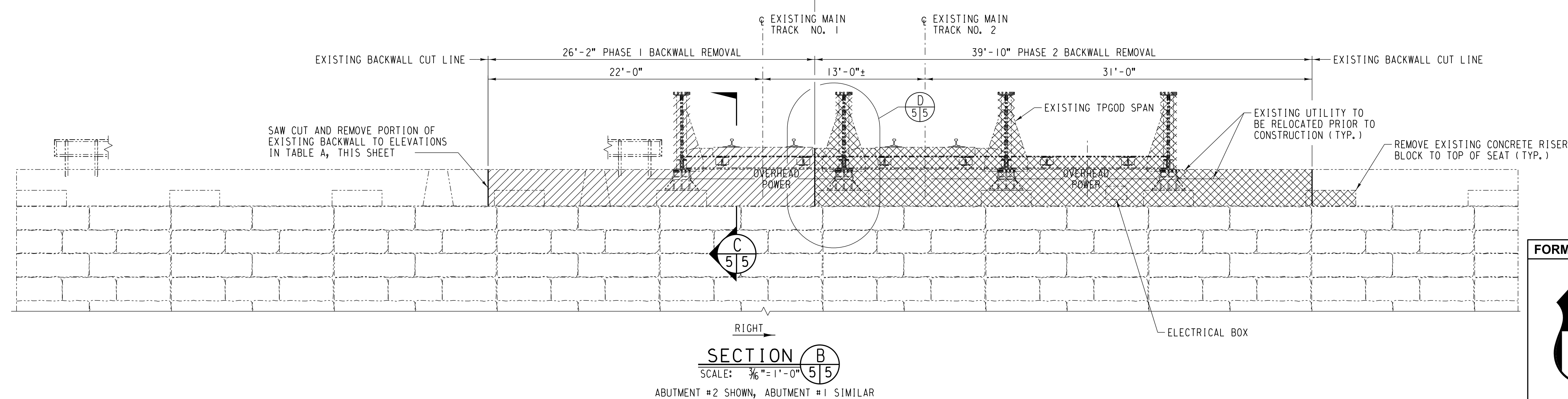
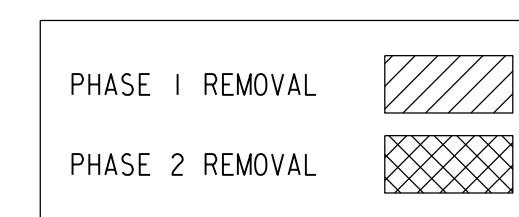


TABLE A	
LOCATION	REMOVAL ELEV.
UNDER PROP. SUPERSTRUCTURE	TOP OF EXIST. RISER BLOCKS
OUTSIDE PROP. SUPERSTRUCTURE	EXIST. MASONRY ABUT. SEAT

NOTES: 1. CONTRACTOR TO FIELD VERIFY EXISTING SUPERSTRUCTURE AND REQUIRED REMOVAL PRIOR TO START OF CONSTRUCTION. 2. CONTRACTOR TO VERIFY LIFTING WEIGHTS OF EXISTING TPG-BD SPANS AND METHOD OF REMOVAL PRIOR TO CONSTRUCTION

NOTE:
EXISTING SPAN ESTIMATED WEIGHT = 310,000 LB (STEEL ONLY)



	/ /	
	/ /	
NO.	DATE	REVISIONS
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FINAL		<u>05/28/2021</u>
STATUS		DATE
 benesch		
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MATTHEW BECKER		<u>05/28/2021</u>
CONSULTANT ENGINEER		DATE
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LATITUDE:	41.88670°N	LONGITUDE: 87.69144°W

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION



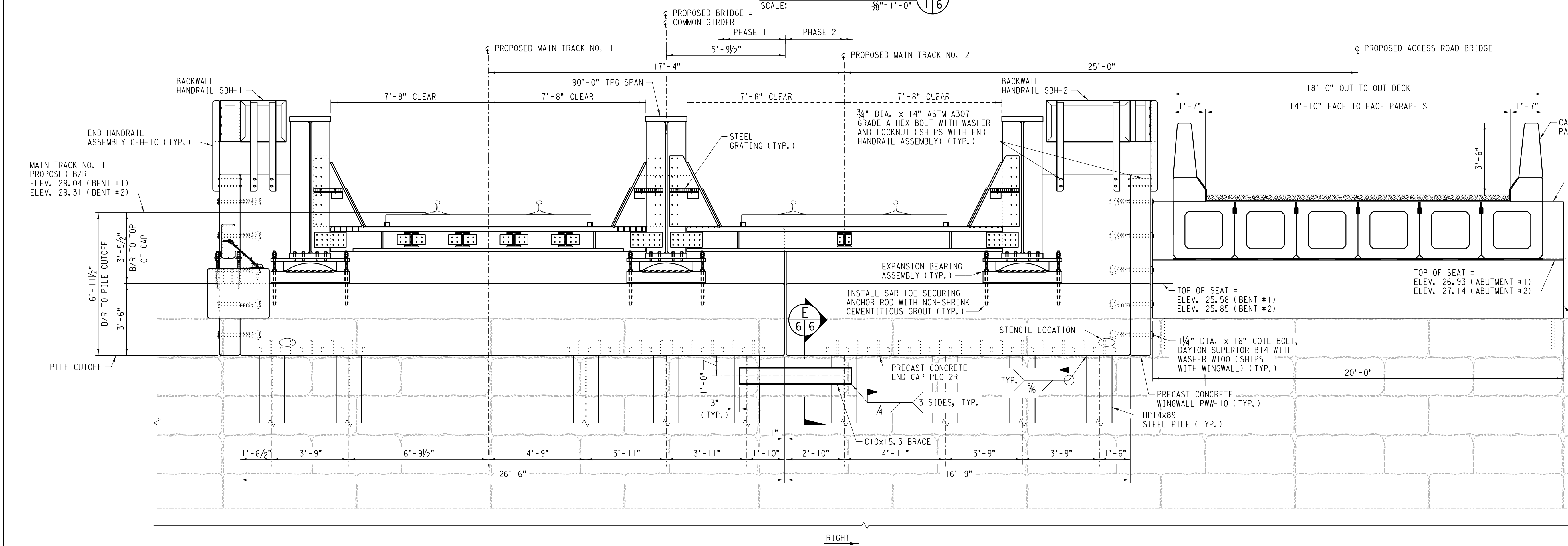
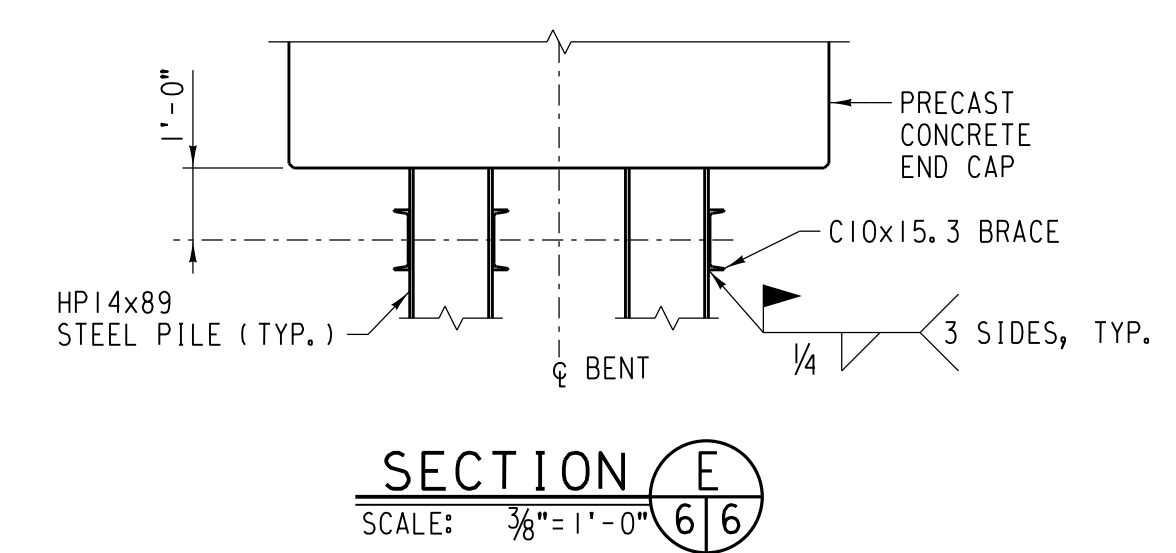
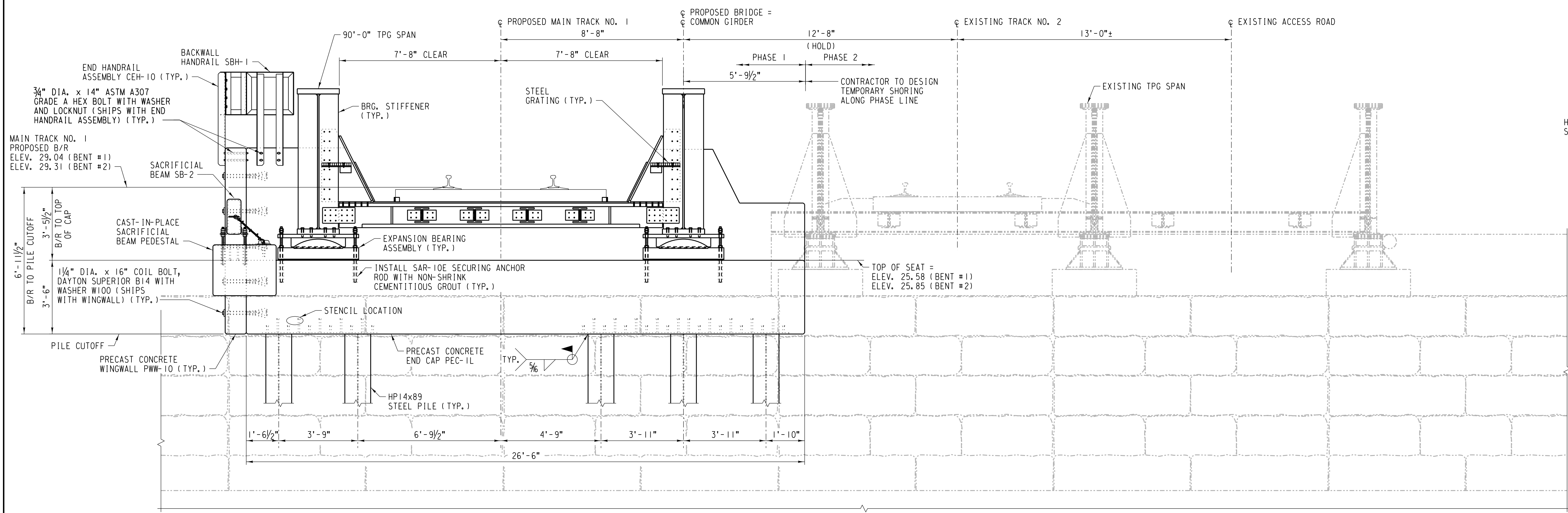
DSN/CHK BY:	FNF/MFB
DRAWN/CHK BY:	ACB/MFB
UPRR ENGINEER:	DEH/ADS
SHT NO.:	E5 of E4

UNION PACIFIC RAILROAD
Office of Director Structures Design

LOCATION & DESCRIPTION:	BRIDGE 0.93 ROCKWELL SUBDIVISION
	1 SPAN TPG x 90'
	REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

SHEET TITLE:	REMOVAL DETAILS
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FILE NAME: C:\user\mrc\mrc\proj\020093.dgn



NOTE:
FOR GRATING CONNECTION DETAILS,
SEE SHEET NO. E27 AND E28.

NO.	DATE	REVISIONS
1	05/28/2021	FINAL
2	05/28/2021	REVISION
3	05/28/2021	REVISION
4	05/28/2021	REVISION
5	05/28/2021	REVISION
6	05/28/2021	REVISION
7	05/28/2021	REVISION
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98	05/28/2021	REVISION
99	05/28/2021	REVISION
100	05/28/2021	REVISION

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION

DESIGNED BY:
FNF/MFB

DRAWN/CHECK BY:
ACB/MFB

UPRR ENGINEER:
DEH/ADS

SHT NO:
E6 of E43

UNION PACIFIC RAILROAD

Office of Director Structures Design

LOCATION & DESCRIPTION:
BRIDGE 0.93 ROCKWELL SUBDIVISION
1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

SHEET TITLE:
TYPICAL SECTIONS

LATITUDE: 41.88670°N
LONGITUDE: 87.69144°W

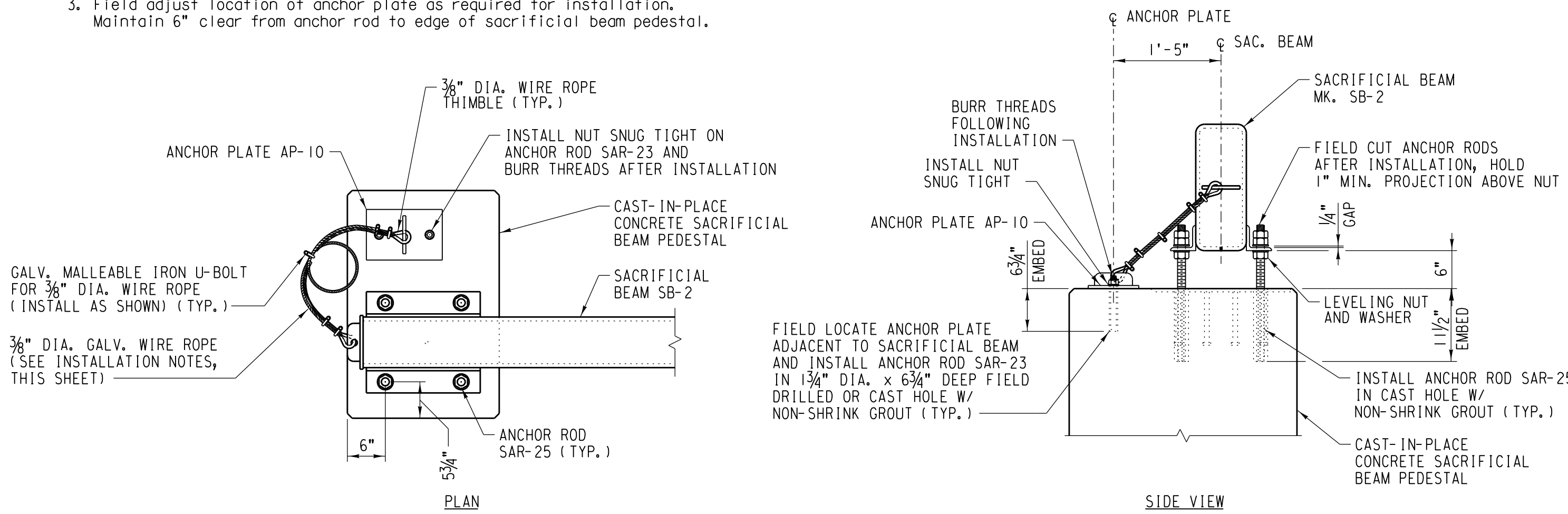
PROJECT ID:
31876

WORK ORDER:
122525

C/E NUMBER:
122525

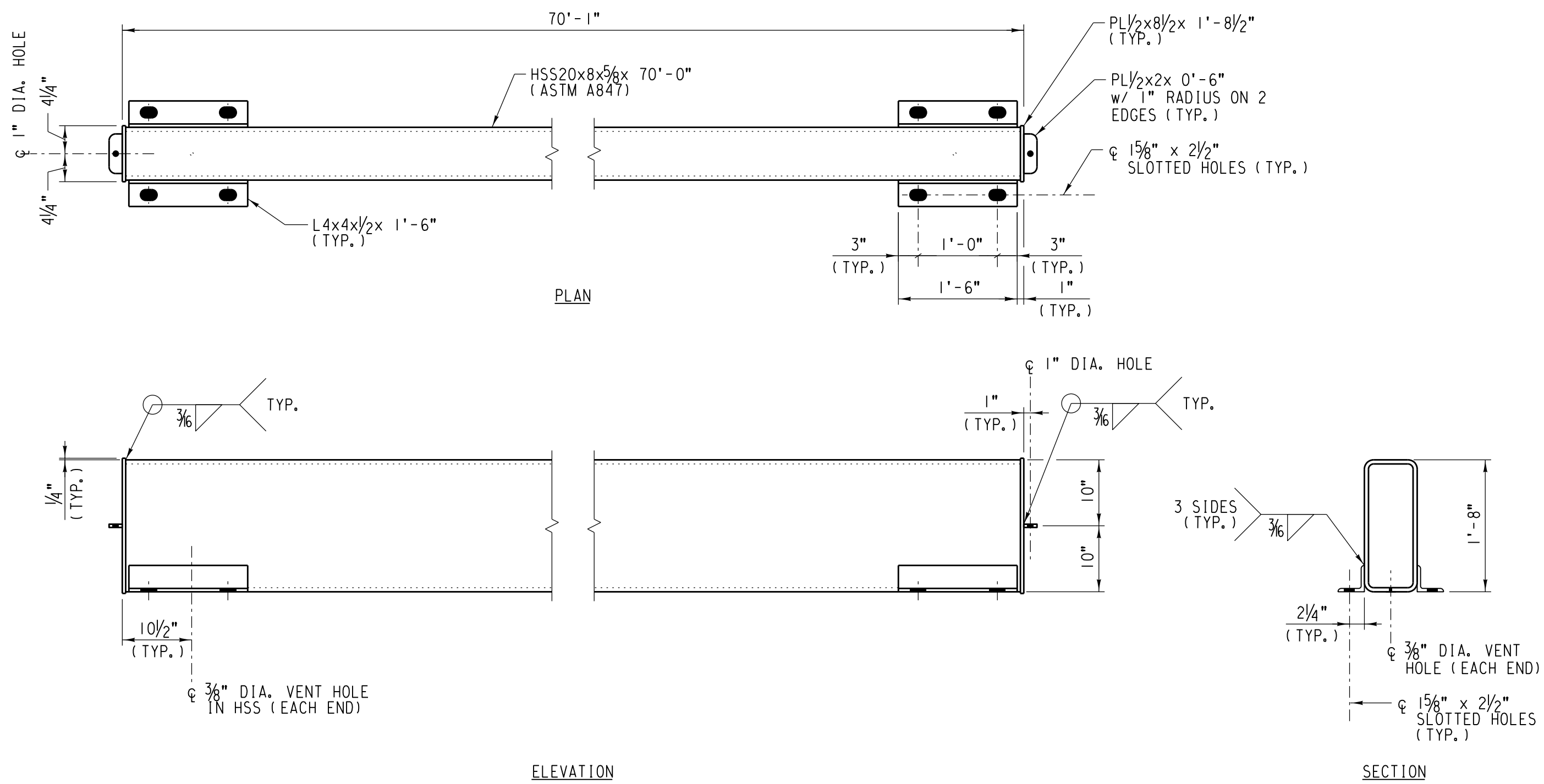
SACRIFICIAL BEAM INSTALLATION NOTES

1. Contractor is responsible for field verifying minimum vertical clearance from low chord to roadway at proposed bridge and match vertical clearance at sacrificial beam location.
2. Sacrificial beam shall be set level using leveling nuts as necessary.
3. Field adjust location of anchor plate as required for installation. Maintain 6" clear from anchor rod to edge of sacrificial beam pedestal.



SACRIFICIAL BEAM INSTALLATION DETAIL

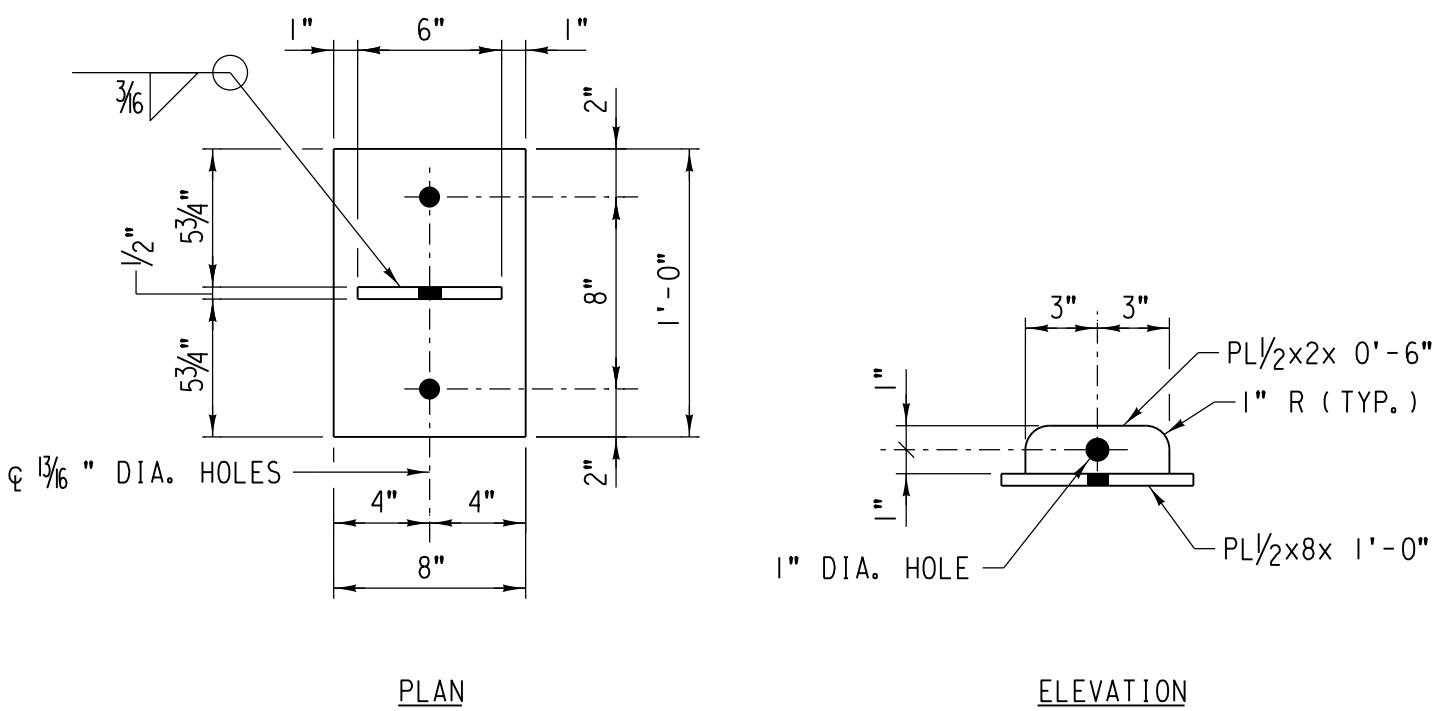
SCALE: 3/4"=1'-0"



SACRIFICIAL BEAM SB-2

SCALE: 3/4"=1'-0"

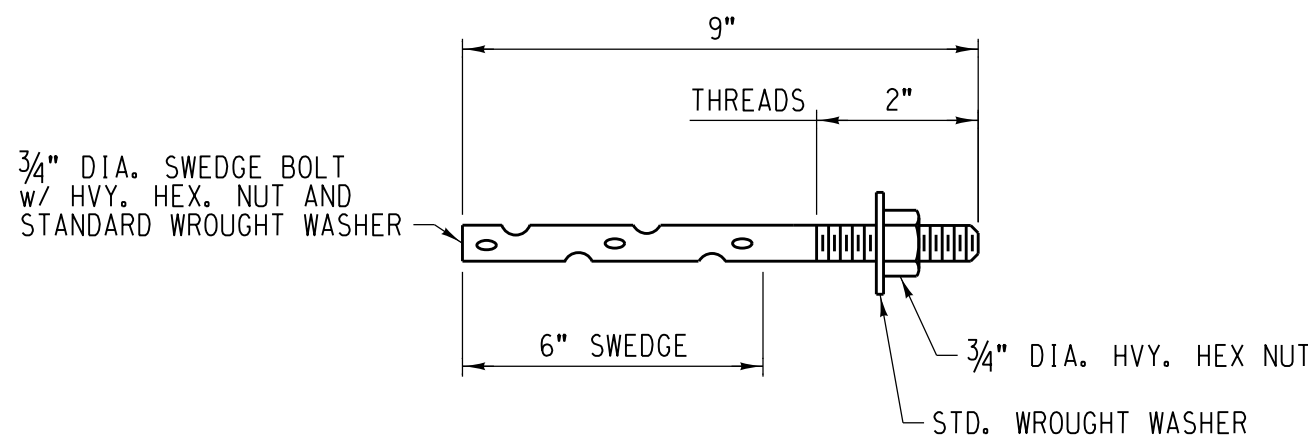
EST. WT. = 7,855 LB. EACH



ANCHOR PLATE AP-10

SCALE: 1 1/2\"/>

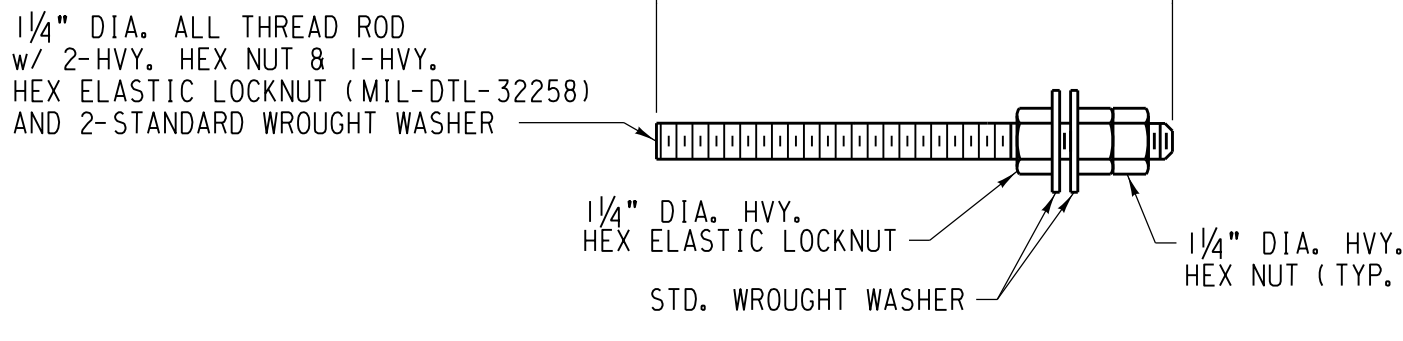
EST. WT. = 15.3 LB. EACH



ANCHOR ROD SAR-23

SCALE: NONE

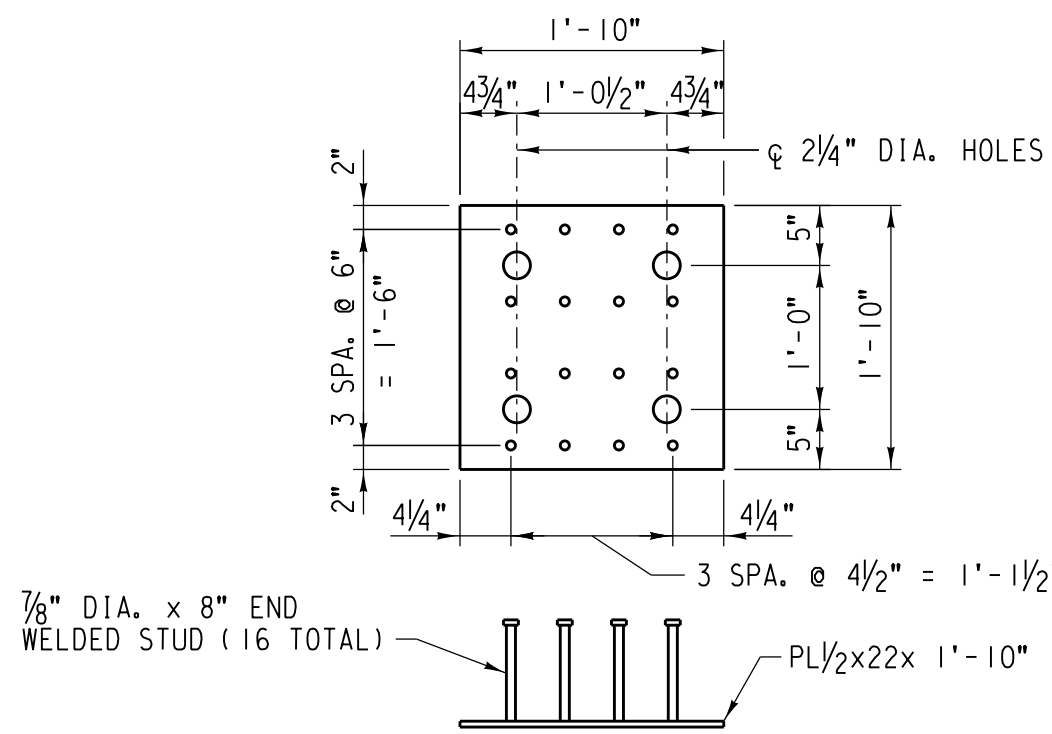
EST. WT. = 1.4 LB. EACH



ANCHOR ROD SAR-25

SCALE: NONE

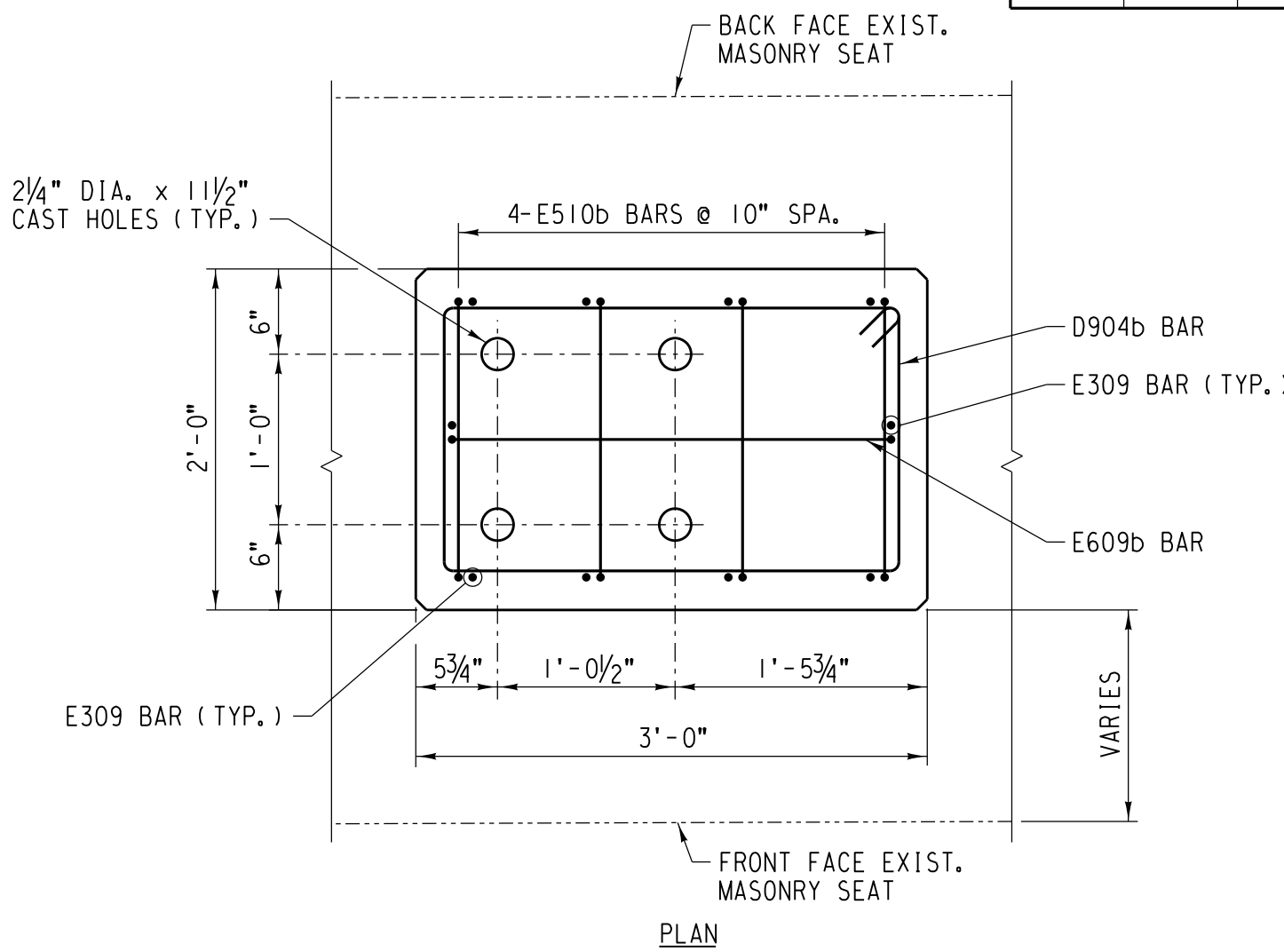
EST. WT. = 8.8 LB. EACH



EMBED PLATE EB-10

SCALE: 3/4\"/>

EST. WT. = 92.2 LB. EACH



CAST-IN PLACE PEDESTAL

SCALE: 1\"/>

NON-STANDARD MISCELLANEOUS STEEL SCHEDULE		
REQ'D.	UNIT	DESCRIPTION
2	EA.	ANCHOR PLATE AP-10 (PER NOTES, STD. DWG 531100 SHT. T3 AND DETAIL, THIS SHEET)
8	EA.	ANCHOR ROD SAR-25 (PER NOTES & DETAILS, THIS SHEET)
4	EA.	HEAVY PATTERN-HEAVY DUTY GALVANIZED CARBON STEEL WIRE ROPE THIMBLE FOR 3/8\"/>
1	EA.	SACRIFICIAL BEAM SB-2 (PER NOTES, STD. DWG 531100 SHT. T3 AND DETAIL, THIS SHEET)
4	EA.	ANCHOR ROD SAR-23 (PER NOTES & DETAILS, THIS SHEET)
20	LIN.FT.	3/8\"/>
10	EA.	GALV. MALLEABLE IRON U-BOLT w/ 2 ELASTIC LOCKNUTS (MIL-DTL-32258), ZINC PLATED, FOR 3/8\"/>

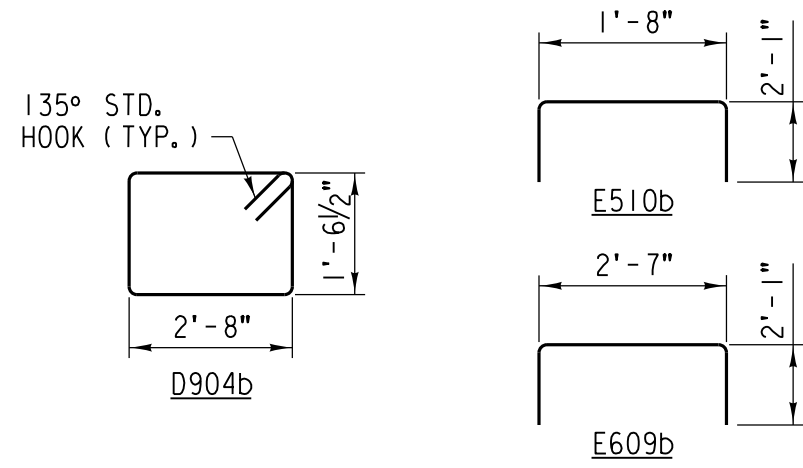
EST. WT. OF NON-STANDARD MISCELLANEOUS STEEL = 7,963 LB.

MATERIAL SCHEDULE (PER PEDESTAL)		
REQ'D.	UNIT	DESCRIPTION
0.6	CU.YD.	4,000 PSI CONCRETE (PER NOTES, STD. DWG 531100 SHT. T3)
1	LOT	REINFORCING STEEL (PER NOTES, STD. DWG 531100 SHT. T3 AND SCHEDULE, THIS SHEET)
1	EA.	EMBED PLATE EB-10 (PER NOTES, STD. DWG 531100 SHT. T3 AND DETAILS, THIS SHEET)

REINFORCING SCHEDULE (PER PEDESTAL)				
TOTAL	MARK	SIZE	LENGTH	SHAPE
3	D904b	#5	9'-4"	
10	E309	#6	3'-9"	
4	E510b	#6	5'-10"	
1	E609b	#6	6'-9"	

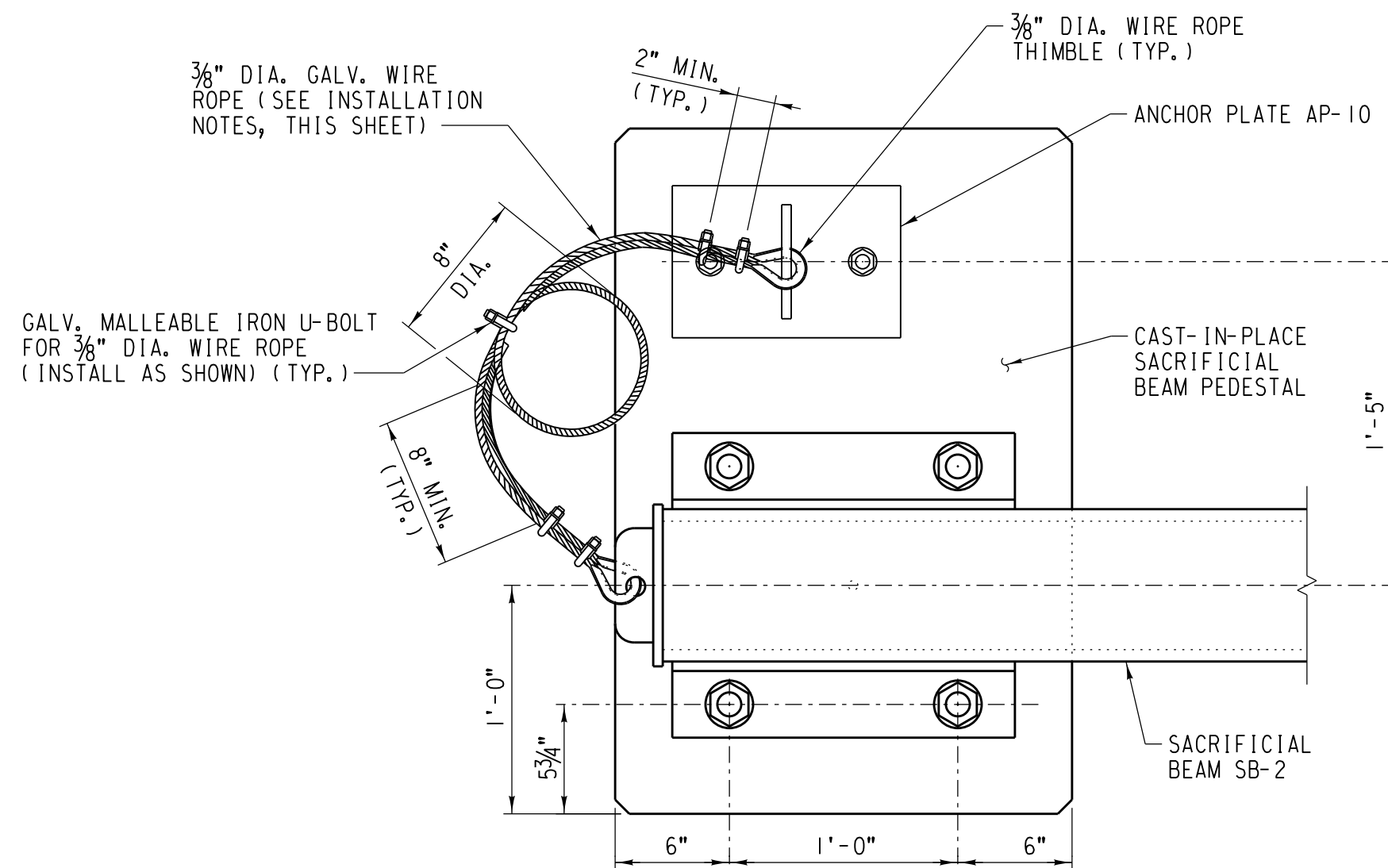
BENDING DIAGRAM

(DIMENSIONS ARE OUT TO OUT)



NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH BAR OWNED BY THE LETTER "B" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.

EST. WT. OF REINFORCING STEEL = 140 LB.



WIRE ROPE INSTALLATION DETAIL

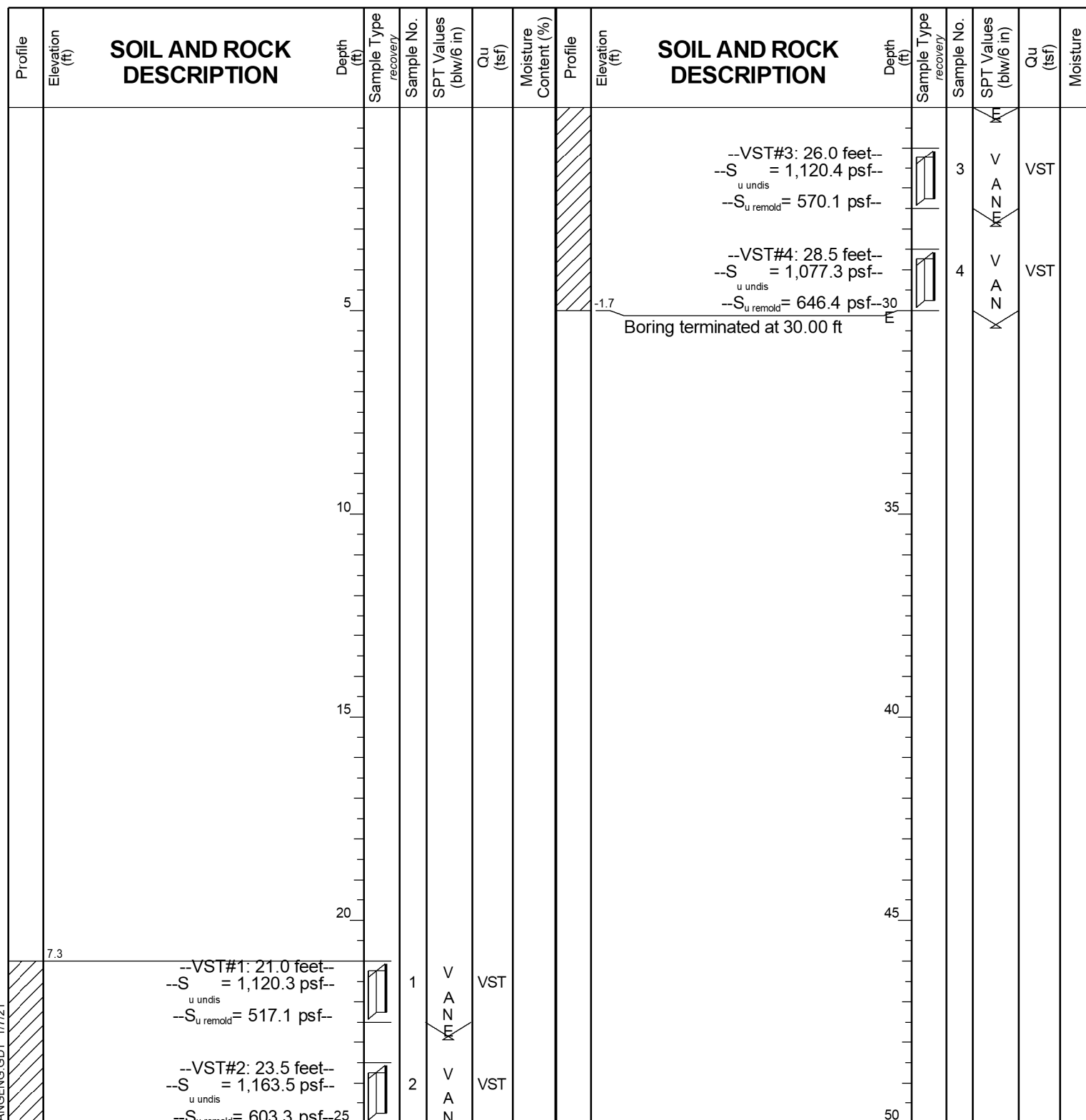
SCALE: 1 1/2\"/>

WIRE ROPE INSTALLATION INSTRUCTIONS FOR SACRIFICIAL BEAMS:

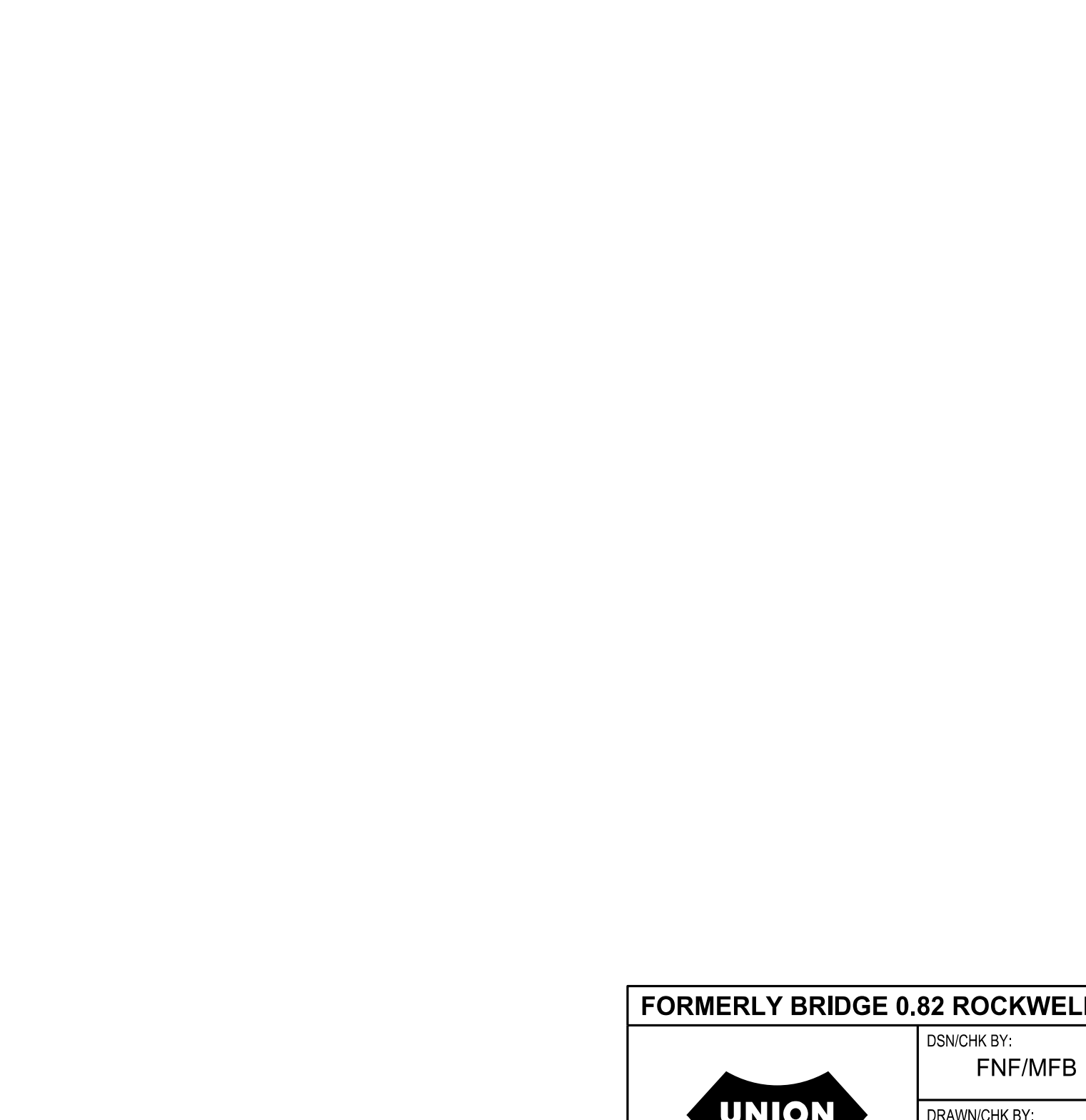
1. THREAD WIRE ROPE THROUGH PL 1/2x2x 0'-6" ON SACRIFICIAL BEAM AND FASTEN WITH U-BOLTS.
2. CREATE 8" DIA. LOOP IN WIRE ROPE, STRETCH WIRE ROPE, THREAD THROUGH HOLE IN ANCHOR PLATE, AND FASTEN. LEAVE CLIP LOOSE.
3. REMOVE ALL SAG BETWEEN CONNECTION POINTS TO 4-6 INCHES. TIGHTEN ALL U-BOLTS.

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSNCHK BY:	UNION PACIFIC RAILROAD Office of Director Structures Design	
	FNF/MFB		
	DRAWN/CHK BY:		
	ACB/MFB		
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGD x 70' (2 TRACKS)	
E7 of E43		SHEET TITLE:	
		SACRIFICIAL BEAM AND MISCELLANEOUS DETAILS	



GENERAL NOTES				WATER LEVEL DATA	
Begin Drilling	02-07-2017	Complete Drilling	02-08-2017	While Drilling	NA
Drilling Contractor	Wang Testing Services		Drill Rig	250 DMR [78%]	NA
Driller	RH&NC	Logger	F. Bozza	Checked by	J. Bensen
Drilling Method	3.25" IDA HSA; boring backfilled upon completion				
				Depth to Water	NA
				Use appropriate	footage to approximate boundary

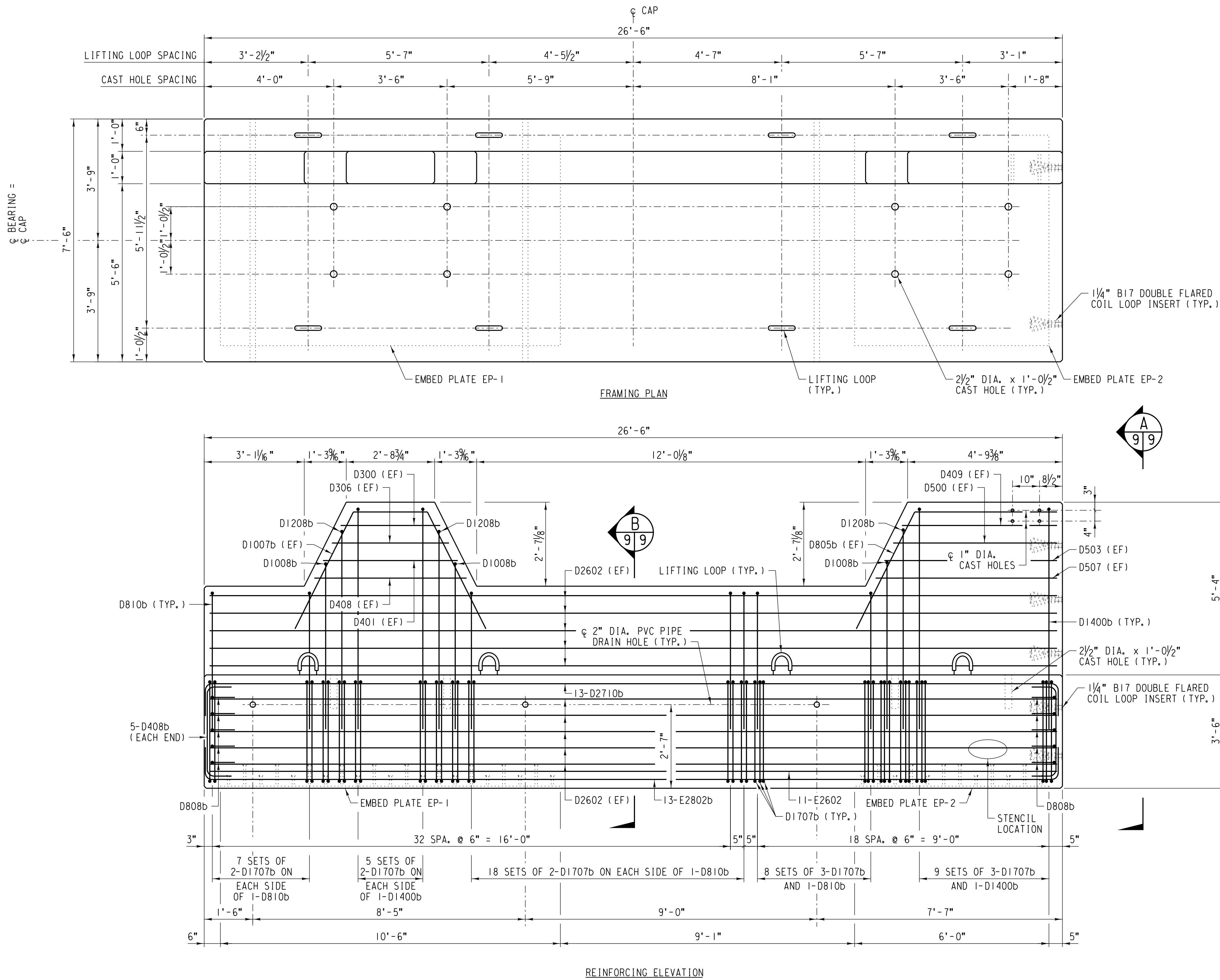


	ACB/MFB
	UPRR ENGINEER: DEH/ADS
	SHT NO.: 50 of 51

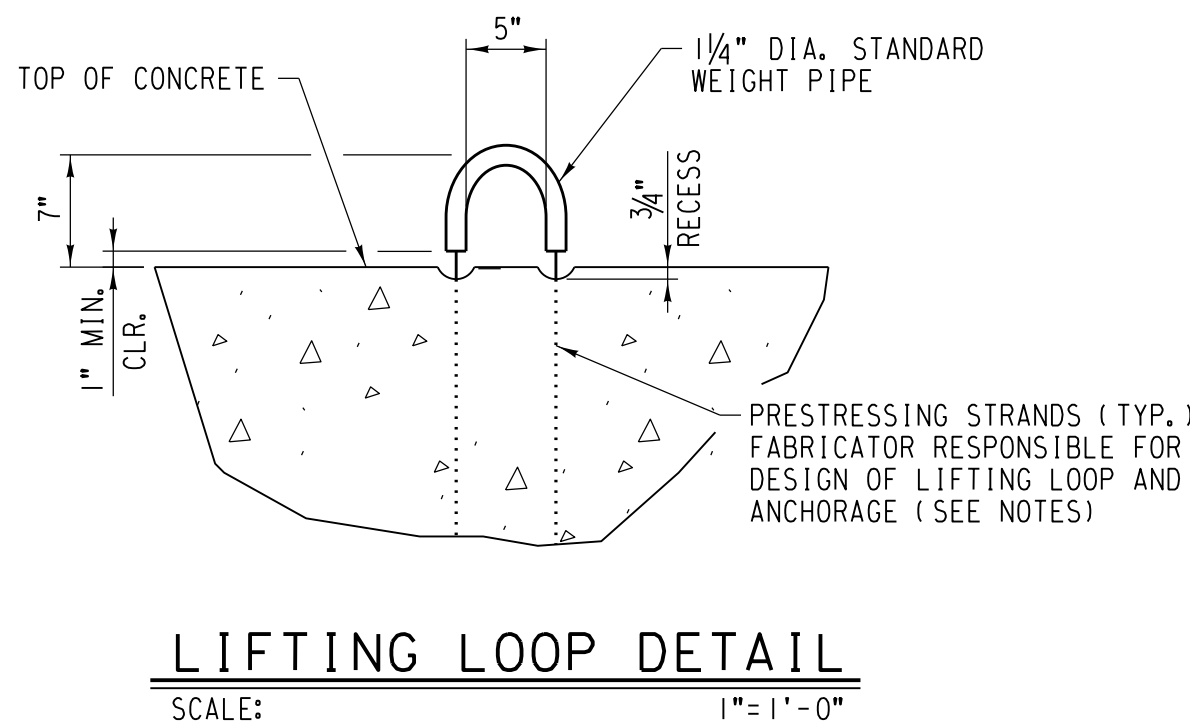
	/ /				
	/ /				
NO.	DATE	REVISIONS			
COMPLETION STATUS:					
FINAL					
STATUS		05/28/2021 DATE			
					
APPROVED FOR UNION PACIFIC RAILROAD BY:					
MATTHEW BECKER					
CONSULTANT ENGINEER		05/28/2021 DATE			
PROJECT ORDER:		WORK ORDER:		C/E NUMBER:	
		31876		122525	

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSNCHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWNCHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG @ 90' REPLACING 1 SPAN TPGOD @ 70' (2 TRACKS)			
	SHT NO.: E8 of E43	SHEET TITLE: BORING LOGS			

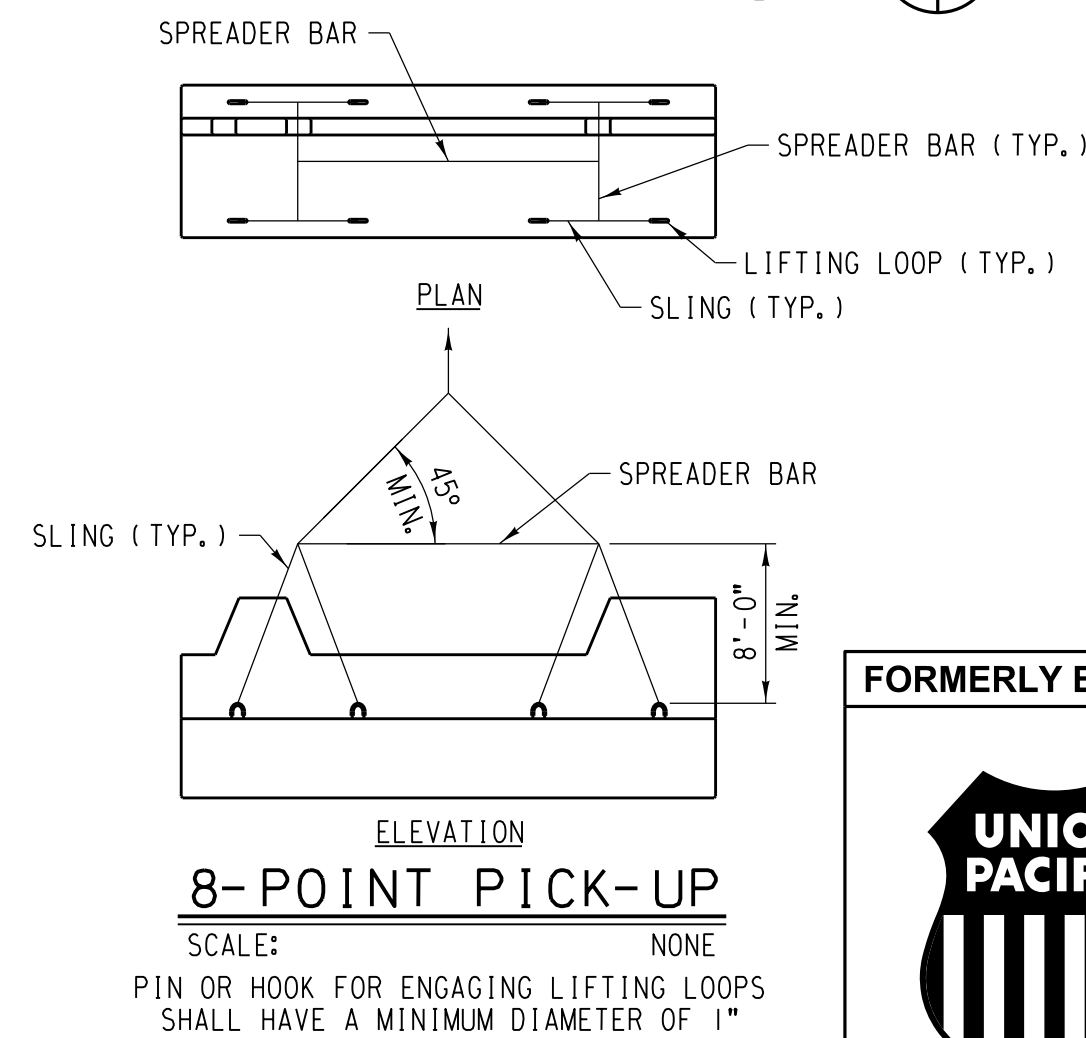
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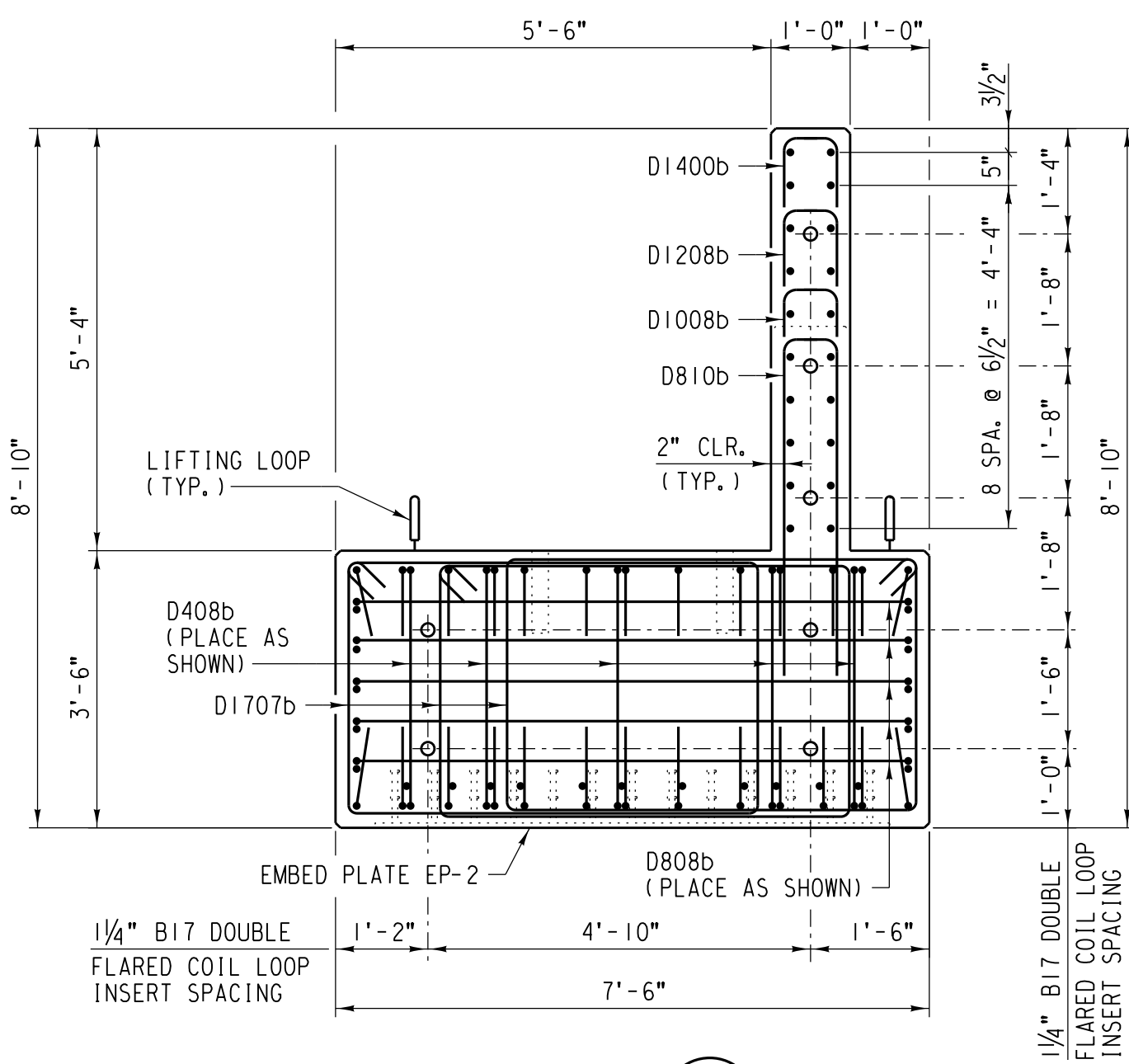
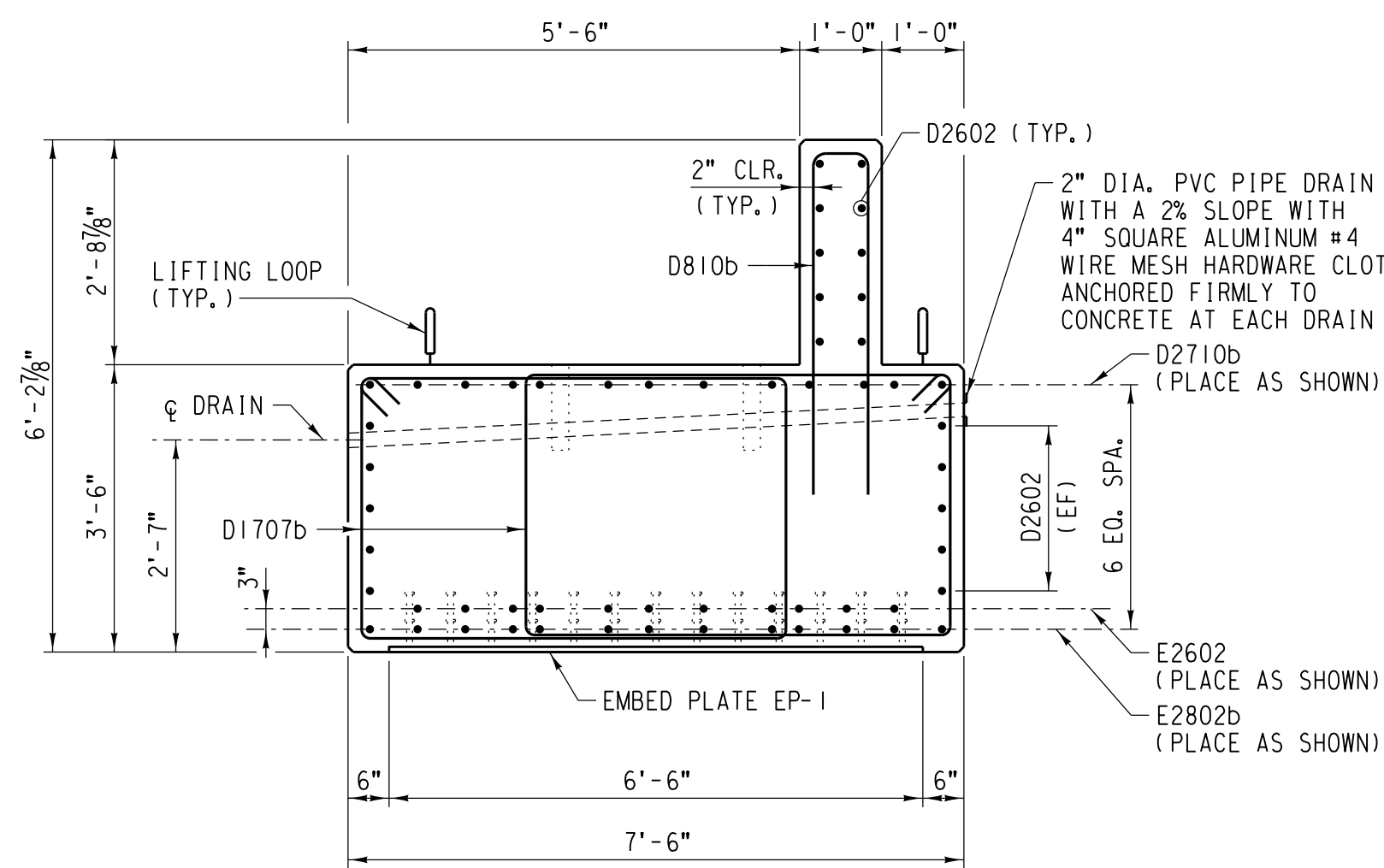
PRECAST CONCRETE END CAP PEC-1R
SCALE: 1/2"=1'-0"



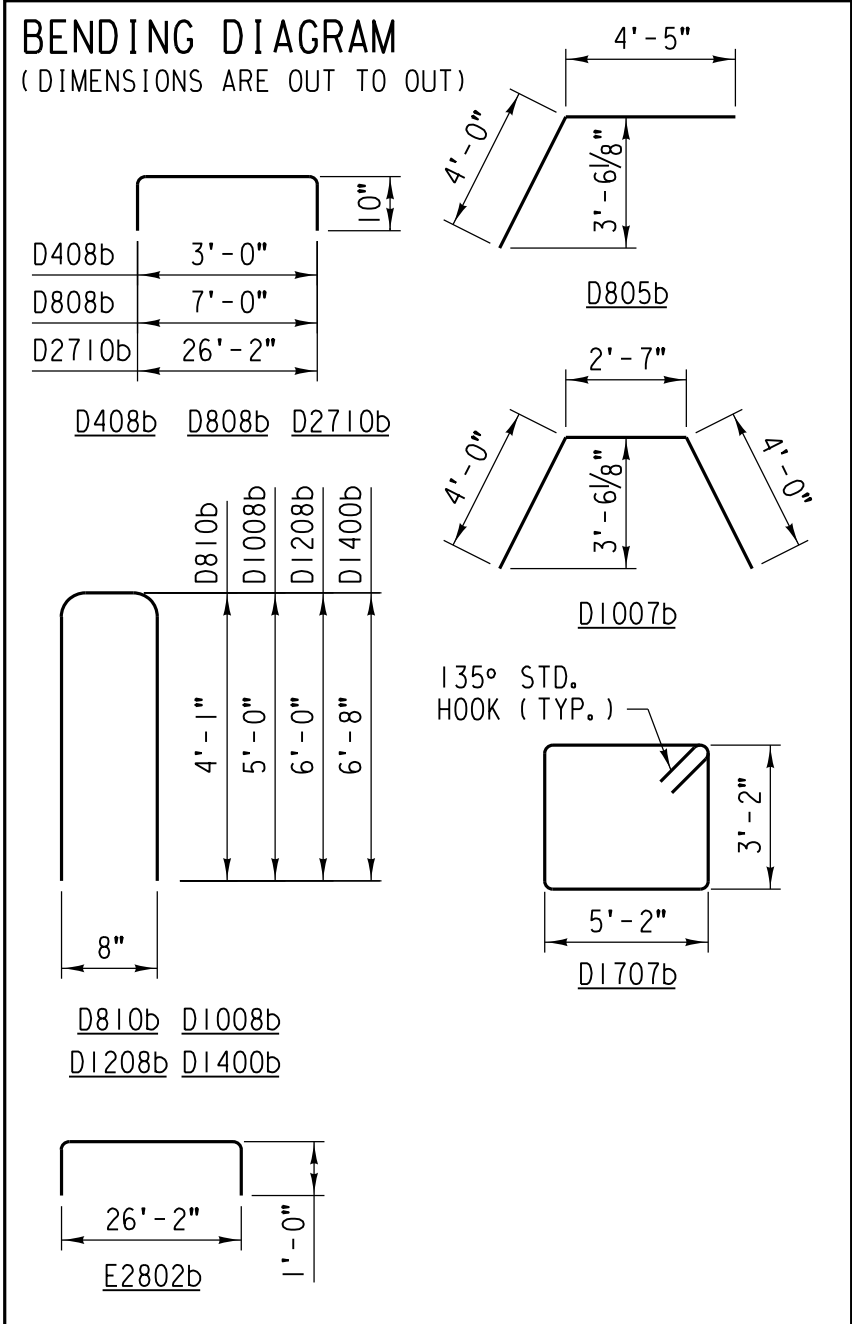
- NOTES:**
- END CAP PEC-1L, PEC-1R, PEC-2L, PEC-2R AND WINGWALL PWN-10 SHALL BE FIT UP AT THE FABRICATION PLANT PRIOR TO SHIPMENT TO ENSURE ACCURACY OF CONNECTIONS.
 - FABRICATOR IS RESPONSIBLE FOR DEVELOPING LIFTING LOOP ANCHORAGE DETAIL TO PROVIDE A SAFETY FACTOR OF 4 ON THE WORKING LOAD. DETAIL SHALL BE PROOF-TESTED WITH TEST RESULTS KEPT ON FILE BY THE FABRICATOR AND AVAILABLE FOR INSPECTION BY THE RAILROAD.
 - THE AREA AROUND LIFTING LOOPS SHALL BE RECESSED TO A DEPTH OF 3/4". AFTER CAP INSTALLATION, STRANDS SHALL BE BURNED OFF AND RECESSED TO A DEPTH OF 3/4". FILL AND FINISH THE RECESSES FLUSH TO PLAN DIMENSIONS USING AN EPOXY BONDING COMPOUND AND GROUT.
 - INSIDE SURFACE OF CAST HOLES SHALL BE EXPOSED UNCOATED CONCRETE. ALTERNATIVELY, CORRUGATED GALVANIZED STEEL DUCT WITH 2 1/2" INSIDE DIAMETER MAY BE USED AS A STAY-IN-PLACE FORM.
 - COVER OR FILL CAST HOLES WITH APPROVED MATERIAL TO PREVENT MOISTURE OR DEBRIS FROM ENTERING DURING STORAGE AND TRANSPORT.
 - MINIMALLY ADJUST REINFORCING AS REQUIRED TO CLEAR CAST HOLES AND EMBEDDED ITEMS.
 - EF = EACH FACE



MATERIAL SCHEDULE		
REQ'D.	UNIT	DESCRIPTION
29.1	CU.YD.	5000 PSI CONCRETE (PER NOTES, STD. DWG. 531100 SHT. T3)
1	LOT	REINFORCING STEEL (PER NOTES, STD. DWG. 531100 SHT. T3 AND SCHEDULE, THIS SHEET)
1	EA.	EMBED PLATE EP-1 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAIL, SHEET NO. 10)
1	EA.	EMBED PLATE EP-2 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAIL, SHEET NO. 10)
7	EA.	1 1/4" DAYTON SUPERIOR B17 DOUBLE FLARED COIL LOOP INSERT
8	EA.	LIFTING LOOP (PER NOTES AND DETAIL, THIS SHEET)
3	EA.	4" SQUARE ALUMINUM #4 MESH HARDWARE CLOTH
24	LIN.FT.	2" DIA. PVC PIPE, SCHEDULE 40



REINFORCING SCHEDULE				
TOTAL	MARK	SIZE	LENGTH	SHAPE
2	D300	#5	3'-0"	—
2	D306	#5	3'-6"	—
2	D401	#5	4'-1"	—
10	D408b	#5	4'-8"	┌
2	D408	#5	4'-8"	—
2	D409	#5	4'-9"	—
2	D500	#5	5'-0"	—
2	D503	#5	5'-3"	—
2	D507	#5	5'-7"	—
2	D805b	#5	8'-5"	└
10	D808b	#5	8'-8"	┌
33	D810b	#5	8'-10"	┌
3	D1008b	#5	10'-8"	┌
2	D1007b	#5	10'-7"	┌
3	D1208b	#5	12'-8"	┌
14	D1400b	#5	14'-0"	┌
125	D1707b	#5	17'-7"	┌
20	D2602	#5	26'-2"	┌
13	D2710b	#5	27'-10"	┌
11	E2602	#6	26'-2"	┌
13	E2802b	#6	28'-2"	┌

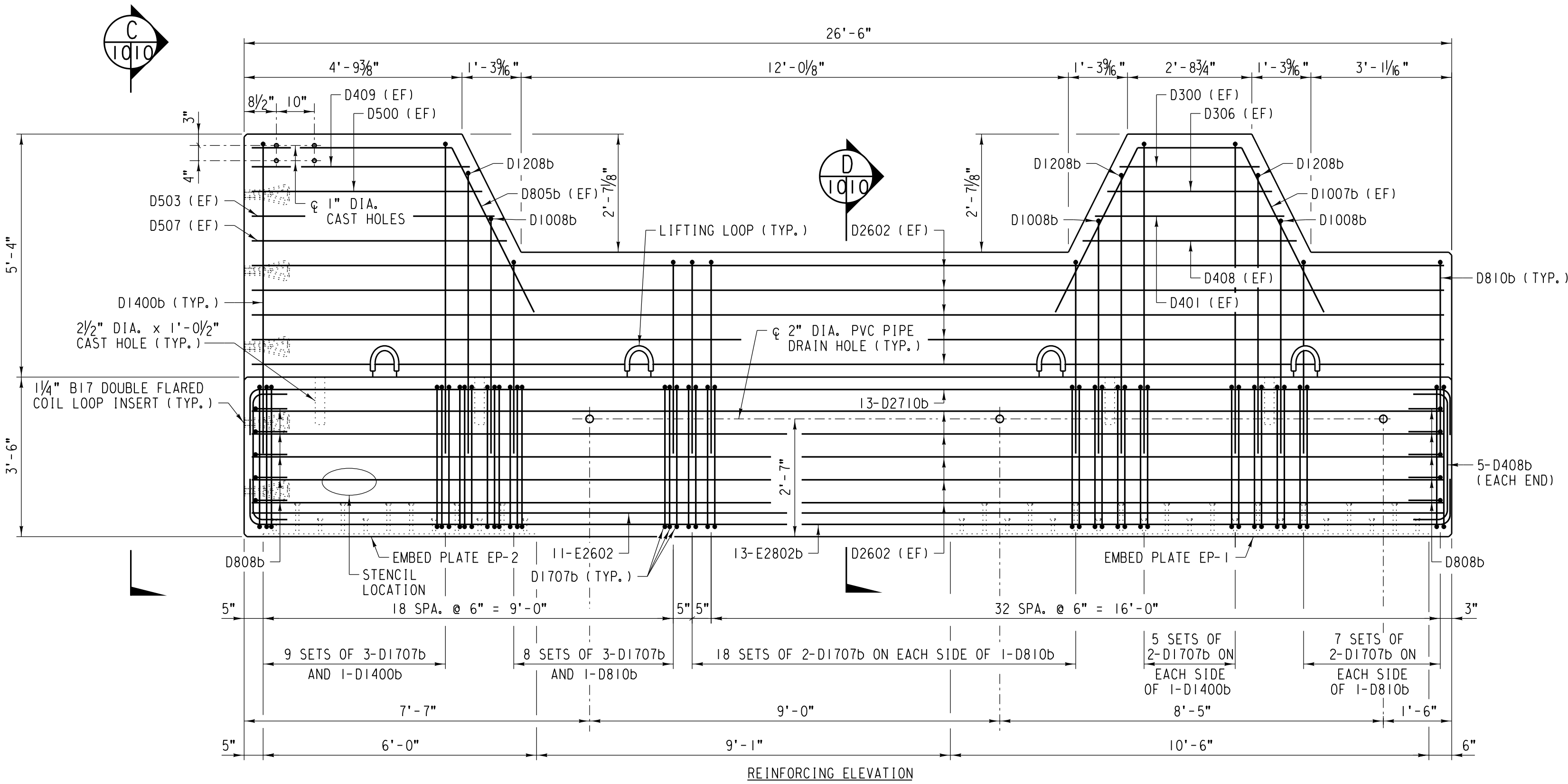


NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.
EST. WT. OF REINFORCING STEEL = 5,035 LB.

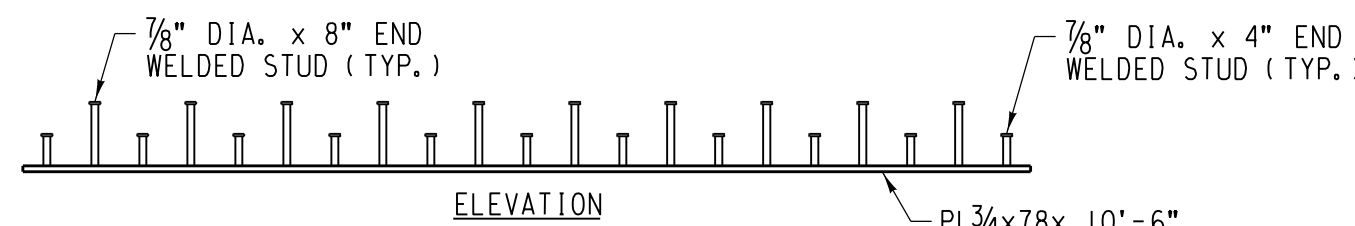
EST. WT. OF PRECAST CONCRETE
END CAP PEC-1R = 121,455 LB. EA. (60.8 TON)

NO.	DATE	REVISIONS
1	05/28/2021	DATE
COMPLETION STATUS:		
FINAL		
STATUS		
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		
CONSULTANT ENGINEER		
05/28/2021		
DATE		
PROJECT ID:	WORK ORDER:	C NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
UNION PACIFIC RAILROAD		Office of Director Structures Design	
LOCATION & DESCRIPTION:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
REPLACING 1 SPAN TPG x 90'		REPLACING 1 SPAN TPG x 70' (2 TRACKS)	
SHEET TITLE:		PRECAST CONCRETE END CAP 1R DETAILS	

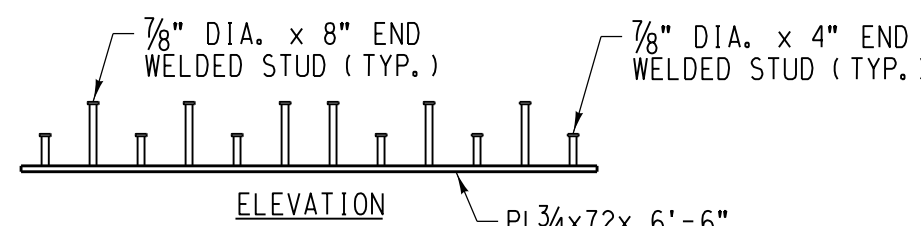


PRECAST CONCRETE END CAP PEC-1L



EMBED PLATE EP- I

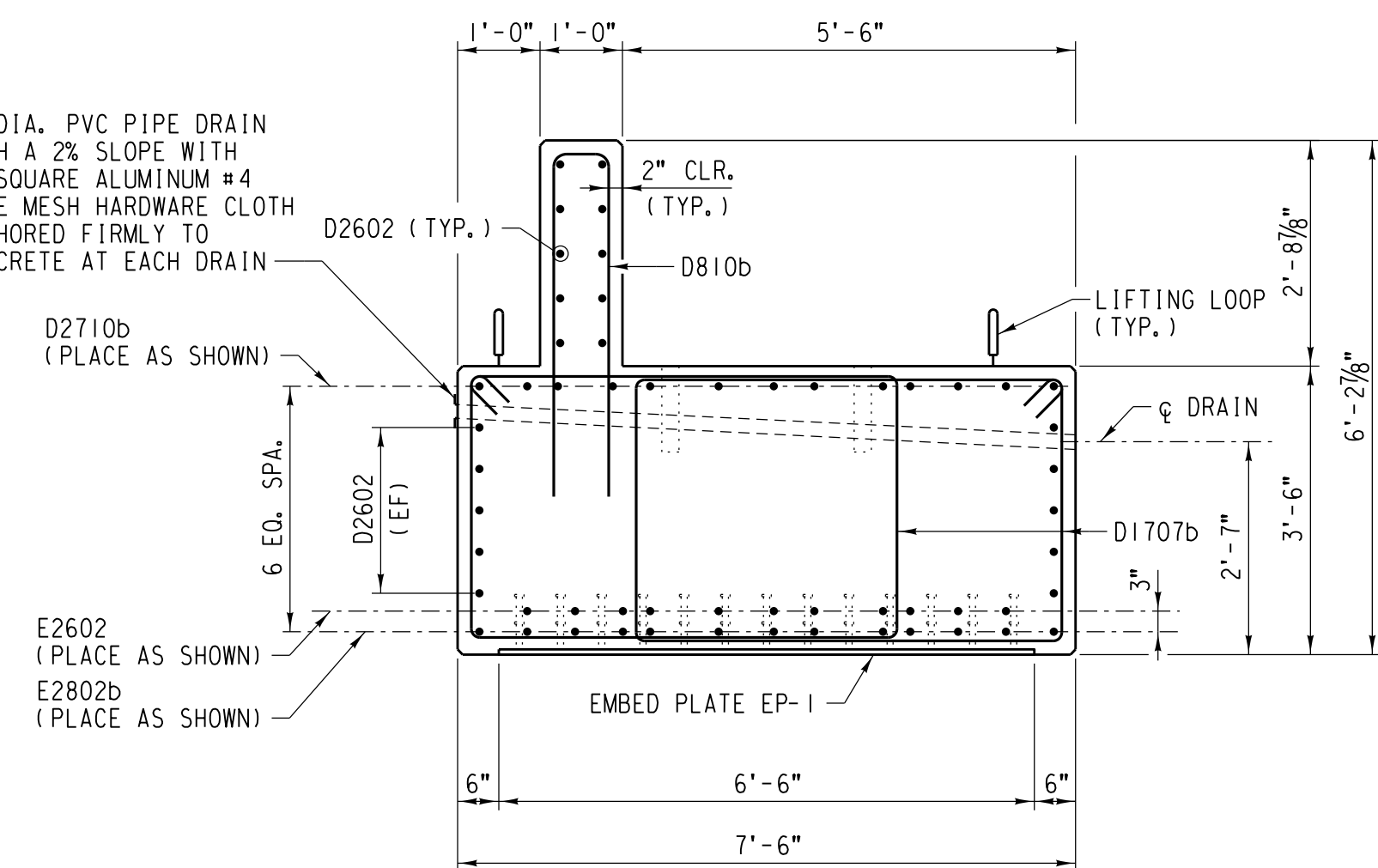
SCALE: $\frac{1}{2}" = 1' - 0"$
EST. WT. = 2,370 LB. EA.



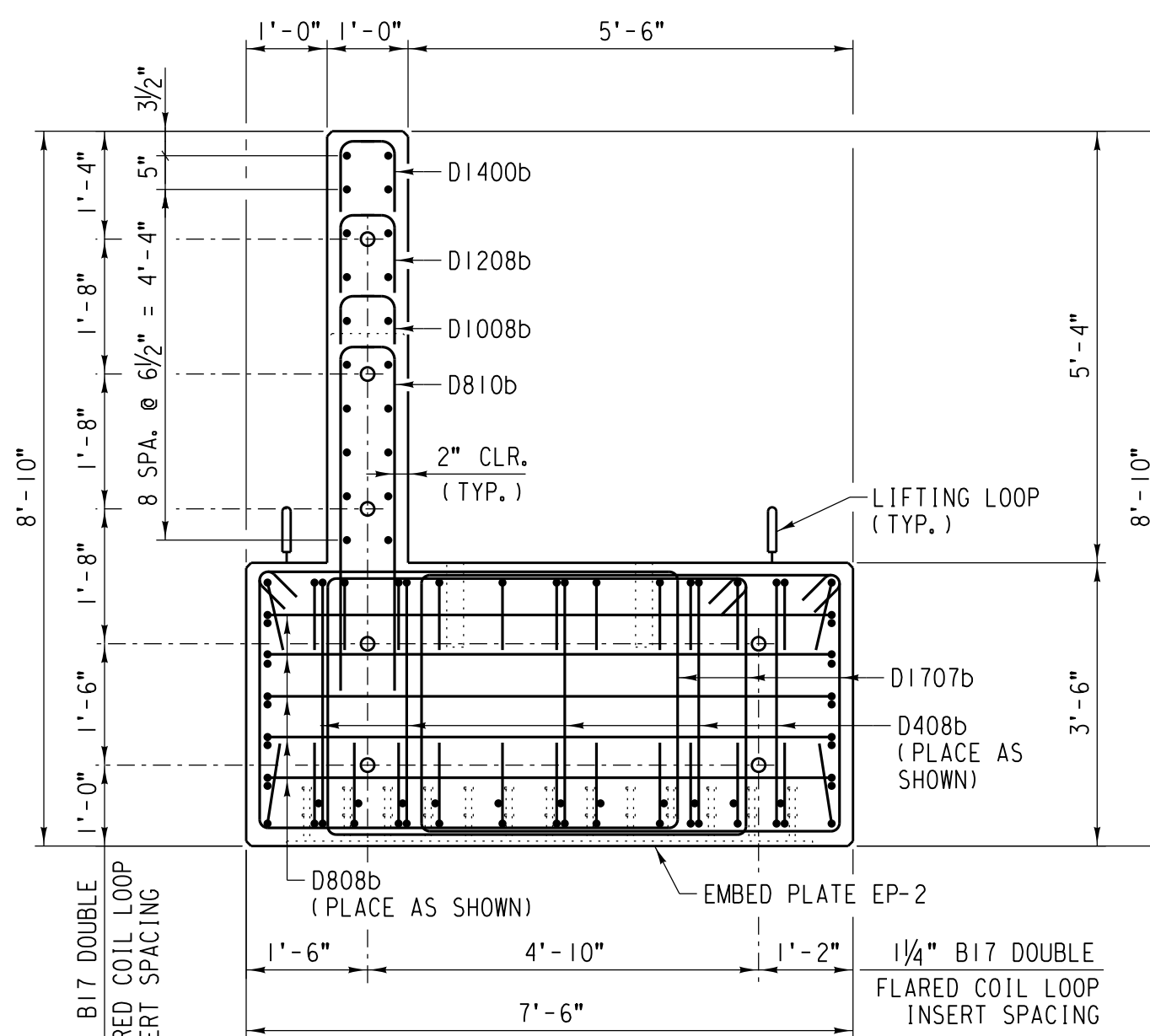
EMBED PLATE EP-2

SCALE: $\frac{1}{2}" = 1' - 0"$
EST. WT. = 1,455 LB. EA.

2" DIA. PVC PIPE DRAIN
WITH A 2% SLOPE WITH
4" SQUARE ALUMINUM #4
WIRE MESH HARDWARE CLOTH
ANCHORED FIRMLY TO
CONCRETE AT EACH DRAIN -



SECTION D
SCALE: 1/2" = 1'-0" 1019

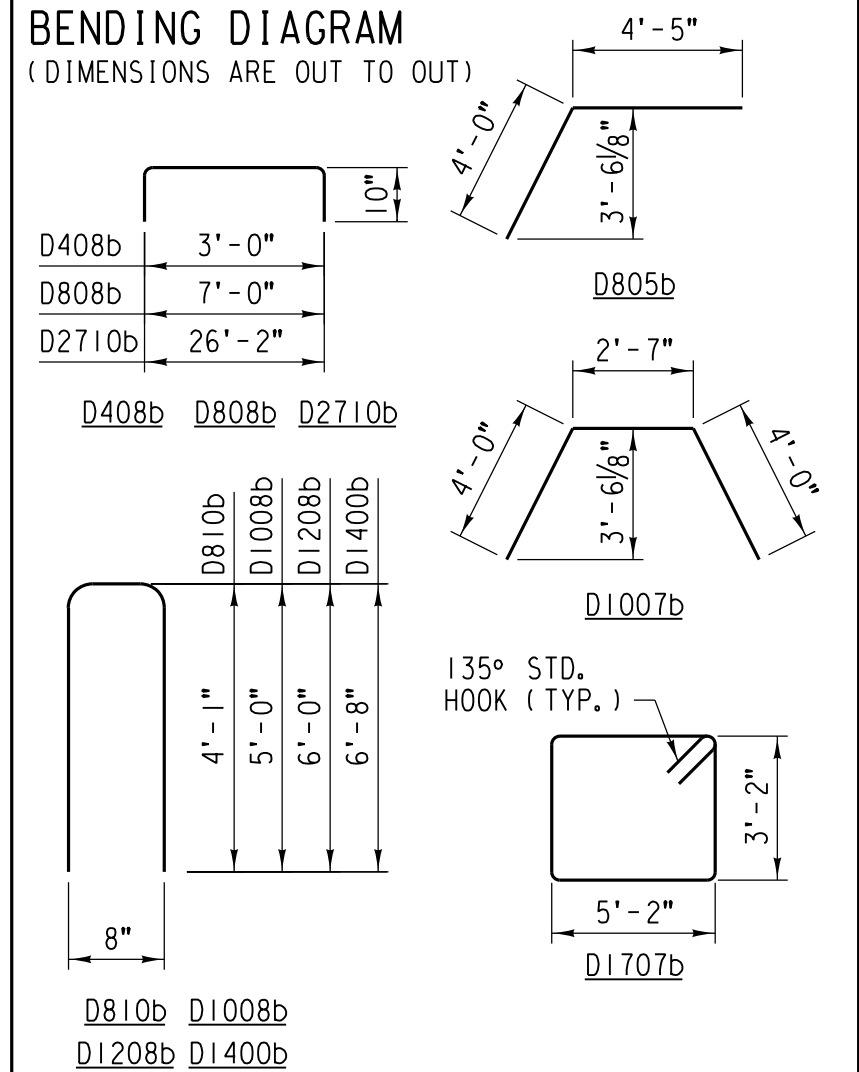


VIEW C
SCALE: 1/2" = 1'-0" 1010

- NOTES:**
1. END CAP PEC-1L, PEC-1R, PEC-2L, PEC-2R AND WINGWALL PWW-10 SHALL BE FIT UP AT THE FABRICATION PLANT PRIOR TO SHIPMENT TO ENSURE ACCURACY OF CONNECTIONS.
 2. FABRICATOR IS RESPONSIBLE FOR DEVELOPING LIFTING LOOP ANCHORAGE DETAIL TO PROVIDE A SAFETY FACTOR OF 4 ON THE WORKING LOAD. DETAIL SHALL BE PROOF-TESTED WITH TEST RESULTS KEPT ON FILE BY THE FABRICATOR AND AVAILABLE FOR INSPECTION BY THE RAILROAD.
 3. THE AREA AROUND LIFTING LOOPS SHALL BE RECESSED TO A DEPTH OF $\frac{3}{4}$ ". AFTER CAP INSTALLATION, STRANDS SHALL BE BURNED OFF AND RECESSED TO A DEPTH OF $\frac{3}{4}$ ". FILL AND FINISH THE RECESSES FLUSH TO PLATE DIMENSIONS USING AN EPOXY BONDING COMPOUND AND GROUT.
 4. INSIDE SURFACE OF CAST HOLES SHALL BE EXPOSED UNCOATED CONCRETE. ALTERNATIVELY, CORRUGATED GALVANIZED STEEL DUCT WITH $\frac{1}{2}$ " INSIDE DIAMETER MAY BE USED AS A STAY-IN-PLACE FORM.
 5. COVER OR FILL CAST HOLES WITH APPROVED MATERIAL TO PREVENT MOISTURE OR DEBRIS FROM ENTERING DURING STORAGE AND TRANSPORT.
 6. MINIMALLY ADJUST REINFORCING AS REQUIRED TO CLEAR CAST HOLES AND EMBEDDED ITEMS.
 7. FOR 8-POINT PICK-UP DETAILS, SEE SHEET NO. 9
 8. EF = EACH FACE

REINFORCING SCHEDULE				
TOTAL	MARK	SIZE	LENGTH	SHAPE
2	D300	#5	3'-0"	
2	D306	#5	3'-6"	
2	D401	#5	4'-1"	
10	D408b	#5	4'-8"	
2	D408	#5	4'-8"	
2	D409	#5	4'-9"	
2	D500	#5	5'-0"	
2	D503	#5	5'-3"	
2	D507	#5	5'-7"	
2	D805b	#5	8'-5"	
10	D808b	#5	8'-8"	
33	D810b	#5	8'-10"	
3	D1008b	#5	10'-8"	
2	D1007b	#5	10'-7"	
3	D1208b	#5	12'-8"	
14	D1400b	#5	14'-0"	
125	D1707b	#5	17'-7"	
20	D2602	#5	26'-2"	
13	D2710b	#5	27'-10"	
11	E2602	#6	26'-2"	
13	E2802b	#6	28'-2"	

BENDING DIAGRAM
(DIMENSIONS ARE OUT TO



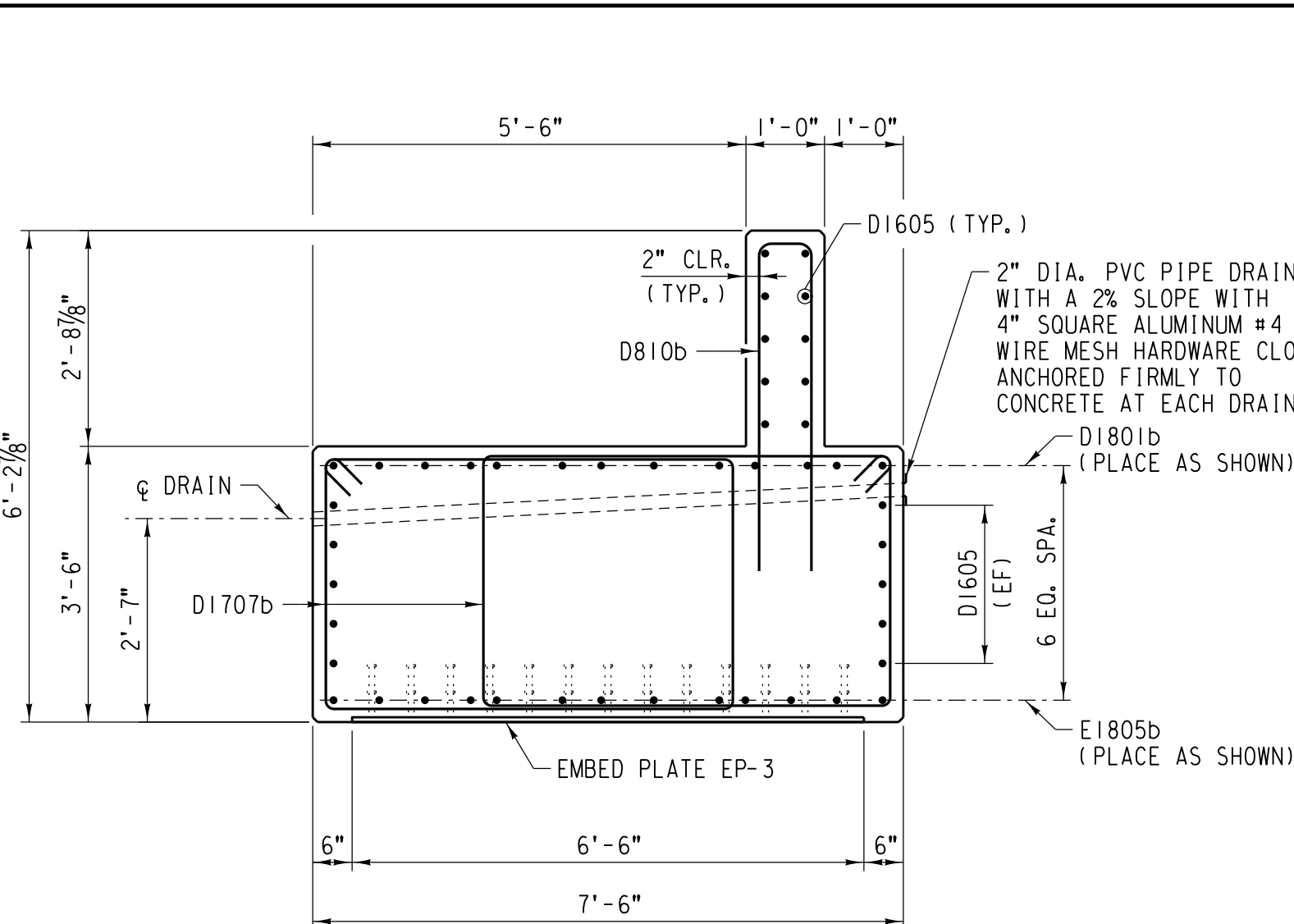
NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.

EST. WT. OF REINFORCING STEEL = 5,035 LB.

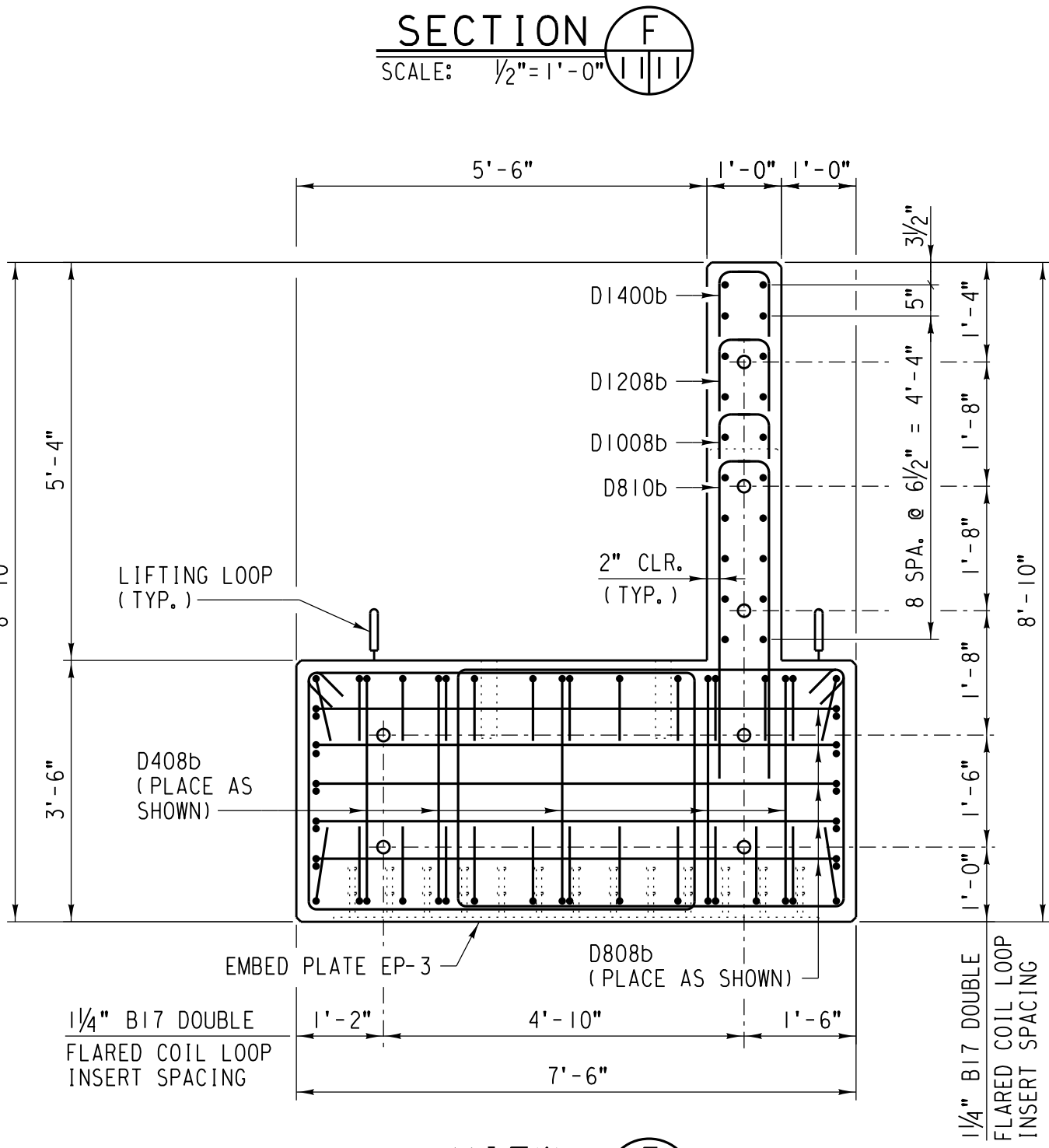
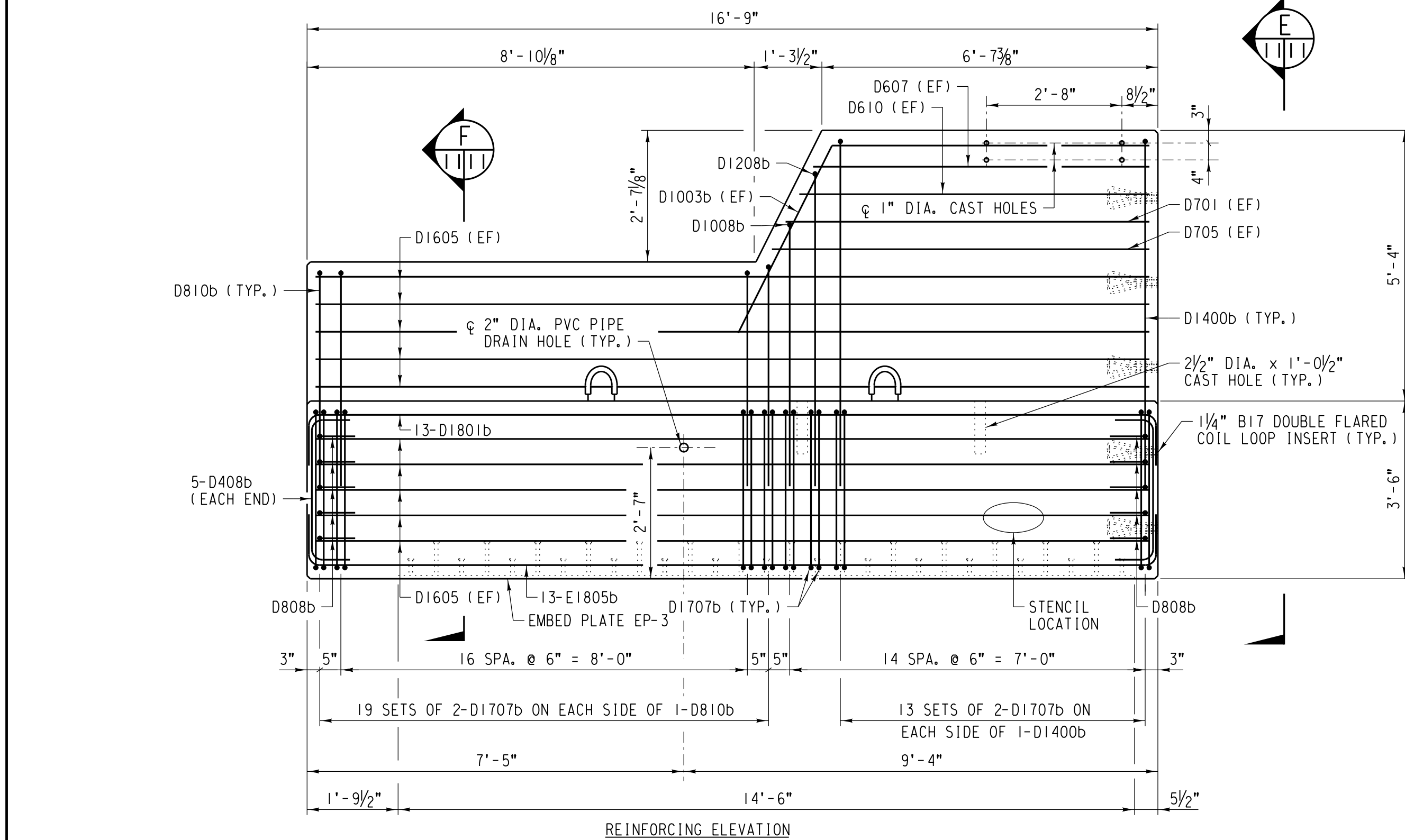
EST. WT. OF PRECAST CONCRETE
END CAP PEC-1L = 121,455 LB. EA. (60.8 TON)

[illegible]

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E10 of E43	SHEET TITLE: PRECAST CONCRETE END CAP 1L DETAILS			

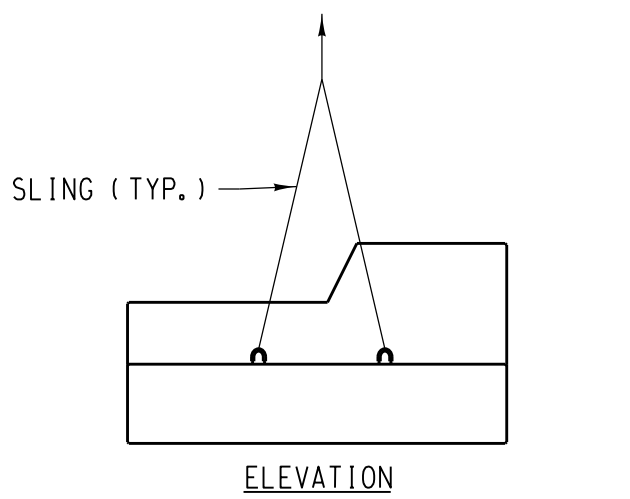


REINFORCING SCHEDULE				
TOTAL	MARK	SIZE	LENGTH	SHAPE
2	D607	#5	6' - 7"	
10	D408b	#5	4' - 8"	
2	D610	#5	6' - 10"	
2	D701	#5	7' - 1"	
2	D705	#5	7' - 5"	
10	D808b	#5	8' - 8"	
2	D1003b	#5	10' - 3"	
19	D810b	#5	8' - 10"	
1	D1008b	#5	10' - 8"	
1	D1208b	#5	12' - 8"	
20	D1605	#5	16' - 5"	
13	D1400b	#5	14' - 0"	
68	D1707b	#5	17' - 7"	
13	D1801b	#5	18' - 1"	
13	E1805b	#6	18' - 5"	



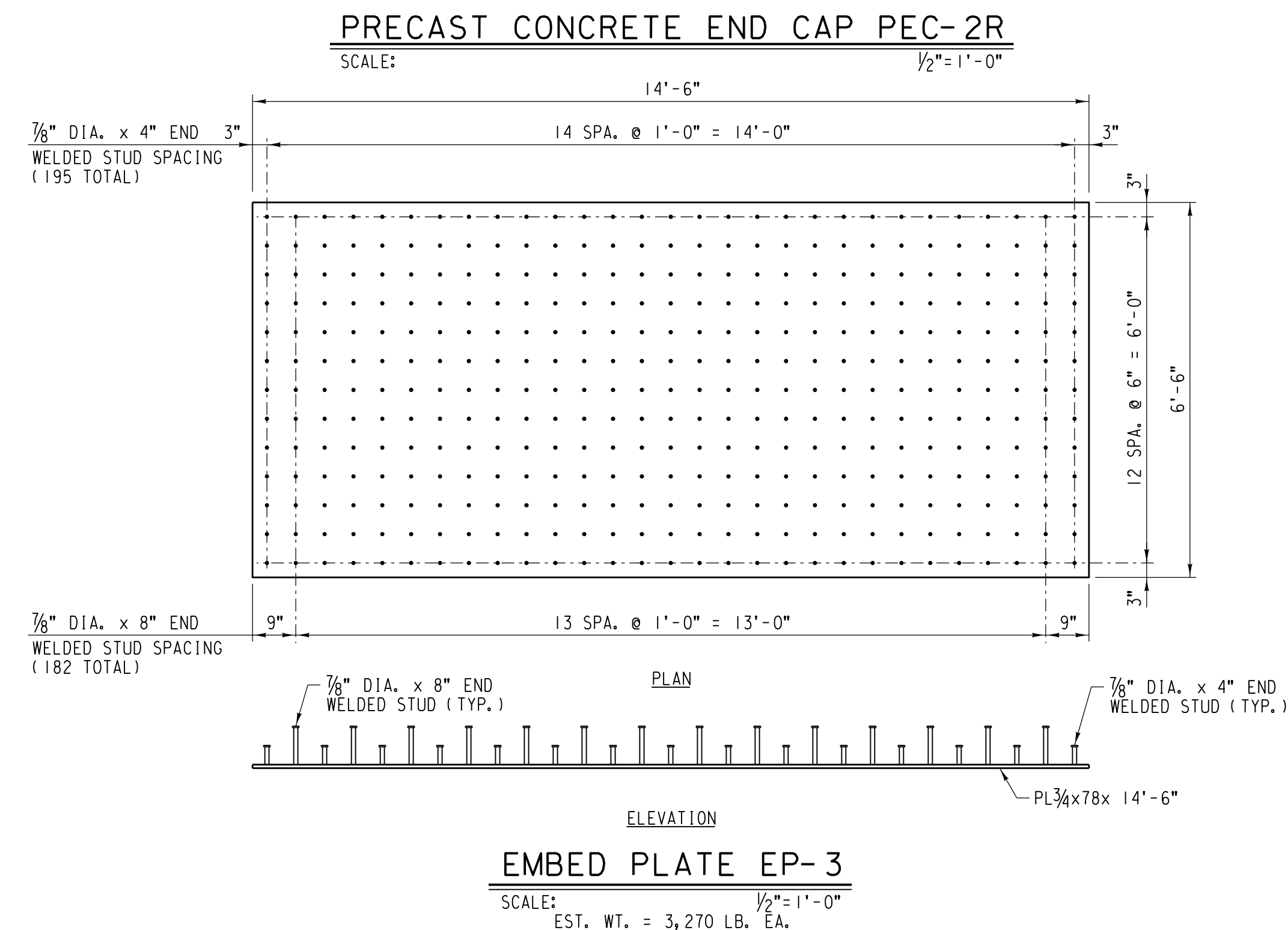
NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.

EST. WT. OF REINFORCING STEEL = 2,805 LB.



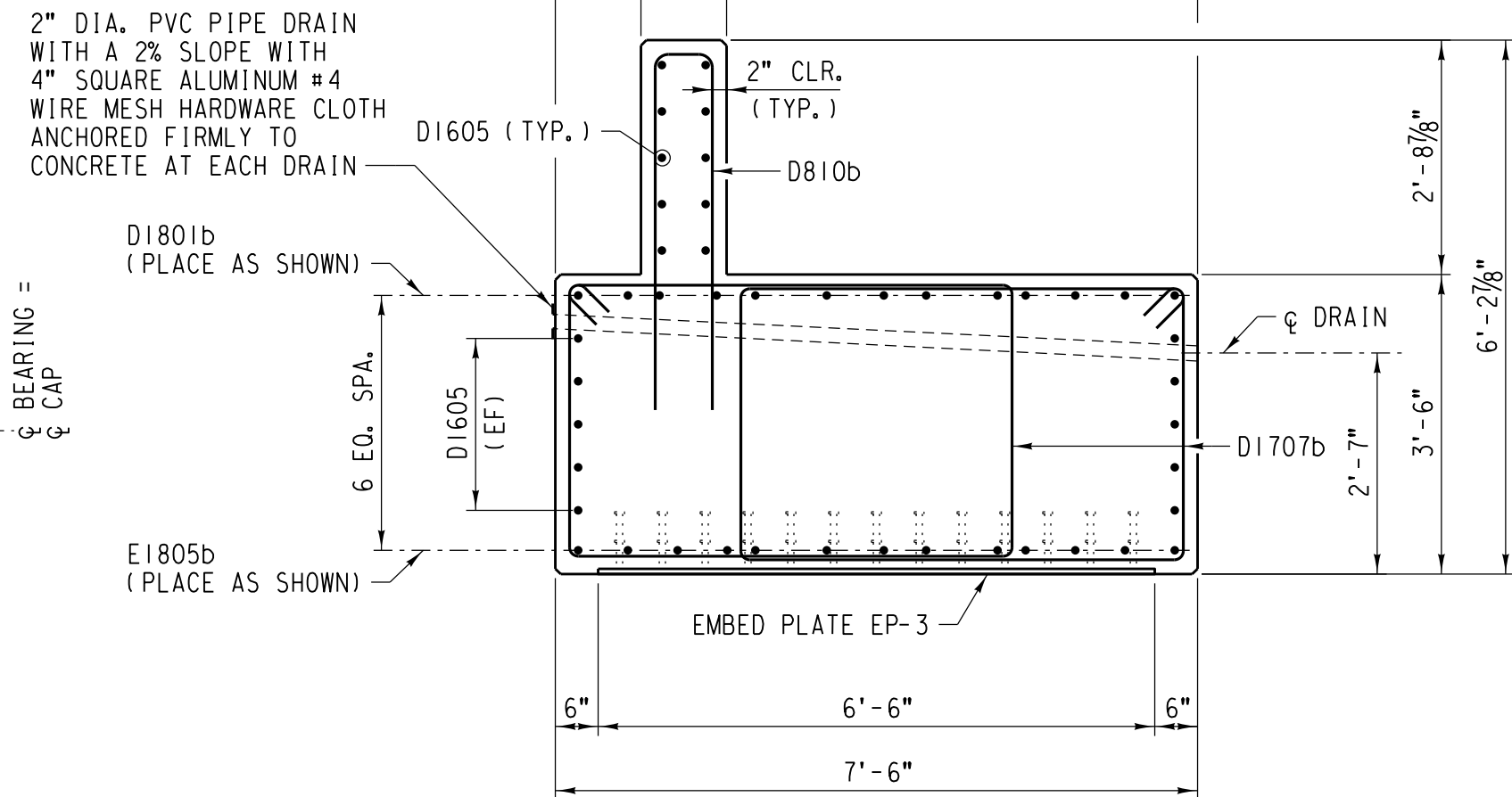
EST. WT. OF PRECAST CONCRETE
END CAP PEC-2R = 77,910 LB. EA. (39.0 TON)

	/ /	
NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		<u>05/28/2021</u> DATE
		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		<u>05/28/2021</u> DATE
CONSULTANT ENGINEER		
PROJECT ID:	WORK ORDER:	C E NUMBER:
	31876	122525

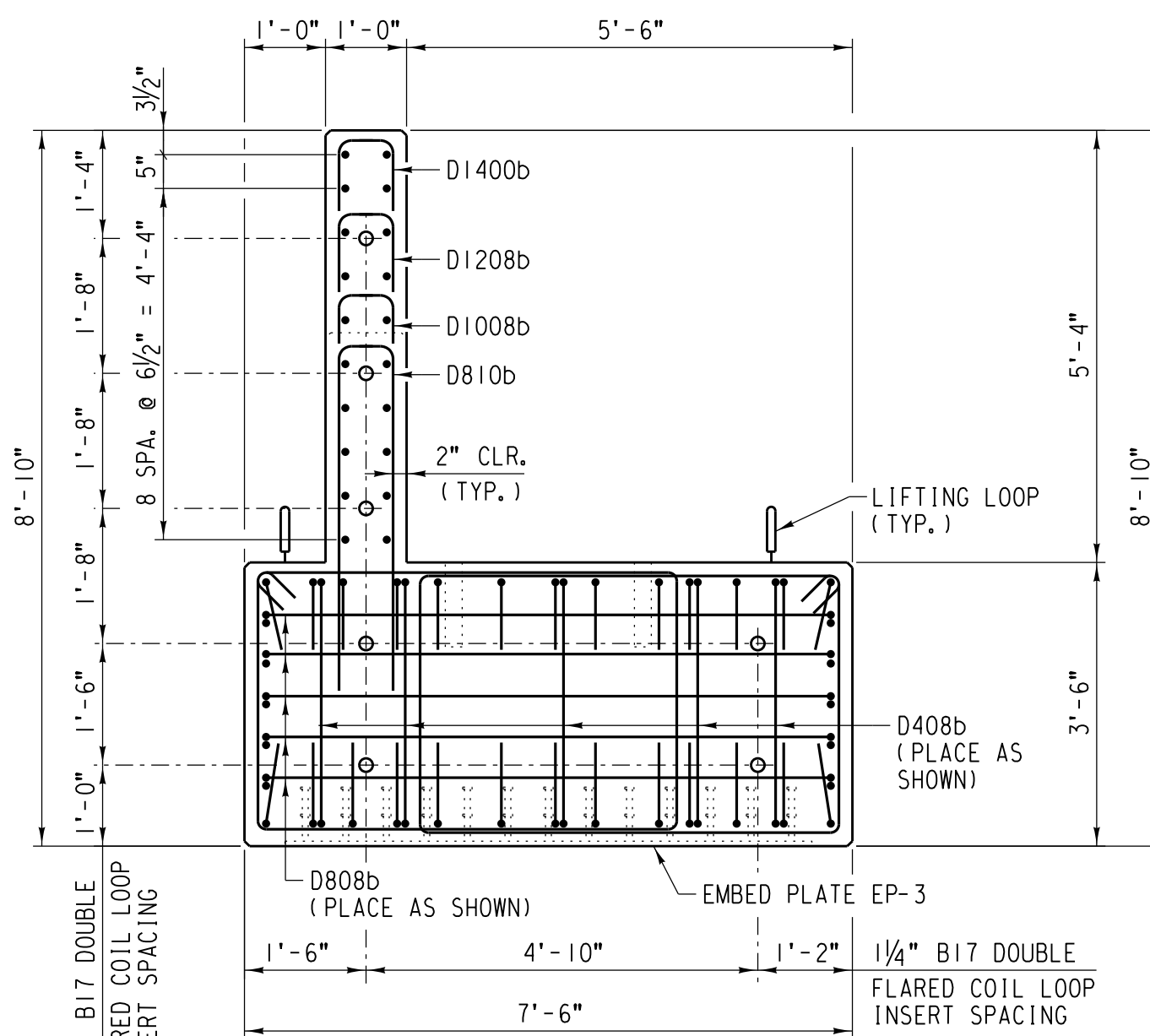


- NOTES:**
1. END CAP PEC-1L, PEC-1R, PEC-2L, PEC-2R AND WINGWALL PFW-10 SHALL BE FIT UP AT THE FABRICATION PLANT PRIOR TO SHIPMENT TO ENSURE ACCURACY OF CONNECTIONS.
 2. FABRICATOR IS RESPONSIBLE FOR DEVELOPING LIFTING LOOP ANCHORAGE DETAIL TO PROVIDE A SAFETY FACTOR OF 4 ON THE WORKING LOAD. DETAIL SHALL BE PROOF-TESTED WITH TEST RESULTS KEPT ON FILE BY THE FABRICATOR AND AVAILABLE FOR INSPECTION BY THE RAILROAD.
 3. THE AREA AROUND LIFTING LOOPS SHALL BE RECESSED TO A DEPTH OF $\frac{3}{4}$ ". AFTER CAP INSTALLATION, STRANDS SHALL BE BURNED OFF AND RECESSED TO A DEPTH OF $\frac{3}{4}$ ". FILL AND FINISH THE RECESSES FLOUSH TO PLAN DIMENSIONS USING AN EPOXY BONDING COMPOUND AND GROUT.
 4. INSIDE SURFACE OF CAST HOLES SHALL BE EXPOSED UNCOATED CONCRETE. ALTERNATIVELY, CORRUGATED GALVANIZED STEEL DUCT WITH $\frac{1}{2}$ " INSIDE DIAMETER MAY BE USED AS A STAY-IN-PLACE FORM.
 5. COVER OR FILL CAST HOLES WITH APPROVED MATERIAL TO PREVENT MOISTURE OR DEBRIS FROM ENTERING DURING STORAGE AND TRANSPORT.
 6. MINIMALLY ADJUST REINFORCING AS REQUIRED TO CLEAR CAST HOLES AND EMBEDDED ITEMS.
 7. EF = EACH FACE

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION			
	SHT NO.: E11 of E43	1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
		SHEET TITLE:		PRECAST CONCRETE END CAP 2R DETAILS	



SECTION H
SCALE: 1/2" = 1'-0" 12 12

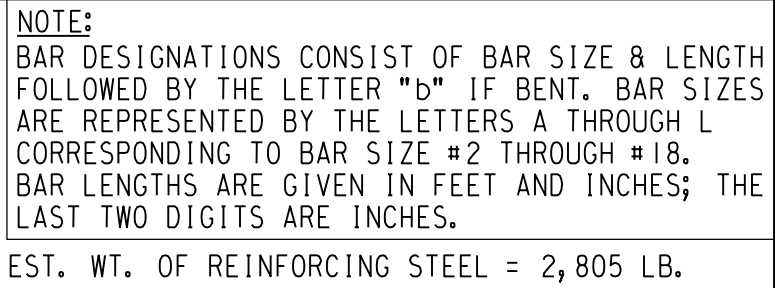


VIEW C
SCALE: 1/2" = 1'-0" 1212

SCALE: $\frac{1}{2}" = 1' - 0"$

REINFORCING SCHEDULE				
TOTAL	MARK	SIZE	LENGTH	SHAPE
2	D607	#5	6'-7"	▬
10	D408b	#5	4'-8"	┌
2	D610	#5	6'-10"	▬
2	D701	#5	7'-1"	▬
2	D705	#5	7'-5"	▬
10	D808b	#5	8'-8"	┌
2	D1003b	#5	10'-3"	└
19	D810b	#5	8'-10"	┌
1	D1008b	#5	10'-8"	┌
1	D1208b	#5	12'-8"	┌
20	D1605	#5	16'-5"	┌
13	D1400b	#5	14'-0"	┌
68	D1707b	#5	17'-7"	┌
13	D1801b	#5	18'-1"	┌
13	E1805b	#6	18'-5"	┌

(DIMENSIONS ARE OUT TO OUT)



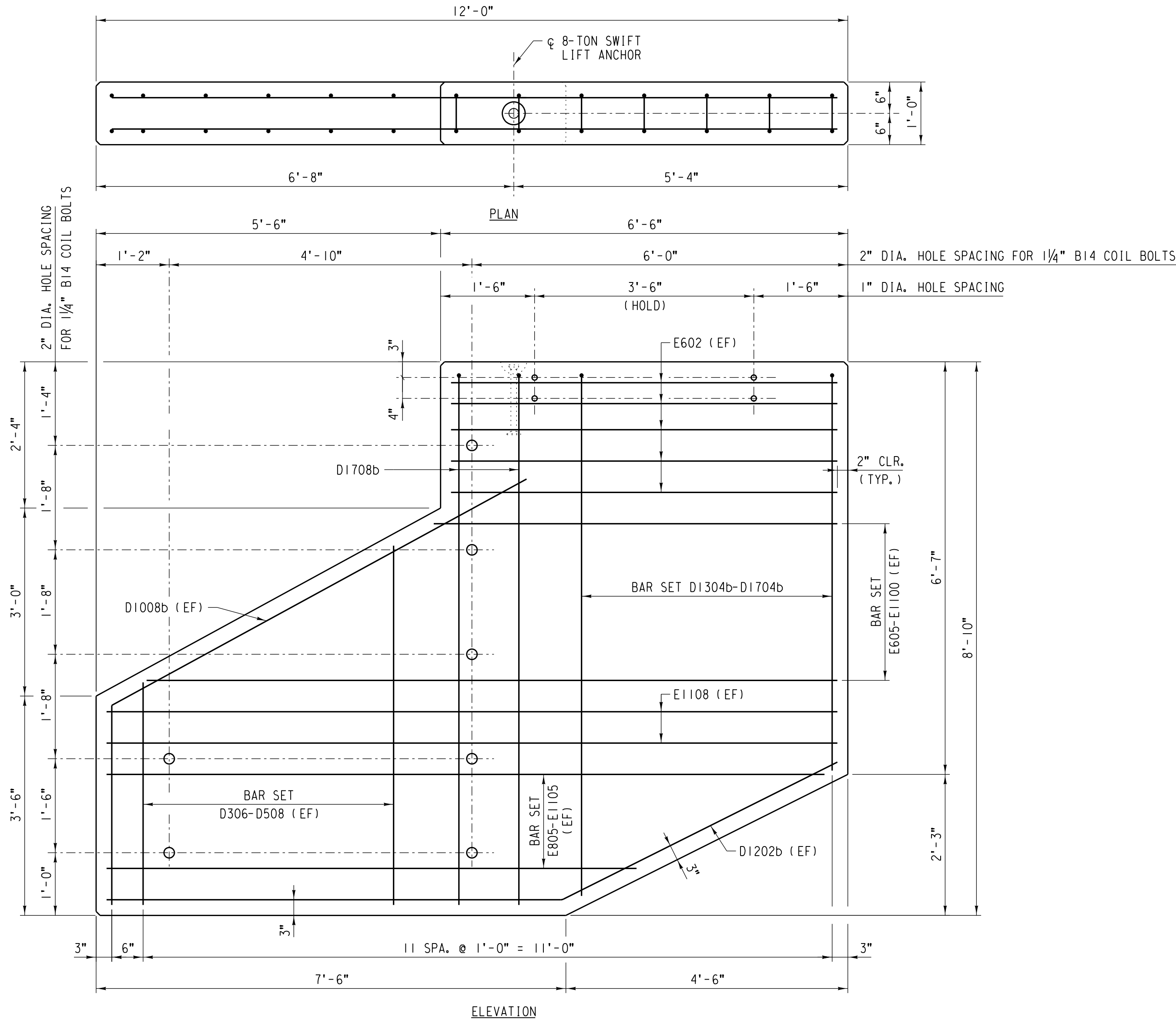
EST. WT. OF REINFORCING STEEL = 2,805 LB.

1. END CAP PEC-1L, PEC-1R, PEC-2L, PEC-2R AND WINGWALL PWM-10 SHALL BE FIT UP AT THE FABRICATION PLANT PRIOR TO SHIPMENT TO ENSURE ACCURACY OF CONNECTIONS.
2. FABRICATOR IS RESPONSIBLE FOR DEVELOPING LIFTING LOOP ANCHORAGE DETAIL TO PROVIDE A SAFETY FACTOR OF 4 ON THE WORKING LOAD. DETAIL SHALL BE PROOF-TESTED WITH TEST RESULTS KEPT ON FILE BY THE FABRICATOR AND AVAILABLE FOR INSPECTION BY THE RAILROAD.
3. THE AREA AROUND LIFTING LOOPS SHALL BE RECESSED TO A DEPTH OF $\frac{3}{4}$ ". AFTER CAP INSTALLATION, STRANDS SHALL BE BURNED OFF AND RECESSED TO A DEPTH OF $\frac{3}{4}$ ". FILL AND FINISH THE RECESSES FLUSH TO FIN DIMENSIONS USING AN EPOXY BONDING COMPOUND AND GROUT.
4. INSIDE SURFACE OF CAST HOLES SHALL BE EXPOSED UNCOATED CONCRETE. ALTERNATIVELY, CORRUGATED GALVANIZED STEEL DUCT WITH $\frac{1}{2}$ " INSIDE DIAMETER MAY BE USED AS A STAY-IN-PLACE FORM.
5. COVER OR FILL CAST HOLES WITH APPROVED MATERIAL TO PREVENT MOISTURE OR DEBRIS FROM ENTERING DURING STORAGE AND TRANSPORT.
6. MINIMALLY ADJUST REINFORCING AS REQUIRED TO CLEAR CAST HOLES AND EMBEDDED ITEMS.
7. FOR 4-POINT PICK-UP DETAILS, SEE SHEET NO. 11
8. EF = EACH FACE

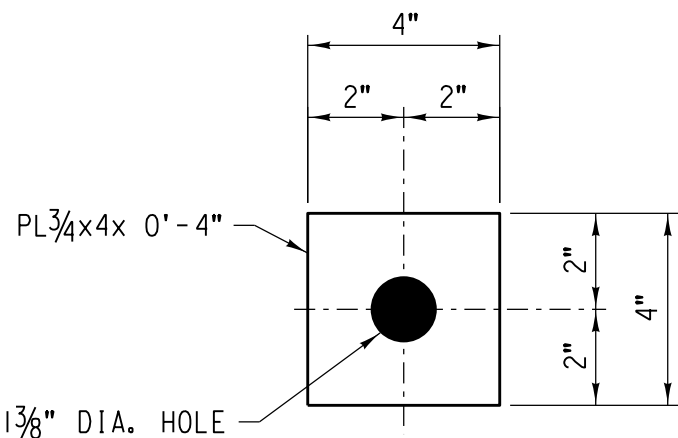
	/	/	
	/	/	
NO.	DATE	REVISIONS	
COMPLETION STATUS:			
FINAL		<u>05/28/2021</u> DATE	
			
APPROVED FOR UNION PACIFIC RAILROAD BY:			
MATTHEW BECKER		<u>05/28/2021</u> DATE	
CONSULTANT ENGINEER			
PROJECT ID:	WORK ORDER:	C E NUMBER:	
	31876	122525	

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG @ 90' REPLACING 1 SPAN TPGOD @ 70' (2 TRACKS)			
	SHT NO.: E12 of E43	SHEET TITLE: PRECAST CONCRETE END CAP 2L DETAILS			

FILE NAME: C:\user\stefan\ez\Benesch\p\000093_b11.dgn



PRECAST CONCRETE WINGWALL PWW-10
SCALE: 3/4"=1'-0"

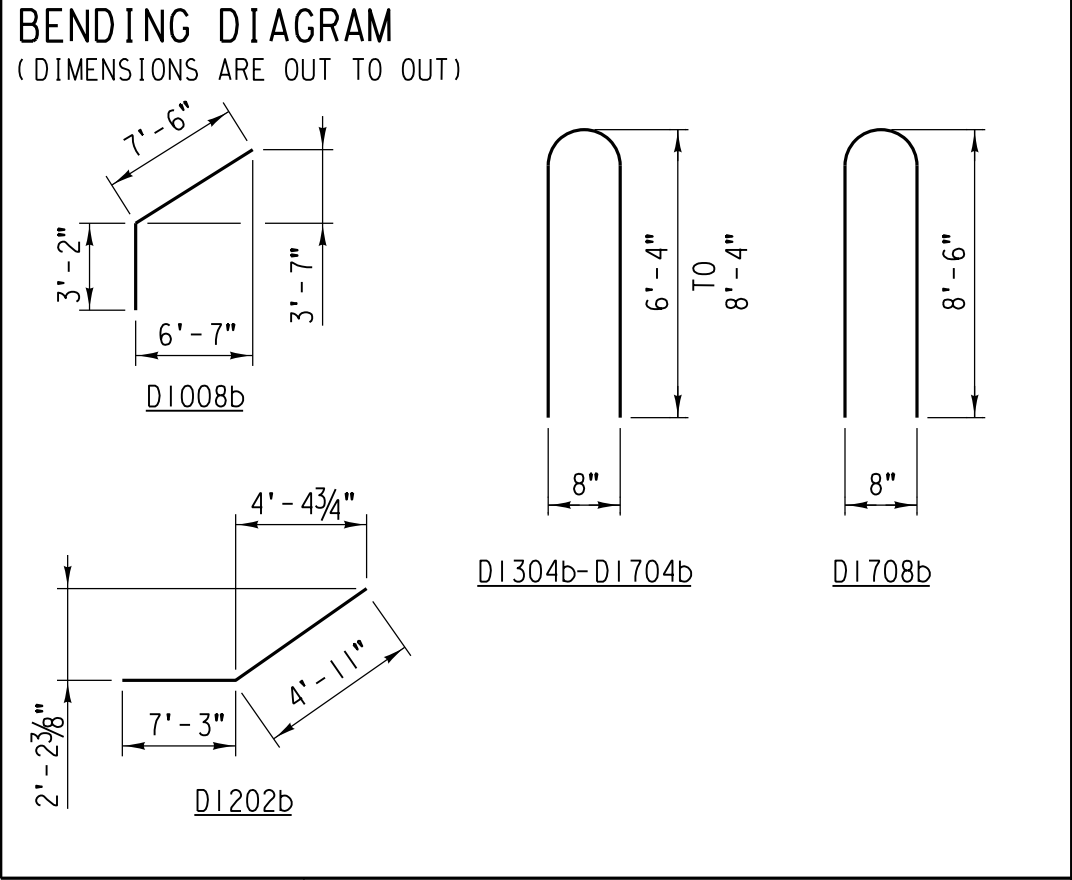


WASHER W100
SCALE: 3"=1'-0"
EST. WT. = 3.4 LB. EA.
GALVANIZE AFTER FABRICATION

MATERIAL SCHEDULE (QUANTITY PER WINGWALL PWW-10)		
REQ'D.	UNIT	DESCRIPTION
3.0	CU. YD.	5000 PSI CONCRETE (PER NOTES, STD. DWG. 531100 SHT. T3)
1	LOT	REINFORCING STEEL (PER NOTES, STD. DWG. 531100 SHT. T3 AND SCHEDULE, THIS SHEET)
1	EA.	8-TON SWIFT LIFT ANCHOR
8	EA.	WASHER W100 (PER NOTES, STD. DWG. 531100 SHT. T3 AND DETAIL, THIS SHEET)
8	EA.	1 1/4" DIA. x 16" COIL BOLT, DAYTON SUPERIOR B-14

REINFORCING SCHEDULE (QUANTITY PER WINGWALL PWW-10)				
TOTAL	MARK	SIZE	LENGTH	SHAPE
2	D1008b	#5	10'-8"	
2	D1202b	#5	12'-2"	
2	D1708b	#5	17'-8"	
10	E602	#6	6'-2"	
4	E1108	#6	11'-8"	

SET LIST							
MARK	SIZE	MIN. LENGTH	MAX. LENGTH	INCREMENT	NO. OF BARS/SET	NO. OF SETS	SHAPE
D306-D508	#5	3'-6"	5'-8"	6 1/2"	5	2	
D1304b-D1704b	#5	13'-4"	17'-4"	1'-0"	5	2	
E605-E1100	#6	6'-5"	11'-0"	11"	6	2	
E805-E1105	#6	8'-5"	11'-5"	1'-0"	4	2	



NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.
EST. WT. OF REINFORCING STEEL = 735 LB.

- NOTES:
- END CAP PEC-1L, PEC-1R, PEC-2L, PEC-2R AND WINGWALL PWW-10 SHALL BE FIT UP AT THE FABRICATION PLANT PRIOR TO SHIPMENT TO ENSURE ACCURACY OF CONNECTIONS.
 - MINIMALLY ADJUST REINFORCING AS REQUIRED TO CLEAR CAST HOLES AND EMBEDDED ITEMS.
 - EF = EACH FACE

EST. WT. OF PRECAST CONCRETE

WINGWALL PWW-10 = 11,955 LB. EA. (6.0 TON)

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION

LATITUDE: 41.88670°N LONGITUDE: 87.69144°W



DESIGNED BY:
FNF/MFB
DRAWN/CHECK BY:
ACB/MFB
UPRR ENGINEER:
DEH/ADS
SHT NO.:
E13 of E43

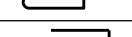
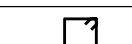
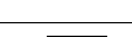
UNION PACIFIC RAILROAD
Office of Director Structures Design

LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION
1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

SHEET TITLE: PRECAST CONCRETE WINGWALL PWW-10 DETAILS

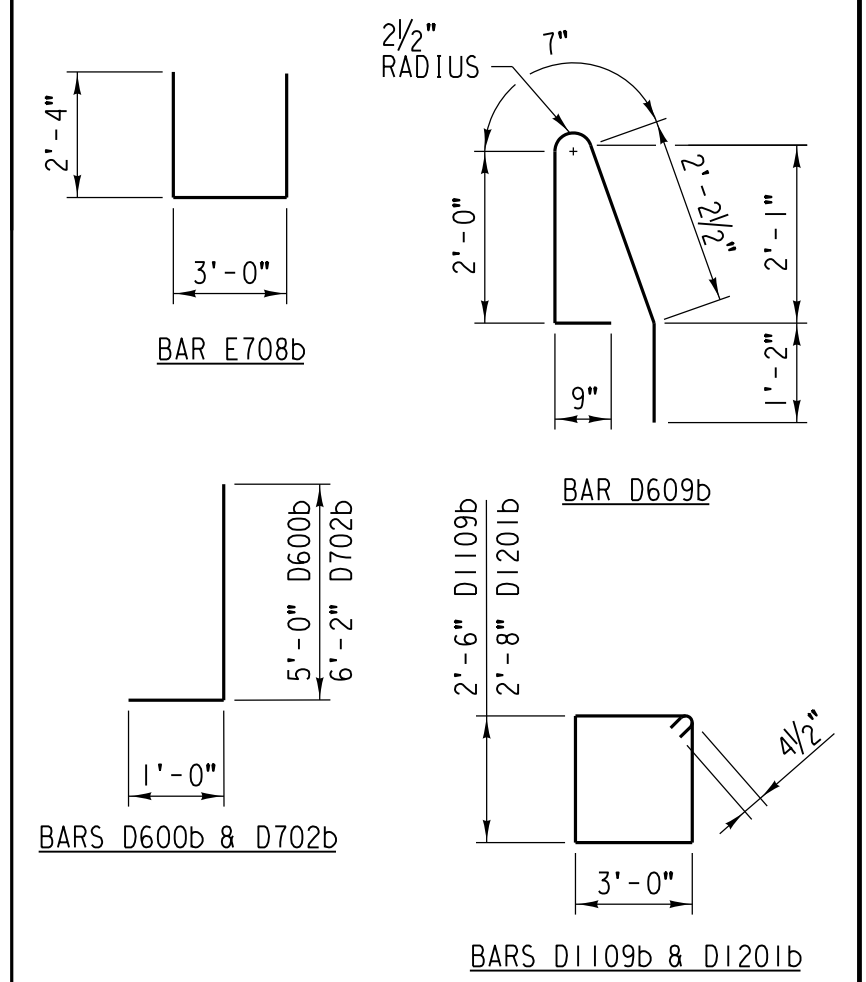
MATERIAL SCHEDULE (INCLUDES BOTH ABUTMENTS)		
REQ'D.	UNIT	DESCRIPTION
28.3	CU. YD.	4000 PSI CAST-IN-PLACE CONCRETE
1	LOT	REINFORCING STEEL
84	EA.	EPOXY GROUTING OF REIN. BARS

QUANTITIES SHOWN FOR INFORMATION ONLY.

REINFORCING SCHEDULE				
(INCLUDES BOTH ABUTMENTS)				
TOTAL	MARK	SIZE	LENGTH	SHAPE
84	D206	#5	2' - 6"	
84	D402	#5	4' - 2"	
48	D504	#5	5' - 4"	
24	D600b	#5	6' - 0"	
32	D609b	#5	6' - 9"	
24	D702b	#5	7' - 2"	
21	D1109b	#5	11' - 9"	
21	D1201b	#5	12' - 1"	
40	D1908	#5	19' - 8"	
16	E708b	#6	7' - 8"	

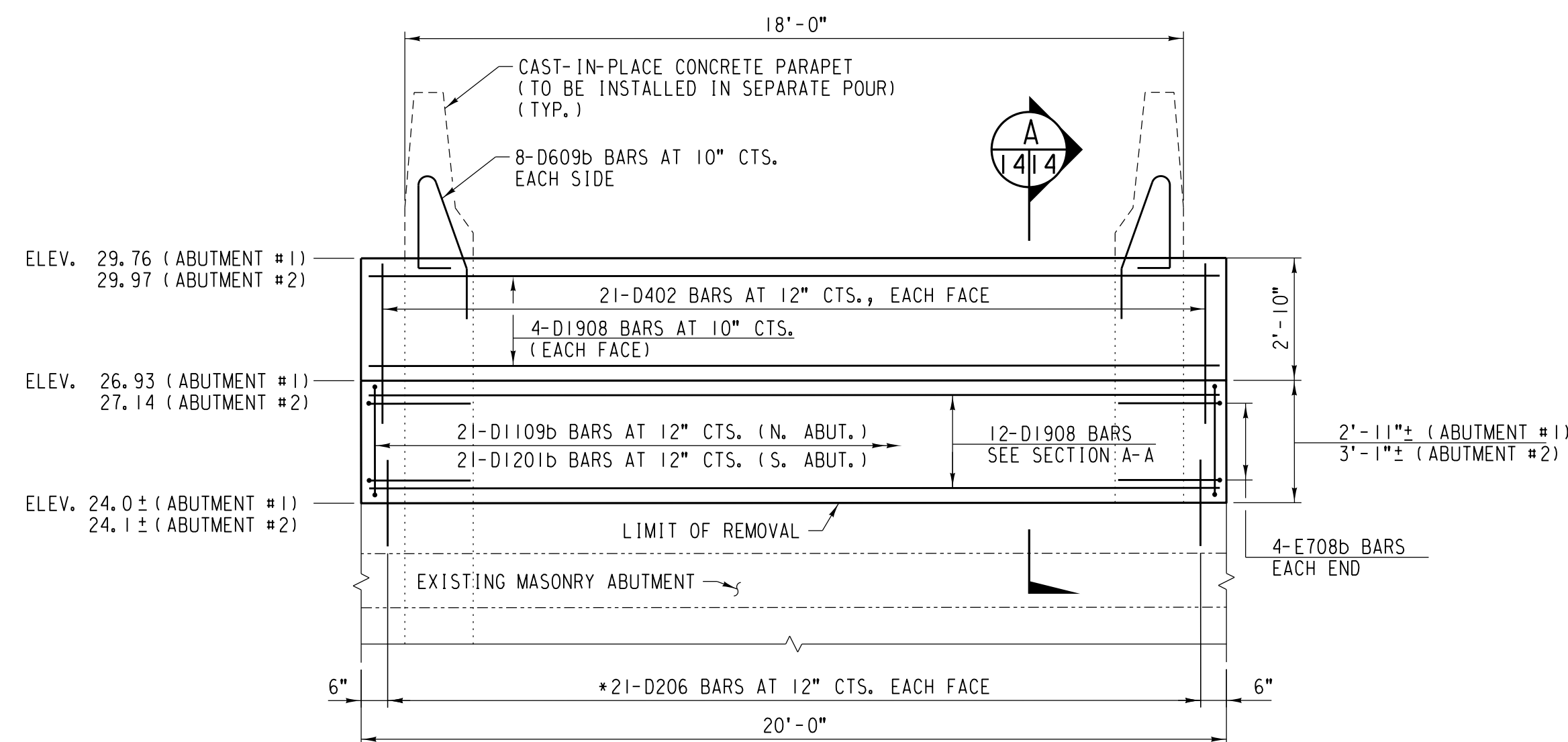
BENDING DIAGRAM

(DIMENSIONS ARE OUT TO OUT)



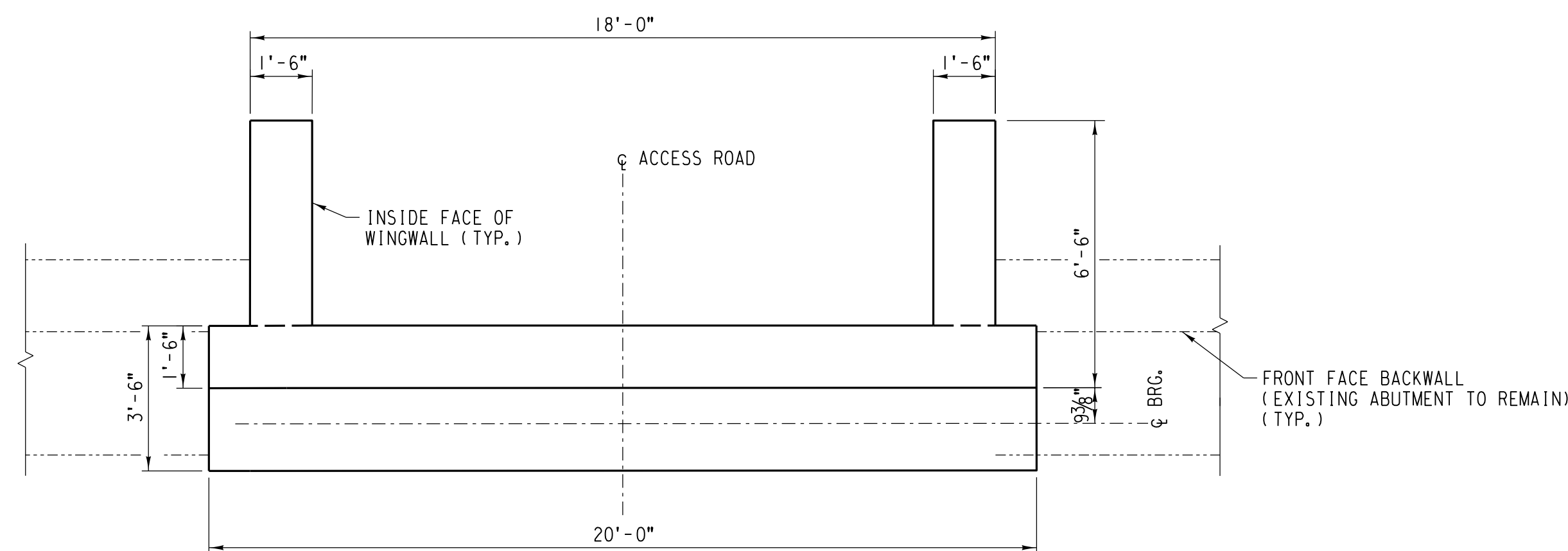
NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "B" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.

EST. WT. OF REINFORCING STEEL = 2,935 LB.



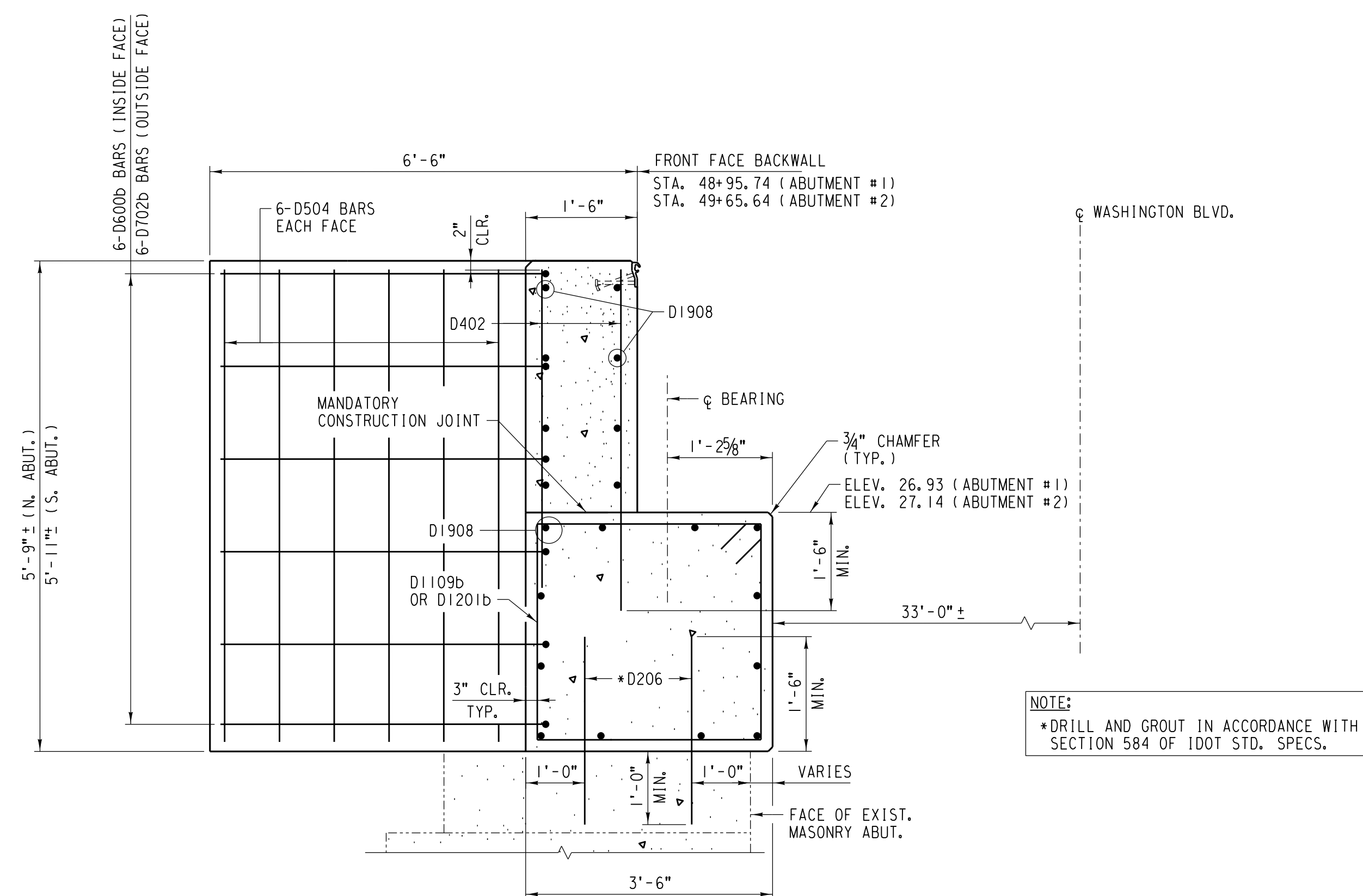
PROPOSED BACKWALL ELEVATION

SCALE: NO SCALE



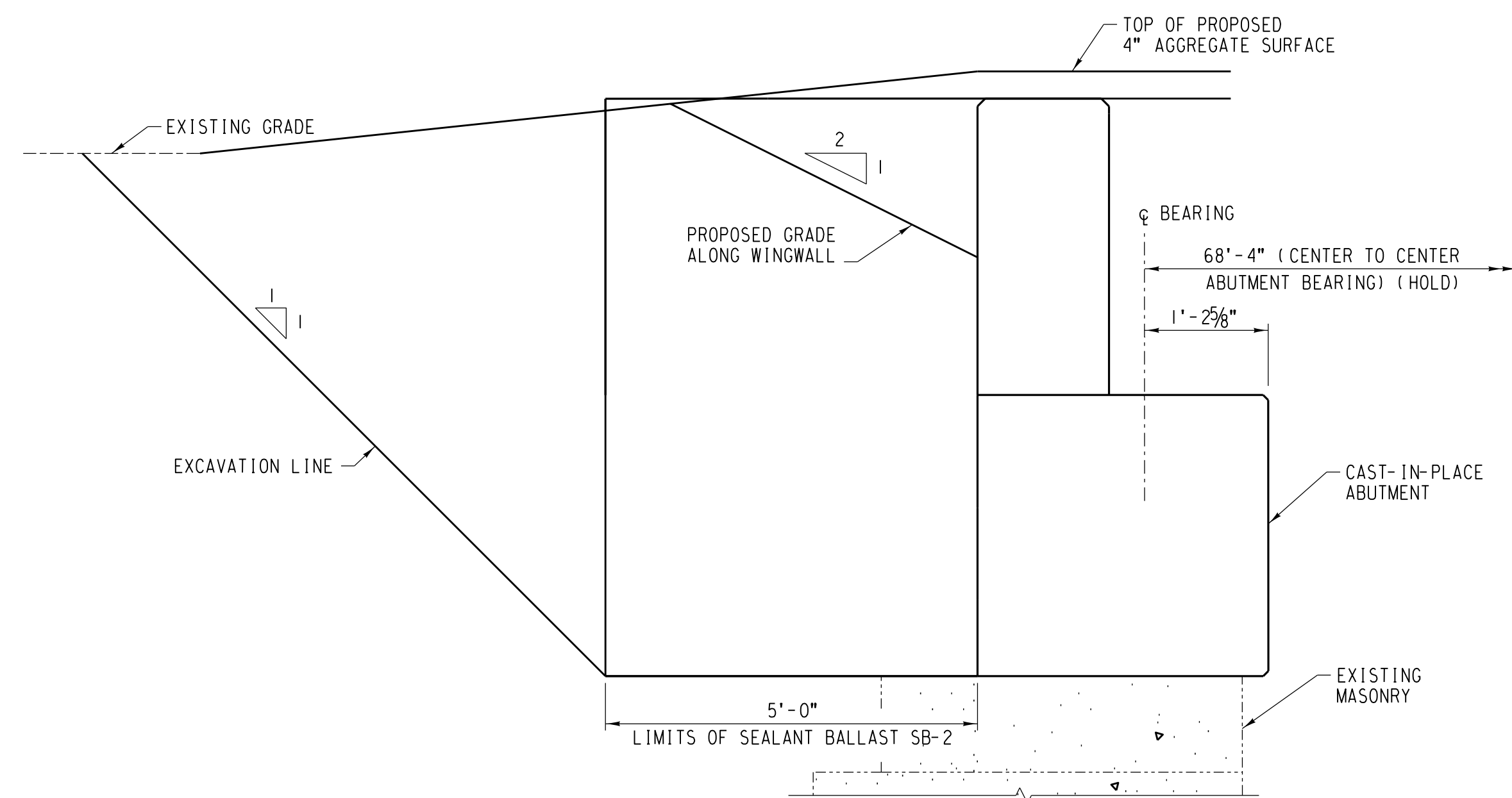
PROPOSED BACKWALL PLAN

SCALE: _____ NO SCALE



SECTION A
SCALE: NO SCALE 1414

SCALE: NO SCALE 1 4 4



EXCAVATION DETAIL

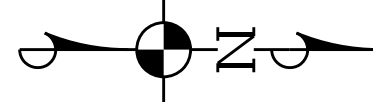
SCALE: _____ NO SCALE

- NOTES:

1. CAST BACKWALL AFTER BEAMS HAVE BEEN ERECTED.
2. D609B BARS SHALL BE INSTALLED WITH BACKWALL AND WINGWALLS.
3. CAST PARAPETS ON BACKWALL AFTER BACKWALL AND WINGWALLS HAVE BEEN ERECTED ALONG WITH BRIDGE PARAPETS.
4. EXISTING MASONRY SURFACE AT REMOVAL LINE SHALL BE CLEANED PRIOR TO POURING NEW ABUTMENT.
5. PARAPET REINFORCEMENT AND DIMENSIONS SHOWN ON SHEET NO. 16.
6. ALL BAR DIMENSIONS ARE OUT TO OUT.
7. COST OF DRILLING D206 BARS SHALL BE INCLUDED WITH EPOXY GROUTING OF REINFORCEMENT BARS.
8. D402 BARS CAST WITH ABUTMENT SEAT.

	/	/	
	/	/	
NO.	DATE	REVISIONS	
COMPLETION STATUS:			
FINAL		05/28/2021	
STATUS		DATE	
 benesch			
APPROVED FOR UNION PACIFIC RAILROAD BY:			
MATTHEW BECKER		05/28/2021	
CONSULTANT ENGINEER		DATE	
PROJECT ID:	WORK ORDER:	C E NUMBER:	
	31876		12255

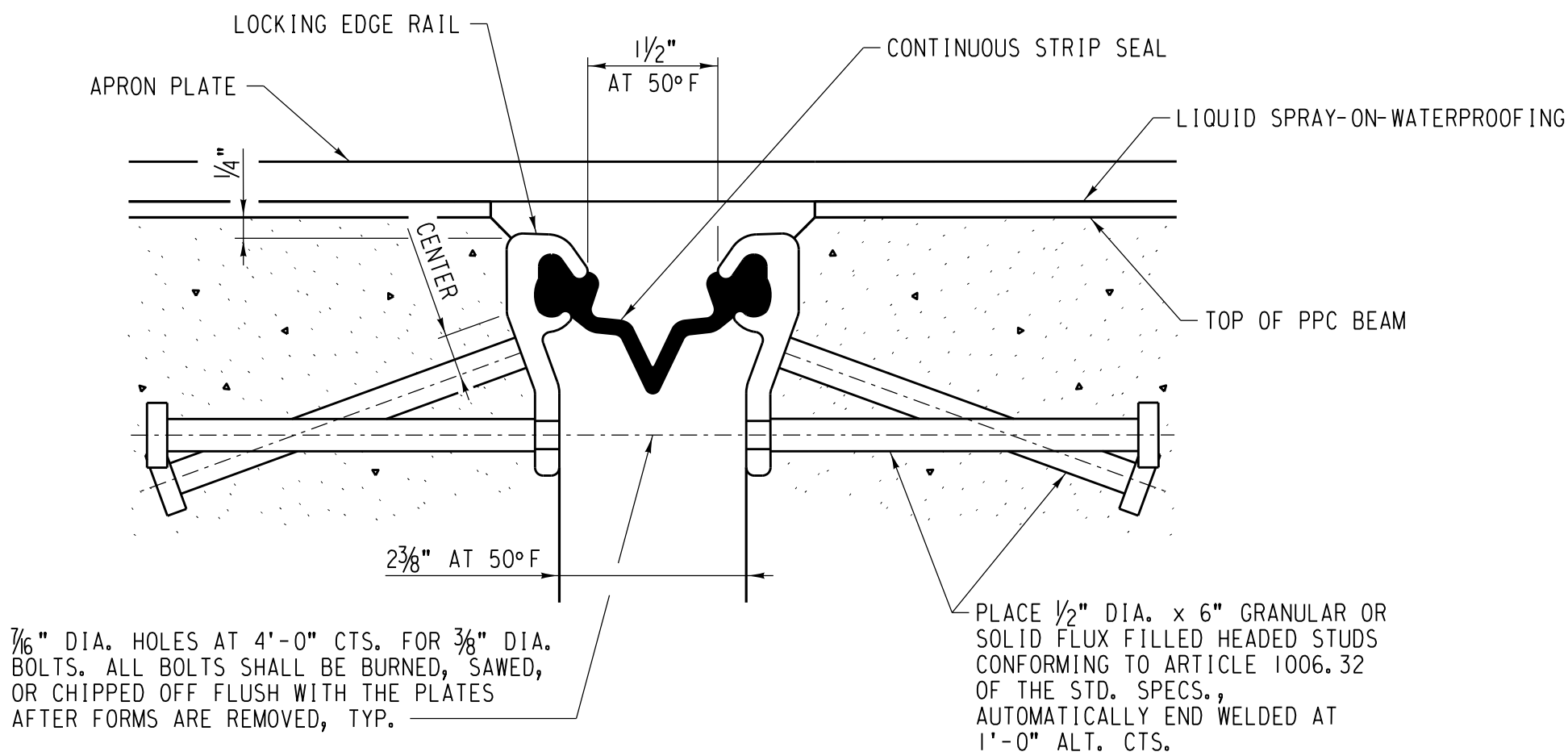
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E14 of E43	SHEET TITLE: ACCESS BRIDGE CAST-IN-PLACE ABUTMENTS			



QUANTITIES SHOWN FOR INFORMATION ONLY.

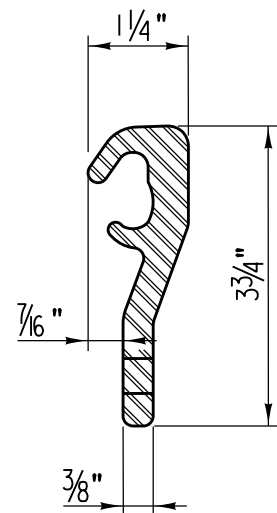
- | | | | | | |
|---|---------------------------|---|--|-----------------------|--|
| FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION | | LATITUDE: 41.88670°N | | LONGITUDE: 87.69144°W | |
|  | DSNCHK BY:
FNF/MFB | UNION PACIFIC RAILROAD
Office of Director Structures Design | | | |
| | DRAWNCHK BY:
ACB/MFB | | | | |
| | UPRR ENGINEER:
DEH/ADS | LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION
1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS) | | | |
| | SHT NO.:
E15 of E43 | SHEET TITLE:
ACCESS BRIDGE PLAN AND CROSS SECTION | | | |

FILE NAME: C:\user\strcm\c2\desk top\roc000931.d11.dgn



SECTION THRU STRIP SEAL JOINT

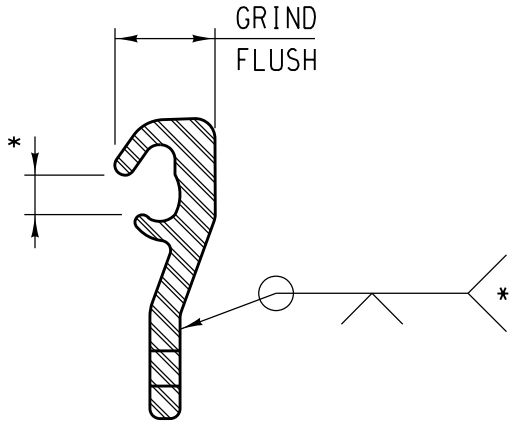
SCALE: NO SCALE



LOCKING EDGE RAIL

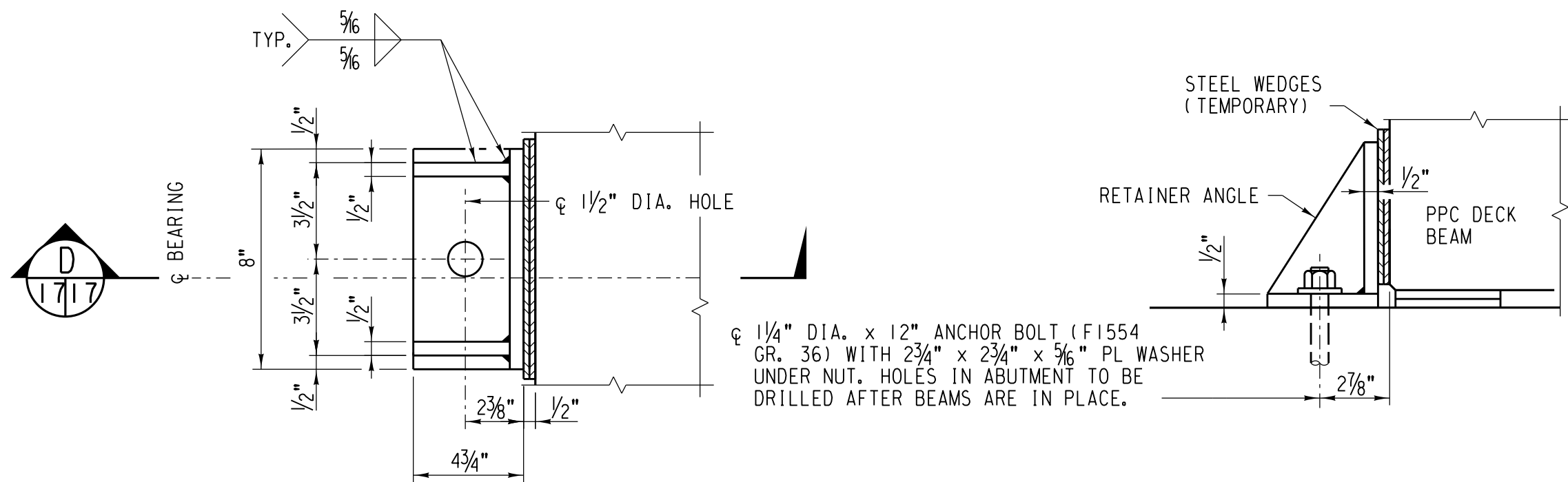
SCALE: NO SCALE

NOTE:
* OMIT WELD AT SEAL OPENING.



LOCKING EDGE RAIL SPLICE

SCALE: NO SCALE



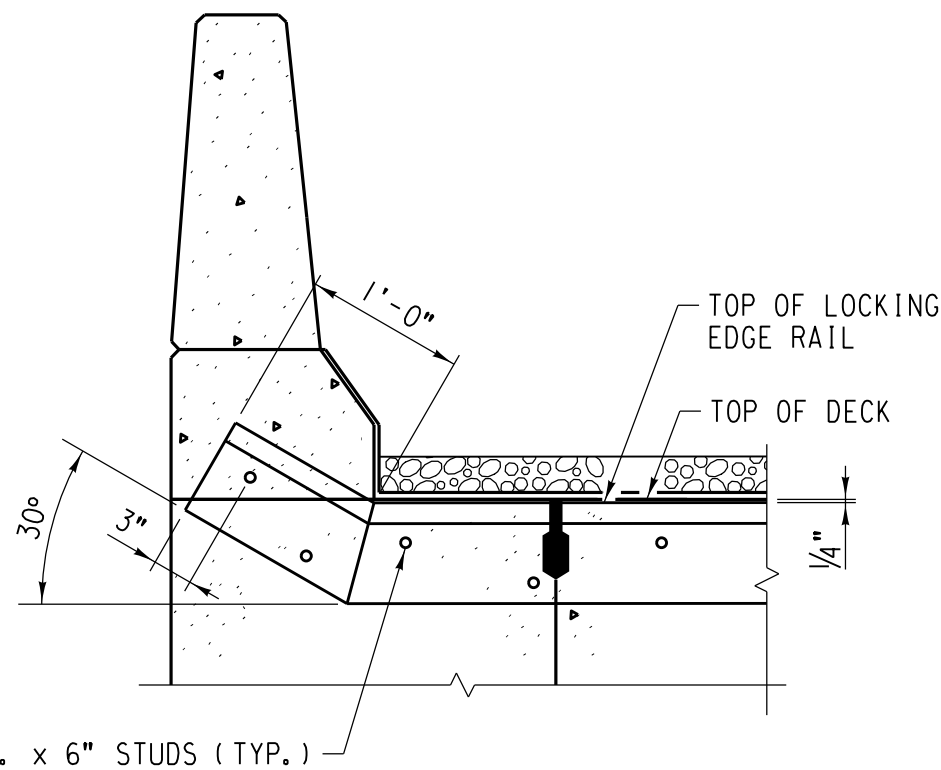
PLAN

SIDE RETAINER ANGLE

SCALE: NO SCALE

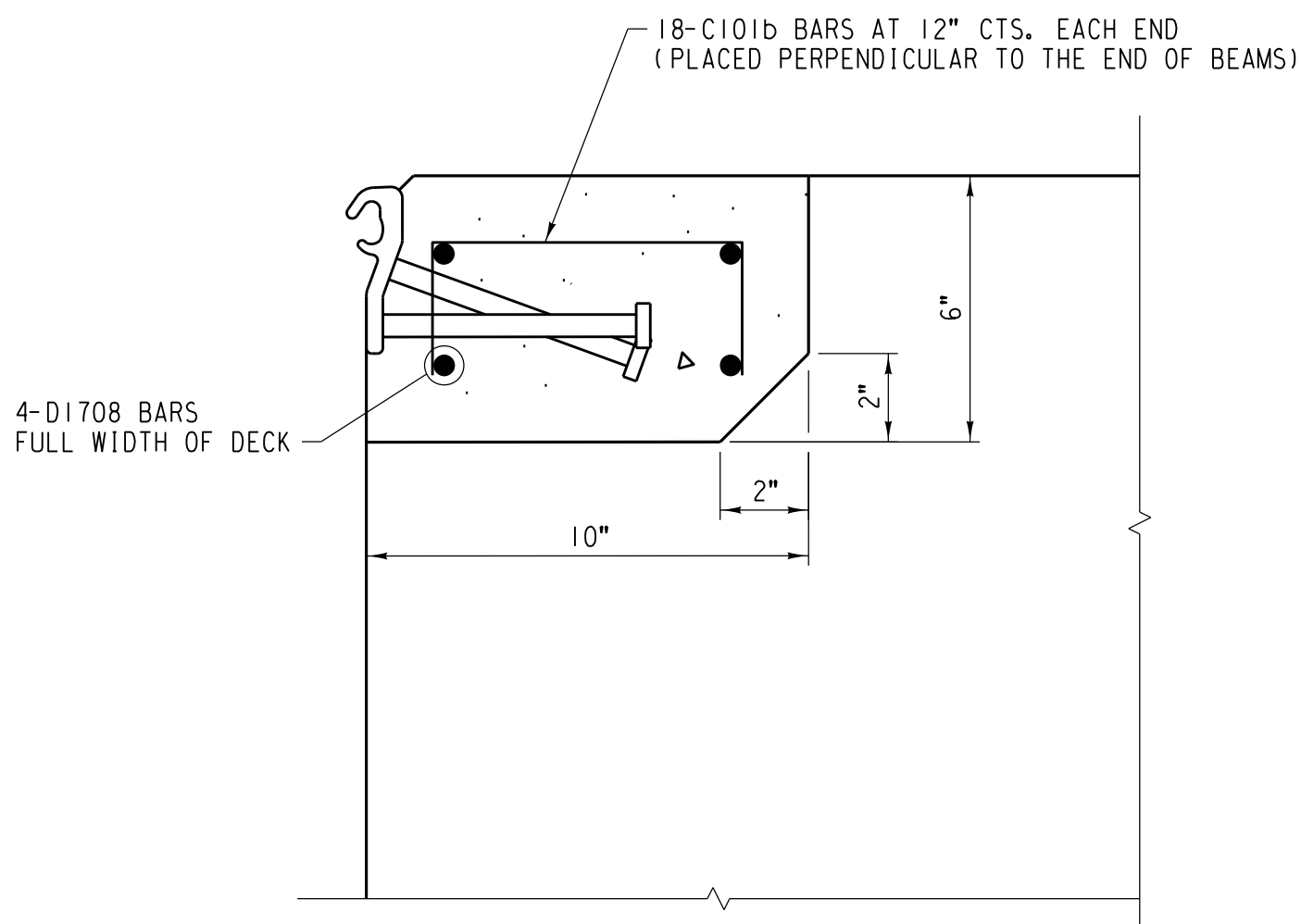
SECTION D

SCALE: NO SCALE



TYPICAL SEAL JOINT END TREATMENTS AT PARAPET

SCALE: NO SCALE



SECTION THRU EXPANSION END BLOCK

SCALE: NO SCALE

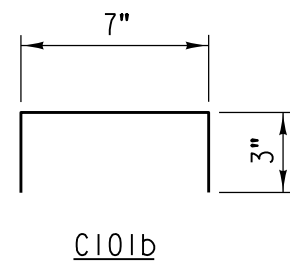
INSERTS AND COIL RODS NOT SHOWN FOR CLARITY.

MATERIAL SCHEDULE		
(QUANTITIES SHOWN ARE FOR BOTH END TREATMENTS)		
REQ'D.	UNIT	DESCRIPTION
0.6	CU. YD.	CAST-IN-PLACE CONCRETE
1	LOT	REINFORCING STEEL
34	L.F.	PREFORMED JOINT STRIP SEAL

REINFORCING SCHEDULE				
(QUANTITIES SHOWN ARE FOR BOTH END TREATMENTS)				
TOTAL	MARK	SIZE	LENGTH	SHAPE
8	D1708	#5	17'-8"	—
36	C101b	#4	1'-1"	□

BENDING DIAGRAM

(DIMENSIONS ARE OUT TO OUT)



NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES ARE REPRESENTED BY THE LETTERS A THROUGH L CORRESPONDING TO BAR SIZE #2 THROUGH #18. BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE LAST TWO DIGITS ARE INCHES.
EST. WT. OF REINFORCING STEEL = 180 LB.

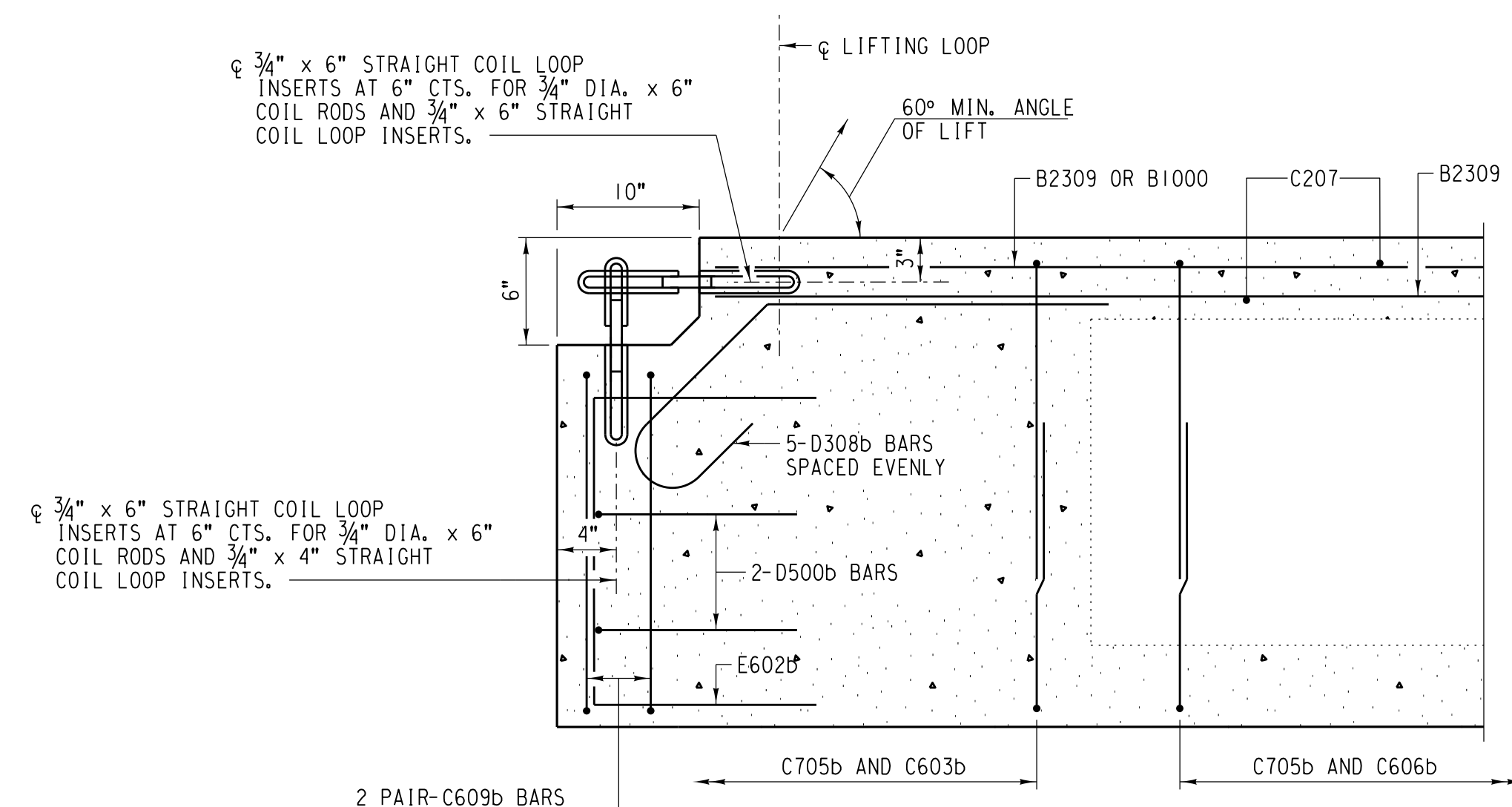
NOTES:

- THE STRIP SEAL SHALL BE MADE CONTINUOUS AND SHALL HAVE A MINIMUM THICKNESS OF 1/4". THE CONFIGURATION OF THE STRIP SEAL SHALL MATCH THE CONFIGURATION OF THE LOCKING EDGE RAILS. OPEN OR "WEBBED" STRIP SEAL GLAND CONFIGURATIONS ARE NOT PERMITTED. THE GLAND SHALL BE SIZED FOR A MAXIMUM RATED MOVEMENT OF 4 INCHES.
- THE LOCKING EDGE RAILS DEPICTED ARE CONCEPTUAL ONLY, EXCEPT FOR THE MINIMUM DIMENSIONS SHOWN. THE ACTUAL CONFIGURATION OF THE LOCKING EDGE RAILS AND MATCHING STRIP SEAL MAY VARY FROM MANUFACTURER TO MANUFACTURER. FLANGED EDGE RAILS WILL NOT BE ALLOWED. THE INSIDE OF THE LOCKING EDGE RAIL GROOVE SHALL BE FREE OF WELD RESIDUE. LOCKING EDGE RAILS MAY BE SPLICED AT SLOPE DISCONTINUITIES.
- THE MANUFACTURER'S RECOMMENDED INSTALLATION METHODS SHALL BE FOLLOWED.
- THE JOINT OPENING AND DECK DIMENSIONS DETAILED ON THE SUPERSTRUCTURE ARE BASED ON A ROLLED RAIL EXPANSION JOINT. IF THE CONTRACTOR ELECTS TO USE THE WELDED RAIL EXPANSION JOINT, THE OPENING AND DECK DIMENSIONS SHALL BE MODIFIED ACCORDINGLY. REQUIRED MODIFICATIONS SHALL BE MADE AT NO ADDITIONAL COST TO UPRR.
- ALL STRIP SEAL EXPANSION JOINT STEEL COMPONENTS SHALL BE GALVANIZED AFTER FABRICATION ACCORDING TO ARTICLE 520.03 OF THE STANDARD SPECIFICATIONS.
- COST OF SIDE RETAINERS AND ACCESSORIES ARE INCLUDED WITH PRECAST PRESTRESSED CONCRETE DECK BEAMS.
- EQUIVALENT ROLLED ANGLE WITH STIFFENERS WILL BE ALLOWED IN LIEU OF WELDED PLATES.
- THE SIDE RETAINERS SHALL BE GALVANIZED AFTER SHOP FABRICATION ACCORDING TO AASHTO M 111 AND ASTM 385.
- ANCHOR BOLTS SHALL BE ASTM F1554 ALL-THREAD (OR AN ENGINEER-APPROVED ALTERNATE MATERIAL) OF THE GRADE AND DIAMETER SPECIFIED. ASTM A307 GRADE C ANCHOR BOLTS MAY BE USED IN LIEU OF ASTM F1554 GRADE 36 (Fy = 36 KSI). THE CORRESPONDING SPECIFIED GRADE OF AASHTO M314 ANCHOR BOLTS MAY BE USED IN LIEU OF ASTM F1554. ANCHOR BOLTS AND PLATE WASHERS SHALL BE GALVANIZED ACCORDING TO AASHTO M 232.
- DRILLED AND SET ANCHOR BOLTS FOR SIDE RETAINERS SHALL BE INSTALLED ACCORDING TO ARTICLE 521.06 OF THE STANDARD SPECIFICATIONS.
- AFTER THE OVERLAY IS PLACED AND THE DECK PARAPETS ARE POURED AND CURED, THE STEEL WEDGES SHALL BE REMOVED.

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

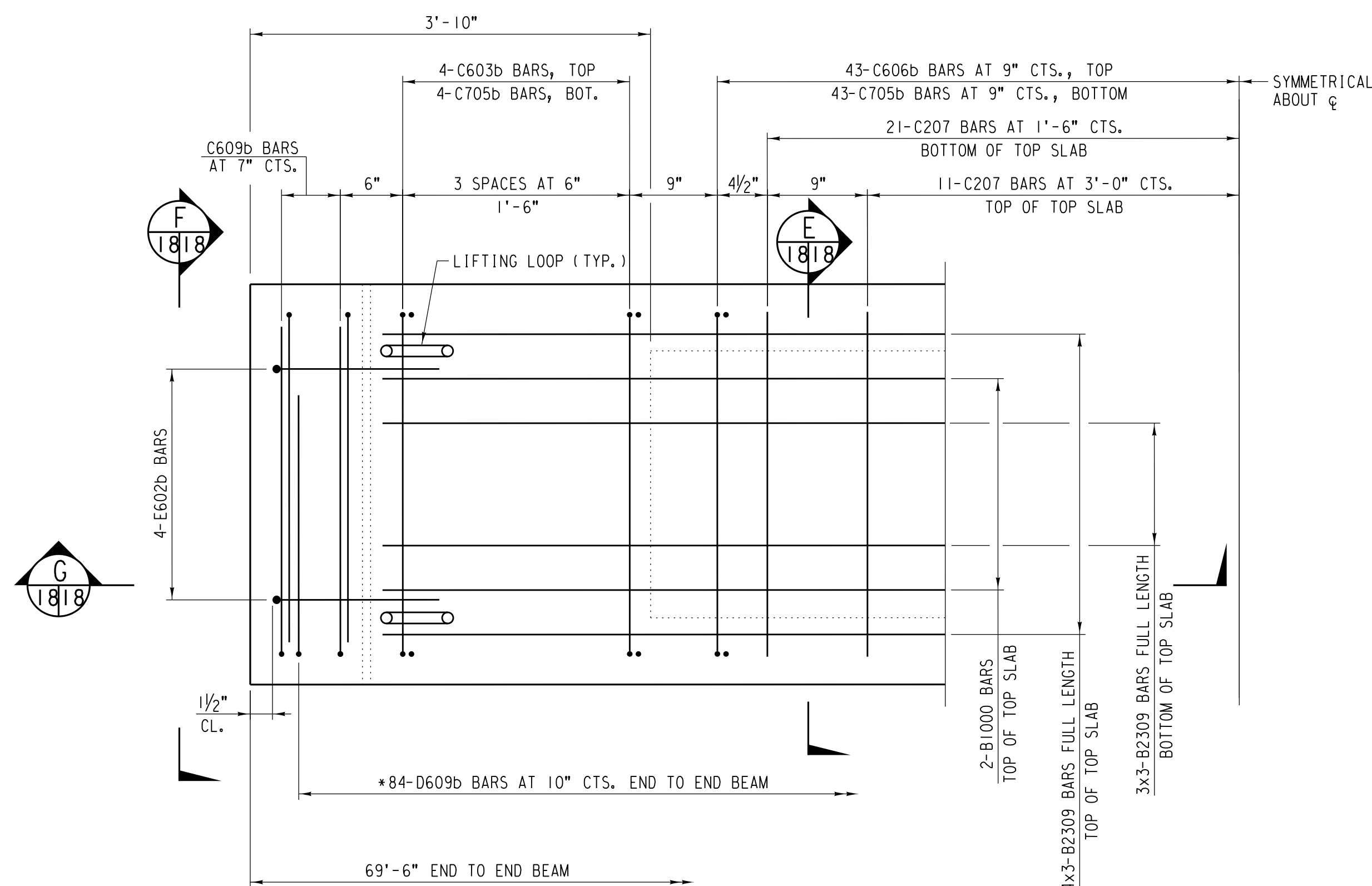
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSNCHK BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHK BY:	Office of Director Structures Design	
	ACB/MFB	LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E17 of E43		SHEET TITLE:	
		ACCESS BRIDGE EXPANSION JOINT DETAILS	

MATERIAL SCHEDULE		
ITEM	UNIT	TOTAL
36" x 33" PRECAST PRESTRESSED CONCRETE DECK BEAMS	S.F.	1,251
1" x 9" x 1'-1 1/2" FABRIC BEARING PAD	EACH	4
1" x 9" x 2'-3" FABRIC BEARING PAD	EACH	10
1" DIA. x 2'-6" DOWEL RODS	EACH	12
TRANSVERSE TIE DIAPHRAGM	EACH	2



SECTION C
SCALE: NO SCALE 1818

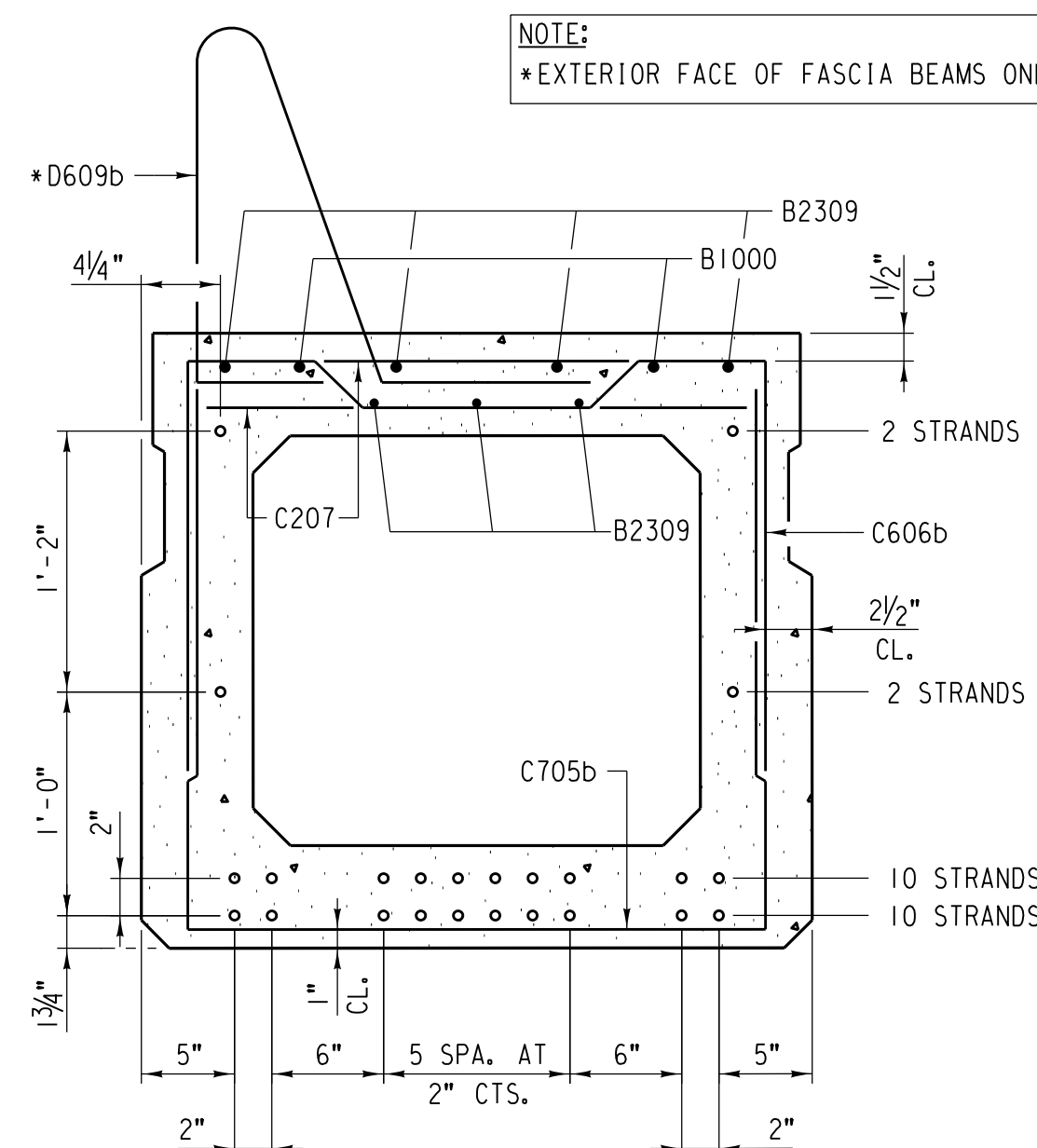
- NOTES:**
1. 1½" CL. FOR REINFORCEMENT BARS UNLESS OTHERWISE NOTED.
 2. COST OF INSERTS AND COIL RODS INCLUDED WITH PRECAST PRESTRESSED CONCRETE DECK BEAMS.



PLAN VIEW

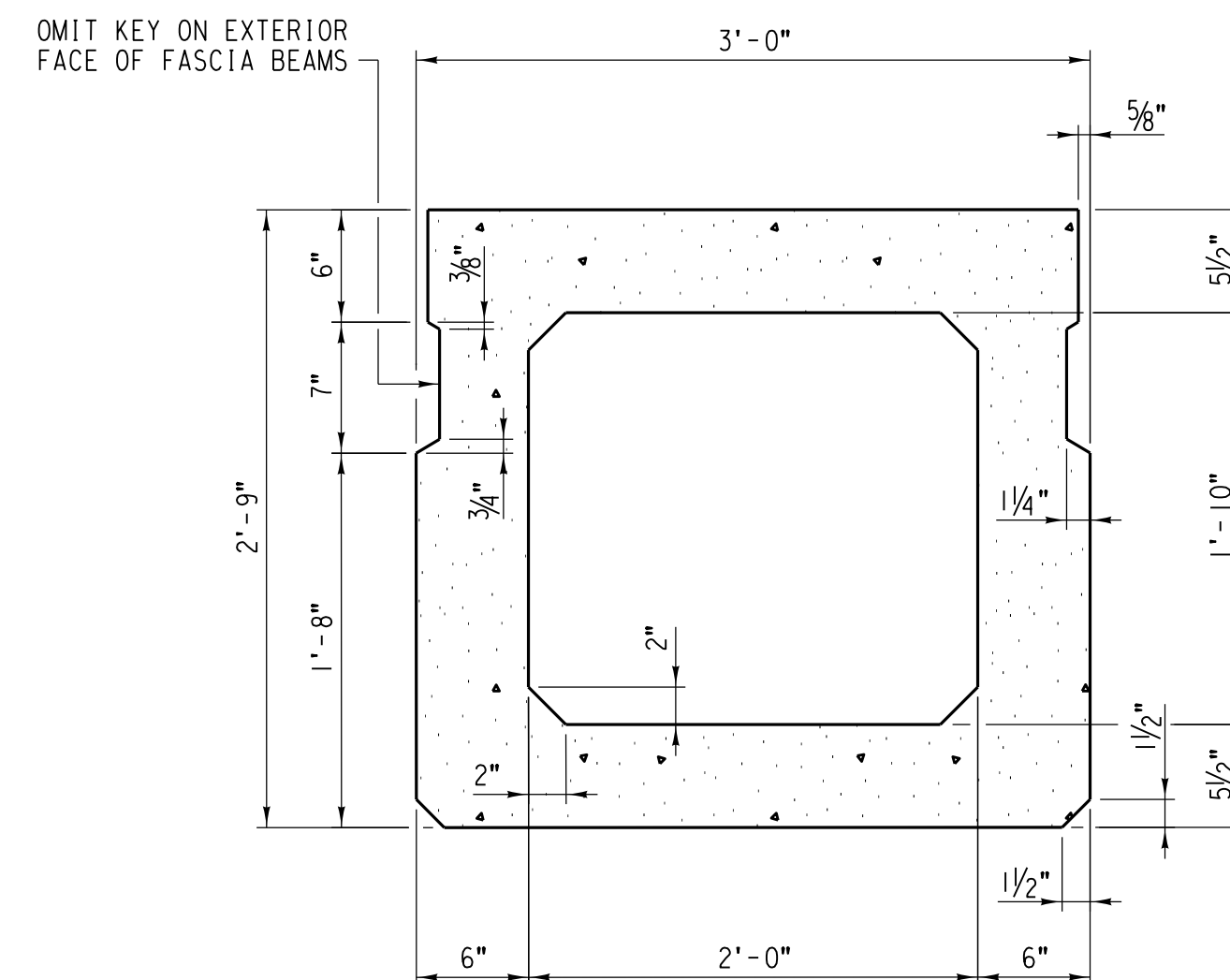
SCALE: NO SCALE

- NOTES:**
1. SPACING OF C705b AND C603b BARS MAY BE ADJUSTED UP TO 4" IN THE IMMEDIATE AREA OF THE TRANSVERSE TIE DIAPHRAGMS TO MISS THE BLOCK OUTS FOR THE TRANSVERSE TIES.
 2. INSERTS AND COIL RODS NOT SHOWN FOR CLARITY. SEE SECTION G.
 3. SEE SHEET 19 FOR DOWEL ROD CAST HOLE LOCATIONS.



SECTION E
SCALE: NO SCALE 1818

INCLUDES 24 - 1/2" DIA. 270 KSI STRANDS PER BEAM



SECTION E
SCALE: NO SCALE 1818

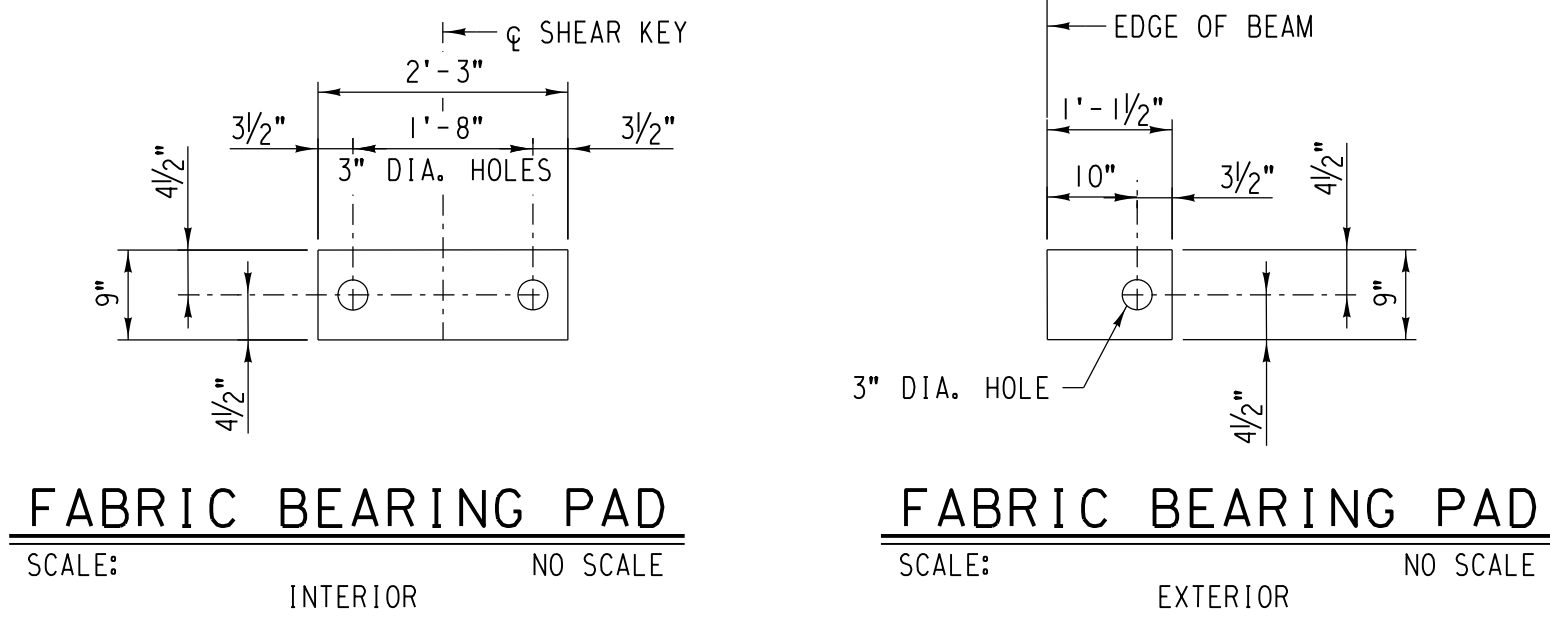
SHOWING DIMENSIONS

- NOTE:**
- | | |
|------------------|----------------|
| MINIMUM BAR LAP: | #3 BAR = 1'-6" |
| | #4 BAR = 2'-0" |
| | #5 BAR = 2'-6" |

	/ /				
NO.	DATE	REVISIONS			
COMPLETION STATUS:					
FINAL					
STATUS		05/28/2021 DATE			
 benesch					
APPROVED FOR UNION PACIFIC RAILROAD BY:					
MATTHEW BECKER					
CONSULTANT ENGINEER		05/28/2021 DATE			
PROJECT ID:		WORK ORDER:		C/E NUMBER:	
		31876		122525	

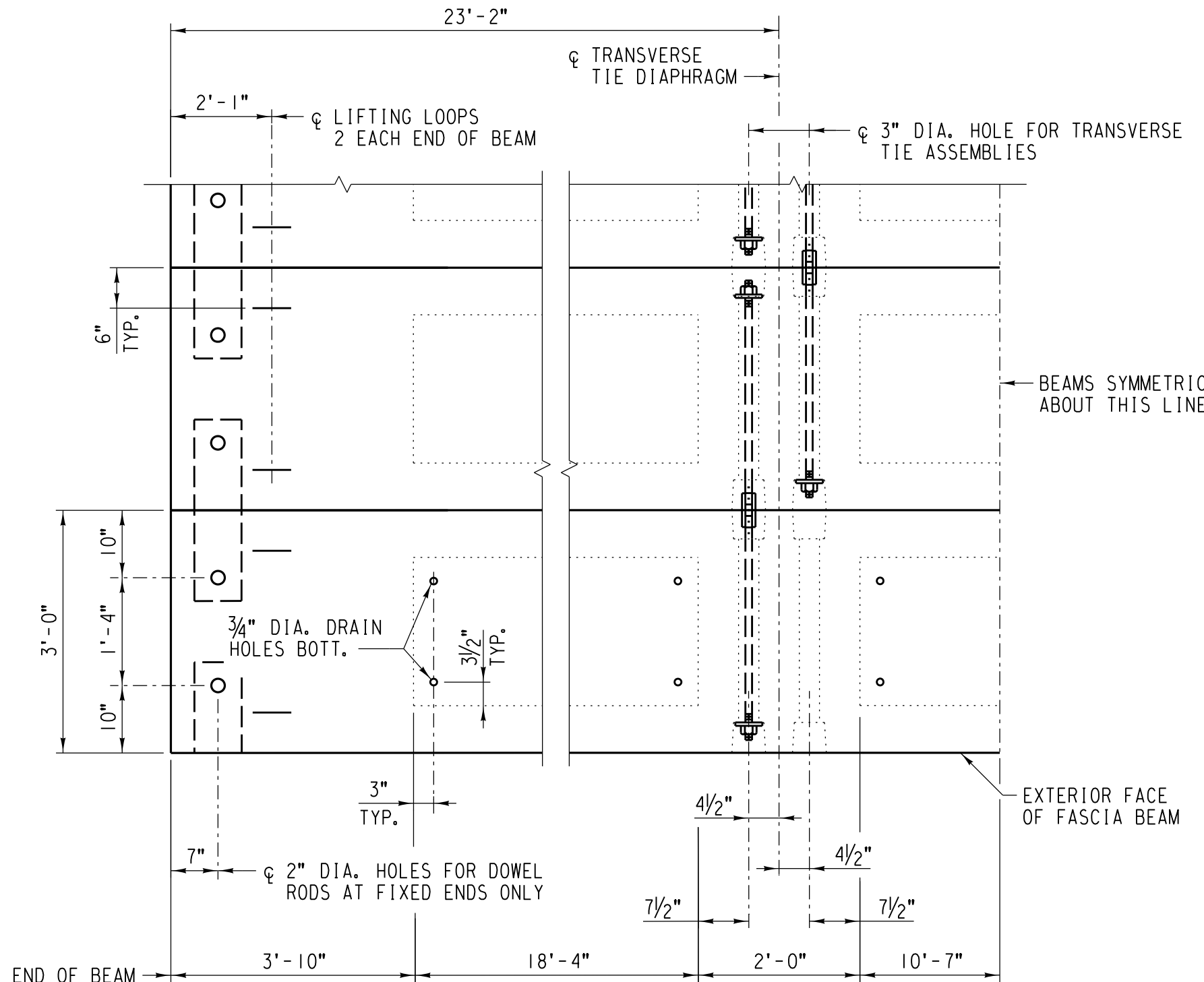
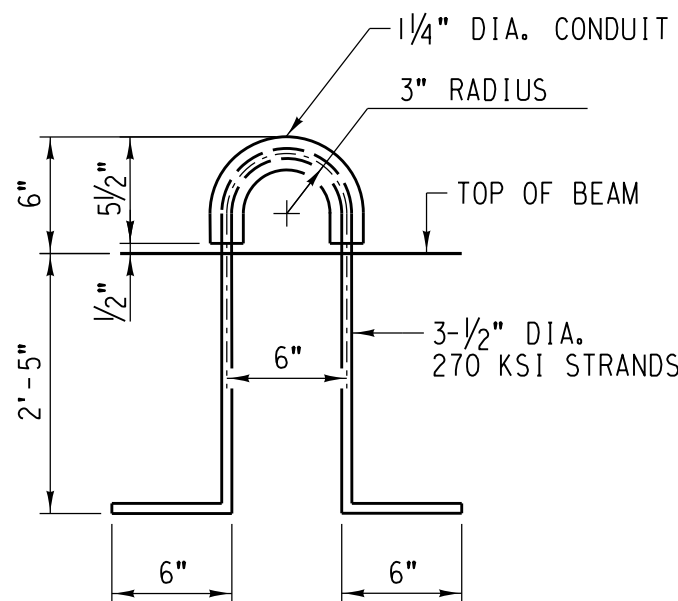
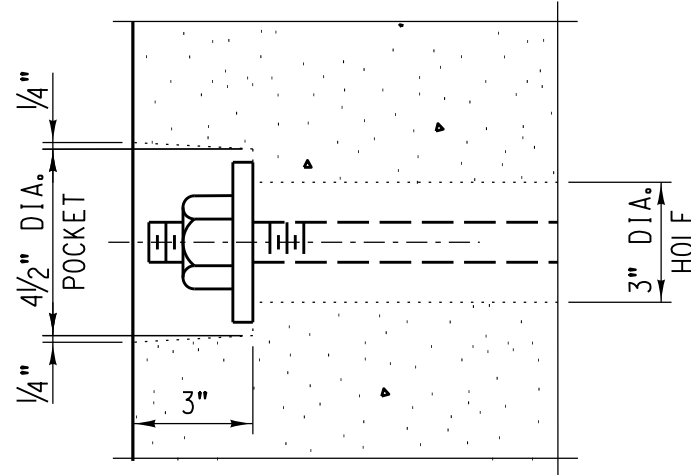
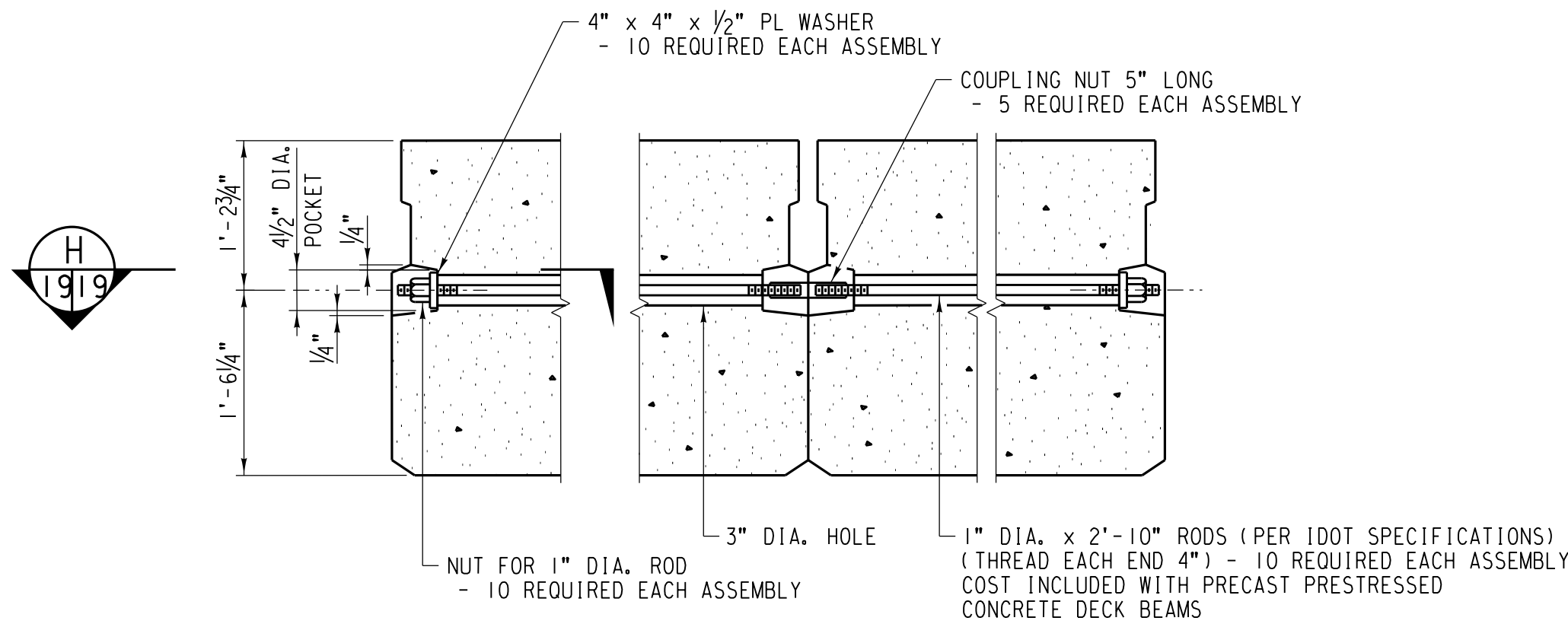
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: ACB/MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E18 of E43	SHEET TITLE: ACCESS BRIDGE REINFORCEMENT DETAILS (SHEET 1 OF 2)			

FILE NAME: C:\user\stcr\mfr\ez\Bask\ap\ro000931.d11.dgn



FIXED
SCALE: NO SCALE

NOTE:
ALL BEARING PADS SHALL BE 1" THICK.
OMIT HOLES WHEN USING EXPANSION BEARINGS.
EXPANSION BEARING PAD SHALL BE BONDED TO THE SUBSTRUCTURE.



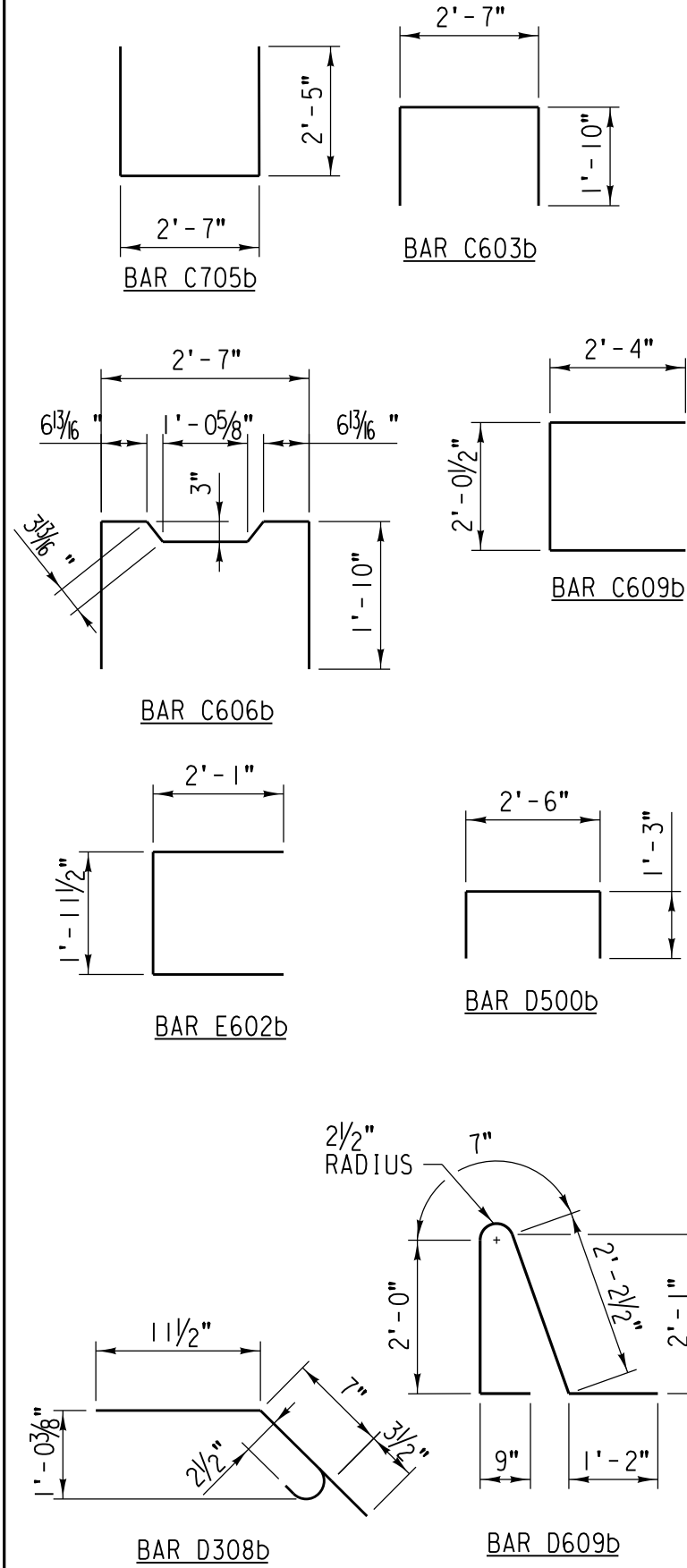
NOTE:
CONNECT BEAMS IN PAIRS WITH THE
TRANSVERSE TIE DIAPHRAGM CONFIGURATION SHOWN.

NOTE:
*EXTERIOR FACE OF FASCIA BEAMS ONLY

REINFORCING SCHEDULE (QUANTITY FOR ONE BEAM ONLY)				
TOTAL	MARK	SIZE	LENGTH	SHAPE
4	B1000	#3	10'-0"	—
21	B2309	#3	23'-9"	—
64	C207	#4	2'-7"	—
8	C603b	#4	6'-3"	┌┐
86	C606b	#4	6'-6"	┌┐
8	C609b	#4	6'-9"	┌┐
94	C705b	#4	7'-5"	┌┐
10	D308b	#5	3'-8"	┌┐
4	D500b	#5	5'-0"	┌┐
168	*D609b	#5	6'-9"	┌┐
8	E602b	#6	6'-2"	┌┐

COST OF REINFORCEMENT BARS INCLUDED IN PRECAST
PRESTRESSED CONCRETE DECK BEAMS.

BENDING DIAGRAM
(DIMENSIONS ARE OUT TO OUT)

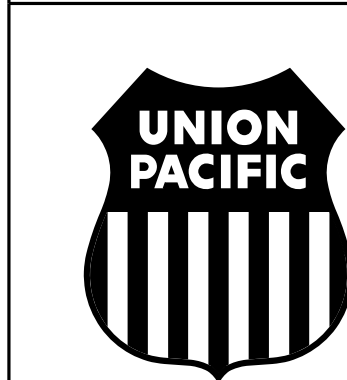


NOTE:
BAR DESIGNATIONS CONSIST OF BAR SIZE & LENGTH
FOLLOWED BY THE LETTER "b" IF BENT. BAR SIZES
ARE REPRESENTED BY THE LETTERS A THROUGH L
CORRESPONDING TO BAR SIZE #2 THROUGH #18.
BAR LENGTHS ARE GIVEN IN FEET AND INCHES; THE
LAST TWO DIGITS ARE INCHES.

NOTES:

- PRESTRESSING STEEL SHALL BE UNCOATED HIGH STRENGTH, LOW RELAXATION 7-WIRE STRAND, ASTM A416 GRADE 270. THE NOMINAL DIAMETER SHALL BE 1/2" AND THE NOMINAL CROSS-SECTIONAL AREA SHALL BE 0.153 SQ. IN.
- THE 1" DIA. RODS IN THE TRANSVERSE TIE ASSEMBLY SHALL BE TIGHTENED TO A SNUG FIT AND THE THREADS SET. POCKETS ON EXTERIOR FACES OF BRIDGE SHALL BE FILLED WITH GROUT AFTER TRANSVERSE TIE ASSEMBLY IS IN PLACE.
- REINFORCEMENT BARS SHALL CONFORM TO ASTM A 706, GRADE 60.
- TWO 1/8" FABRIC ADJUSTING SHIMS OF THE DIMENSIONS OF THE EXTERIOR BEARING PAD SHALL BE PROVIDED FOR EACH BEARING PAD LOCATION.
- FABRIC BEARING PADS SHALL CONFORM TO IDOT STANDARD SPECIFICATION 1082.01.
- EACH BEAM SHALL HAVE FOUR LIFTING LOOPS, TWO CAST IN EACH END. LOOPS SHALL BE BURNED OFF AFTER BEAMS HAVE BEEN ERECTED.
- A MINIMUM 2 1/2" DIA. LIFTING PIN SHALL BE USED TO ENGAGE THE LIFTING LOOPS DURING HANDLING.
- CORROSION INHIBITOR, PER ARTICLE 1020.05(B)(12) AND 1021.06 OF THE STANDARD SPECIFICATIONS, SHALL BE USED IN THE CONCRETE FOR PRECAST PRESTRESSED CONCRETE DECK BEAMS.
- COMPRESSIVE STRENGTH OF PRESTRESSED CONCRETE, f'c, SHALL BE 6000 PSI.
- COMPRESSIVE STRENGTH OF PRESTRESSED CONCRETE AT RELEASE, f'ci, SHALL BE 5000 PSI.
- KEYWAY SURFACES SHALL BE CLEANED TO REMOVE FORM OIL OR OTHER BOND BREAKING MATERIAL PRIOR TO SHIPMENT OF THE BEAMS. CLEANING SHALL BE DONE BY SANDBLASTING INTO THE KEYWAY AREAS BETWEEN TOP OF THE BEAM AND THE BOTTOM EDGE OF THE BEAM.
- ALL BAR DIMENSIONS ARE OUT TO OUT.

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION



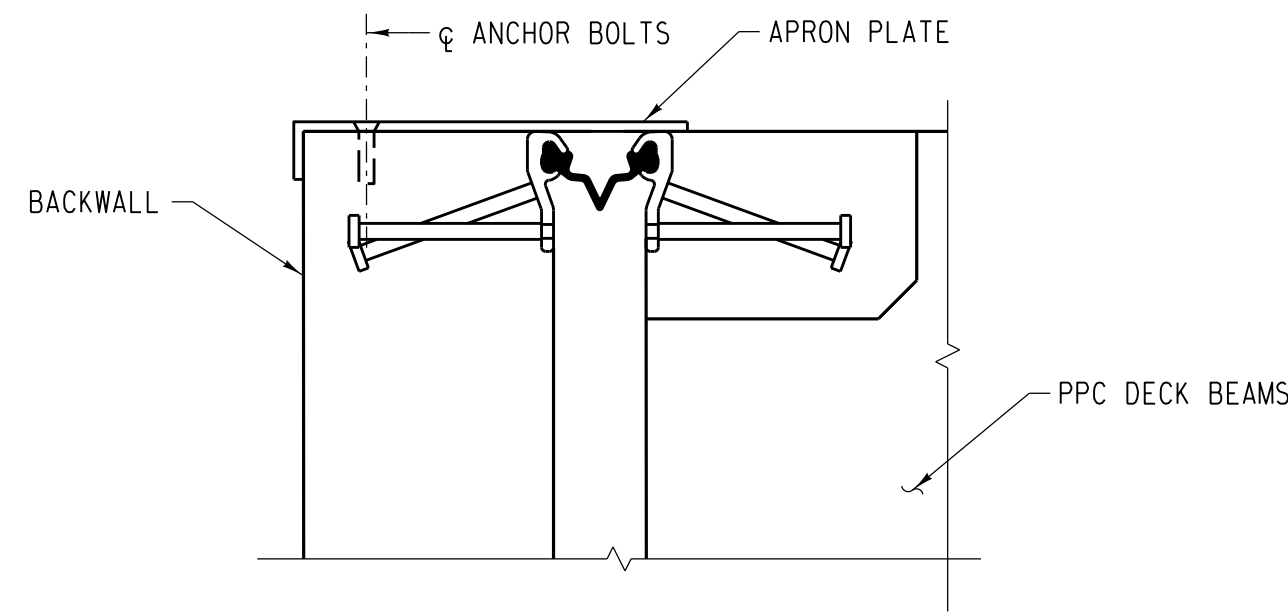
DSNCHK BY:
FNF/MFB
DRAWN/CHK BY:
ACB/MFB
UPRR ENGINEER:
DEH/ADS
SHT NO:
E19 of E43

UNION PACIFIC RAILROAD
Office of Director Structures Design

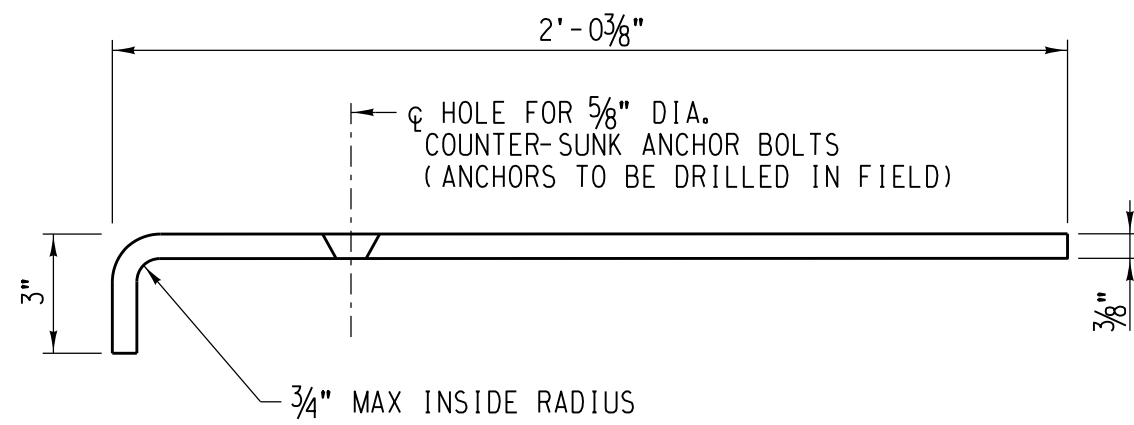
LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION
1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

SHEET TITLE: ACCESS BRIDGE REINFORCEMENT DETAILS (SHEET 2 OF 2)

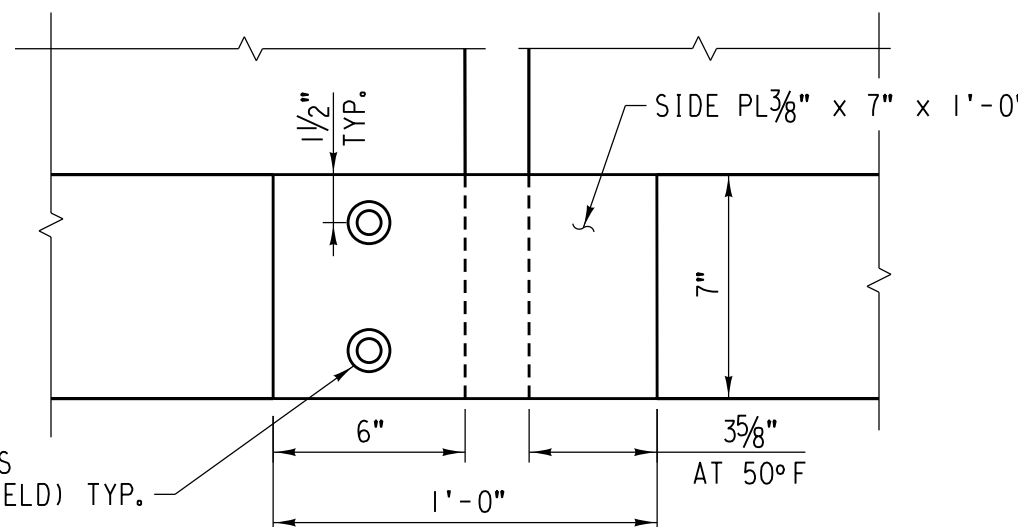
LATITUDE: 41.88670°N LONGITUDE: 87.69144°W



SECTION J
SCALE: NO SCALE



SECTION L
SCALE: NO SCALE

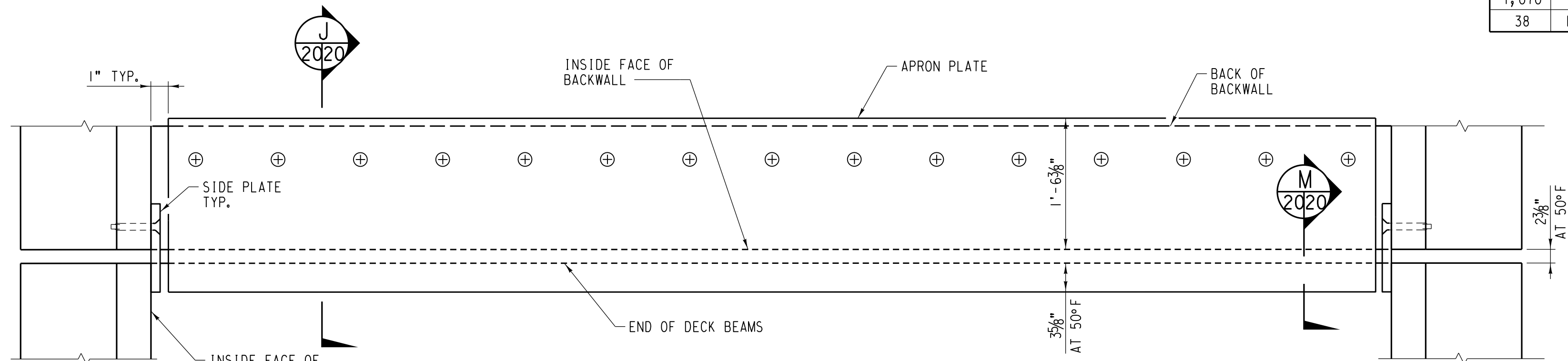


VIEW M
SCALE: NO SCALE

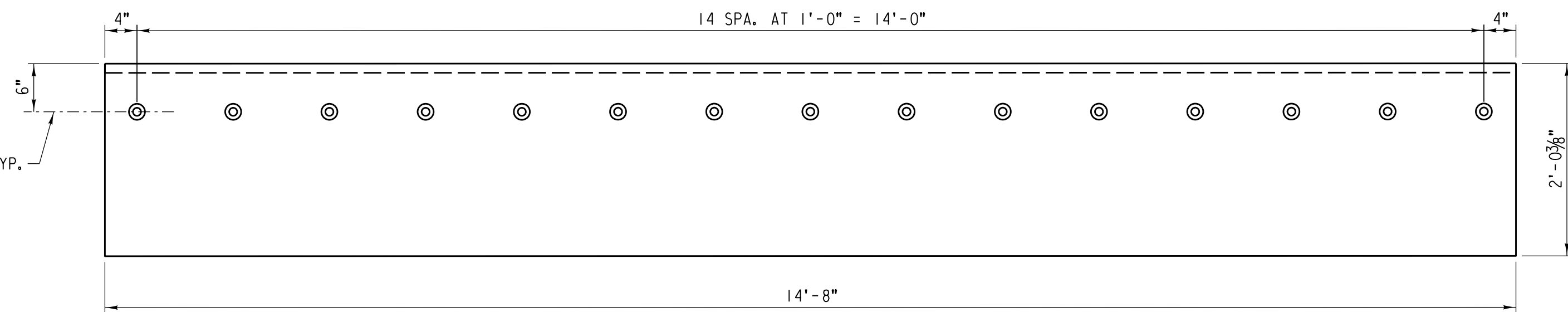
3/4" DIA. HOLES FOR
5/8" DIA. COUNTER-SUNK ANCHORS
(ANCHORS TO BE DRILLED IN FIELD) TYP.

NOTES:

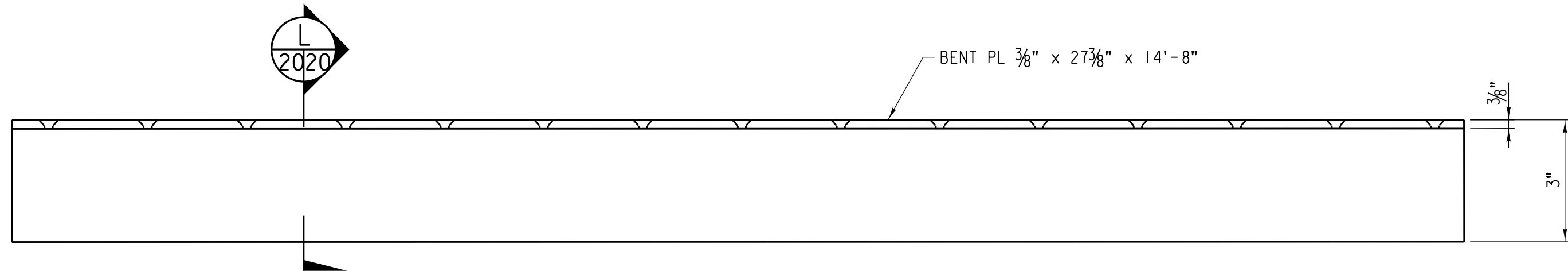
1. STEEL PLATES TO BE ASTM A36, GALVANIZED PER ASTM A123.
2. APRON PLATES WILL BE CONNECTED TO ABUTMENT BACKWALL BY COUNTER-SUNK BOLTS PLACED IN THE FIELD AT LOCATIONS SHOWN ON THIS SHEET. PLACE APRON PLATES SUCH THAT SHORT LEG OF PLATE IS FLUSH WITH BACK OF BACKWALL AS SHOWN ON THIS SHEET.
3. COUNTER-SUNK BOLTS SHALL BE INSTALLED WITH NON-SHRINK CEMENTITIOUS GROUT.
4. COST OF DRILLING & INSTALLING ANCHORS IS INCLUDED IN THE COST OF CAST-IN-PLACE CONCRETE.
5. LIQUID SPRAY-ON DECK WATERPROOFING SHALL BE APPLIED TO DECK, PARAPETS AND TOP FACE OF BACKWALL PRIOR TO INSTALLATION OF APRON PLATES AND SIDE PLATES.



PLAN OF APRON PLATE AT JOINT
SCALE: NO SCALE



PLAN



ELEVATION

APRON PLATE
SCALE: NO SCALE

MATERIAL SCHEDULE

(QUANTITIES SHOWN ARE FOR BOTH ENDS)

REQ'D.	UNIT	DESCRIPTION
1,070	LB.	STRUCTURAL STEEL FOR APRON PLATE AND SIDE PLATES
38	EACH	5/8" DIA. 5" COUNTER-SUNK BOLTS

NO.	DATE	REVISIONS
1	05/28/2021	FINAL
2	05/28/2021	FINAL
3	05/28/2021	FINAL
4	05/28/2021	FINAL
5	05/28/2021	FINAL
6	05/28/2021	FINAL
7	05/28/2021	FINAL
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100	05/28/2021	FINAL

PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		
LATITUDE: 41.88670°N LONGITUDE: 87.69144°W		
UNION PACIFIC RAILROAD		
Office of Director Structures Design		
LOCATION & DESCRIPTION:		
BRIDGE 0.93 ROCKWELL SUBDIVISION		
REPLACING 1 SPAN TPG x 90'		
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)		
SHEET TITLE:		
ACCESS BRIDGE MISCELLANEOUS DETAILS		



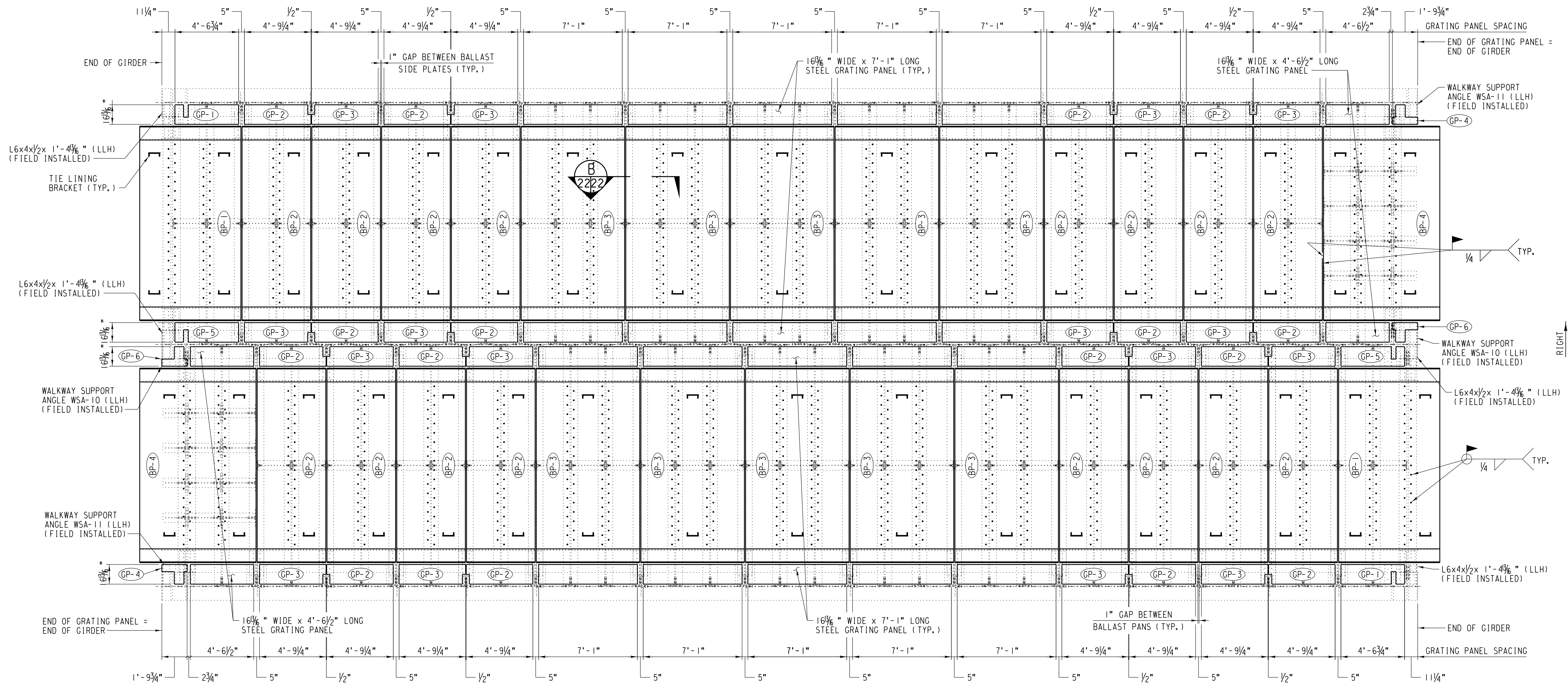
BOTTOM FLANGE OF SIDE GIRDER SHOWN,
BOTTOM FLANGE OF COMMON GIRDER SIMILAR

1. FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
2. FCM = FRACTURE CRITICAL MEMBER

	/ /	
	/ /	
NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		<u>05/28/2021</u>
STATUS		DATE
 benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		<u>05/28/2021</u>
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C E NUMBER:
	31876	122525

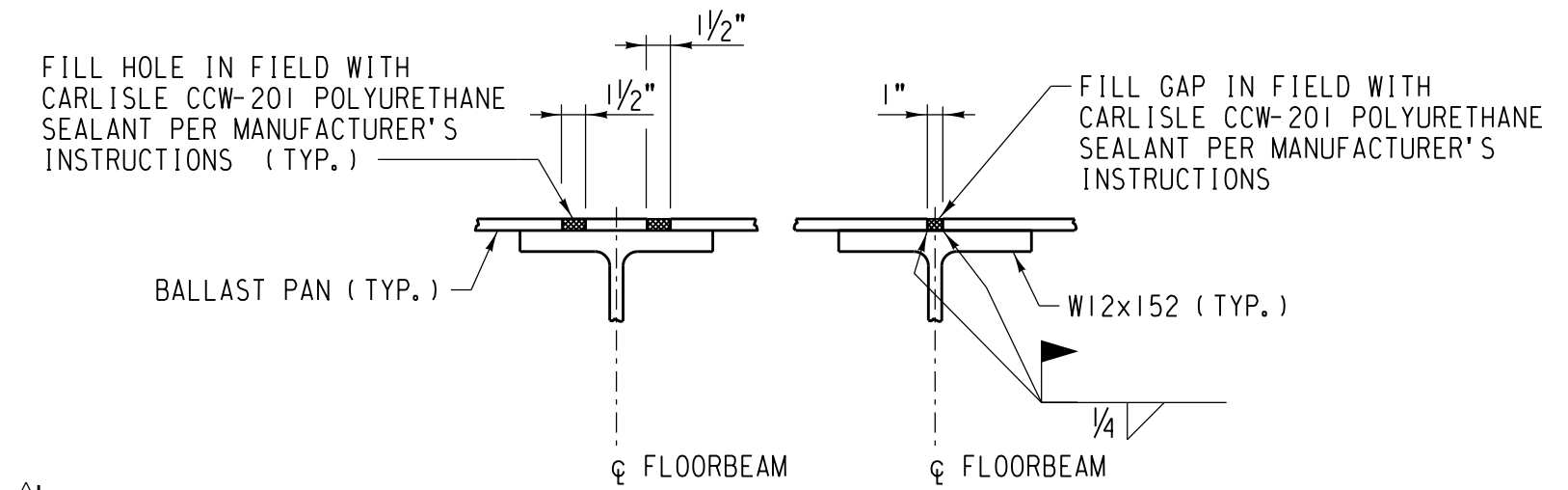
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: RR /MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E21 of E43	SHEET TITLE: TPG FRAMING PLAN			

FILE NAME: C:\user\gfr\cmr\ez\Bask\tp\ro000931_b11.dgn



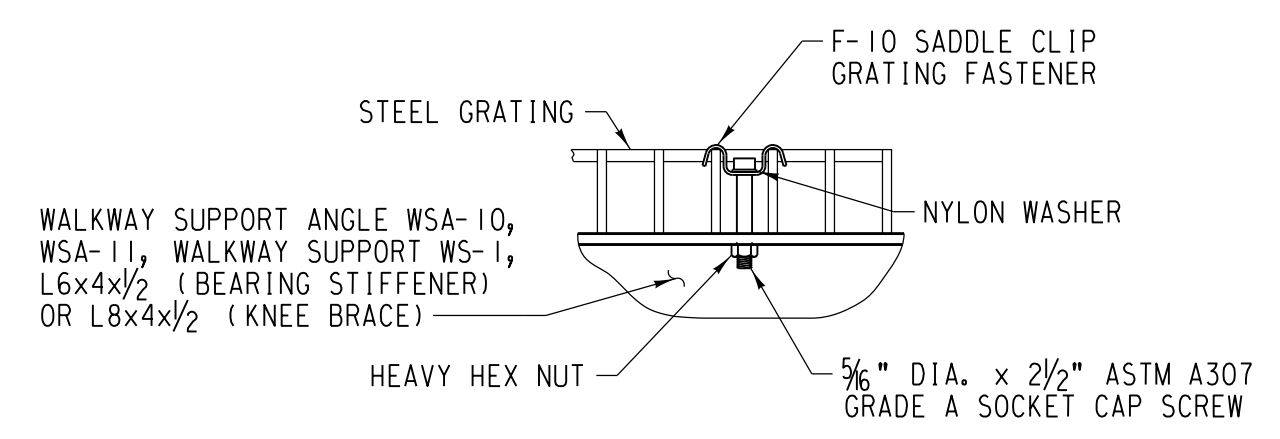
BALLAST PAN AND GRATING PLAN
SCALE: 1/4"=1'-0"

NOTE:
TOP FLANGES OF GIRDERS AND FLANGES OF KNEE BRACES NOT SHOWN FOR CLARITY.

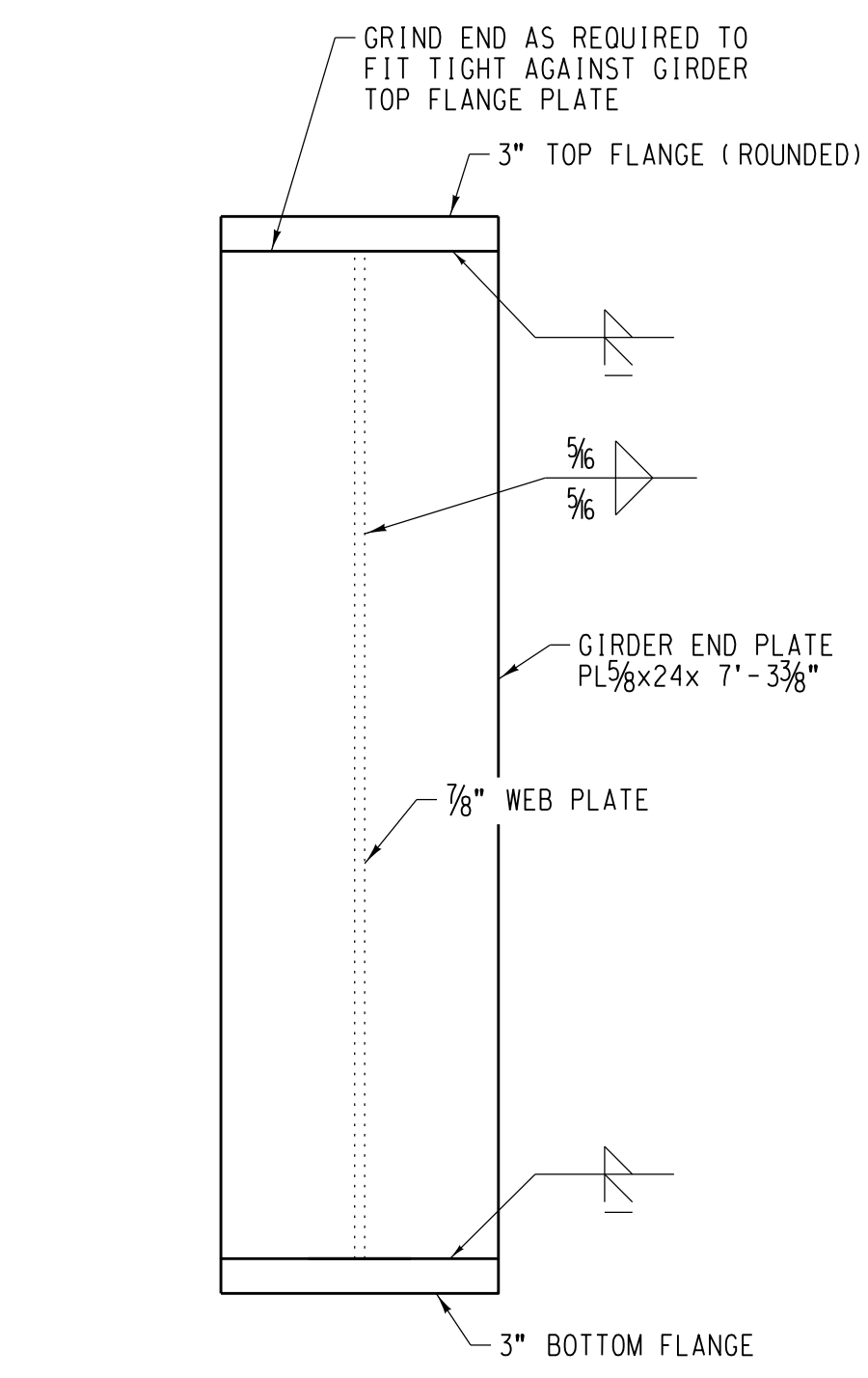


- NOTES:**
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 - THE REQUIREMENTS OF FULLY AUTOMATIC SUBMERGED ARC WELDING MAY BE WAIVED FOR FIELD WELDING THE BALLAST PANS TO THE FLOORBEAMS, KNEE BRACES AND WALKWAY SUPPORTS.
 - PLACE BALLAST PANS AND GRATING PANELS WITH PIECE MARKS AS SHOWN.
 - INSTALL BALLAST PANS STARTING WITH BP-3 AND THEN WORK TOWARDS BP-1 AND BP-4 ENDS.
 - LLH = LONG LEG HORIZONTAL

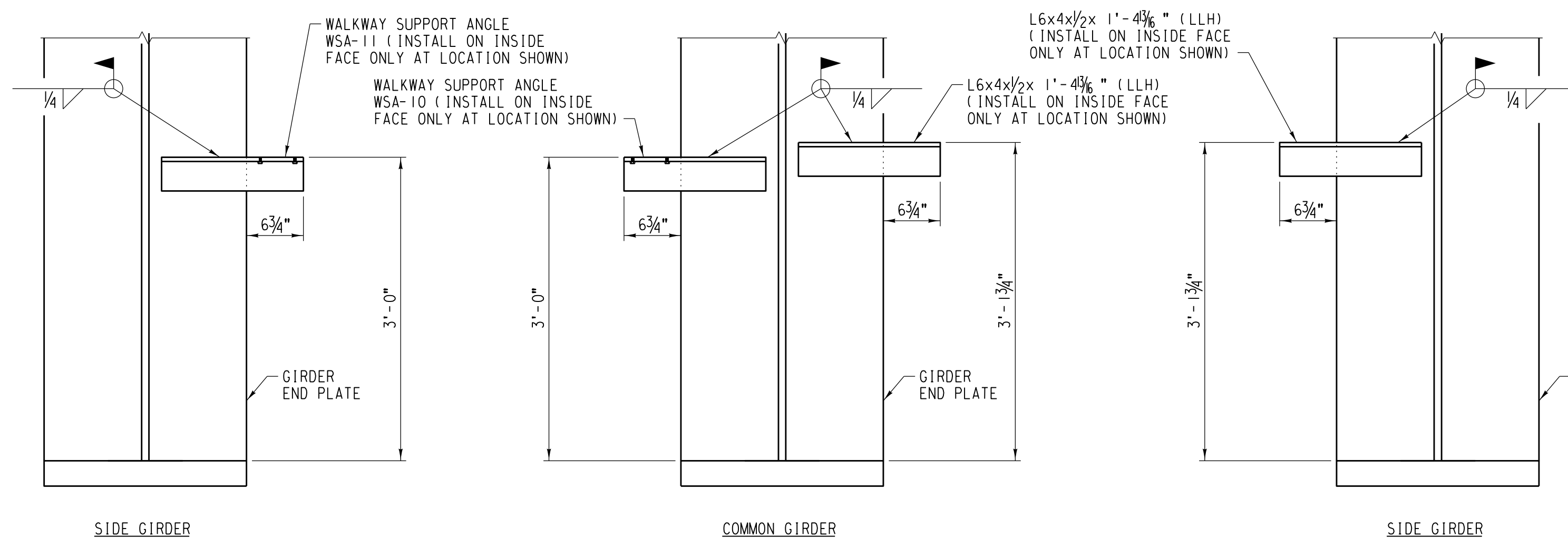
SECTION B
SCALE: 1"=1'-0"



GRATING CONNECTION DETAIL
SCALE: 3"=1'-0"



**GIRDER END PLATE
INSTALLATION DETAIL**
SCALE: 3/8"=1'-0"
END VIEW OF GIRDER AT END PLATE, TYP. FOR SIDE AND COMMON GIRDER



WALKWAY SUPPORT ANGLE INSTALLATION DETAIL
SCALE: 1"=1'-0"
SECTION THRU GIRDER AT END PLATE

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C.E. NUMBER:
	31876	122525

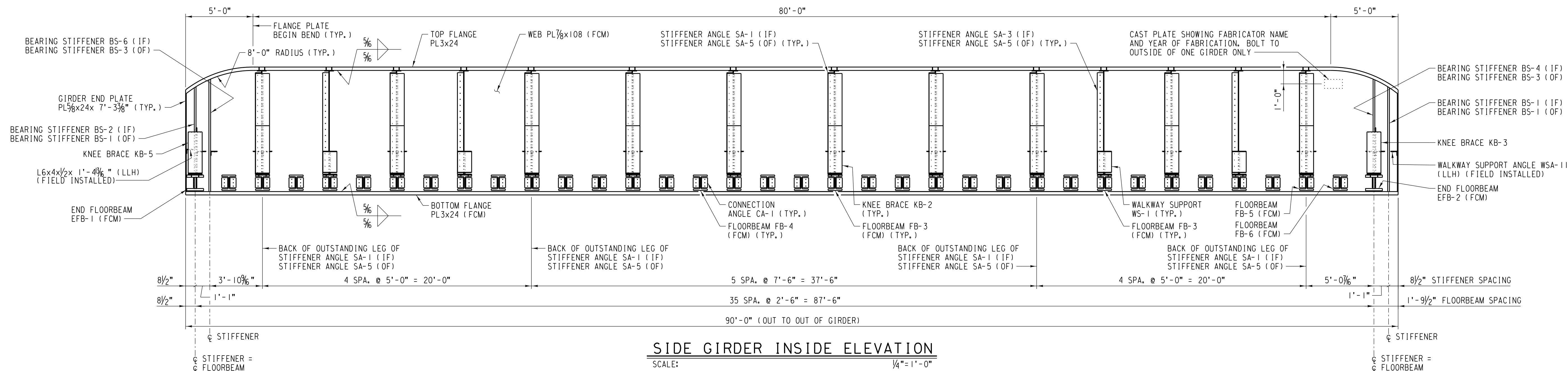
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION

LATITUDE: 41.88670°N LONGITUDE: 87.69144°W

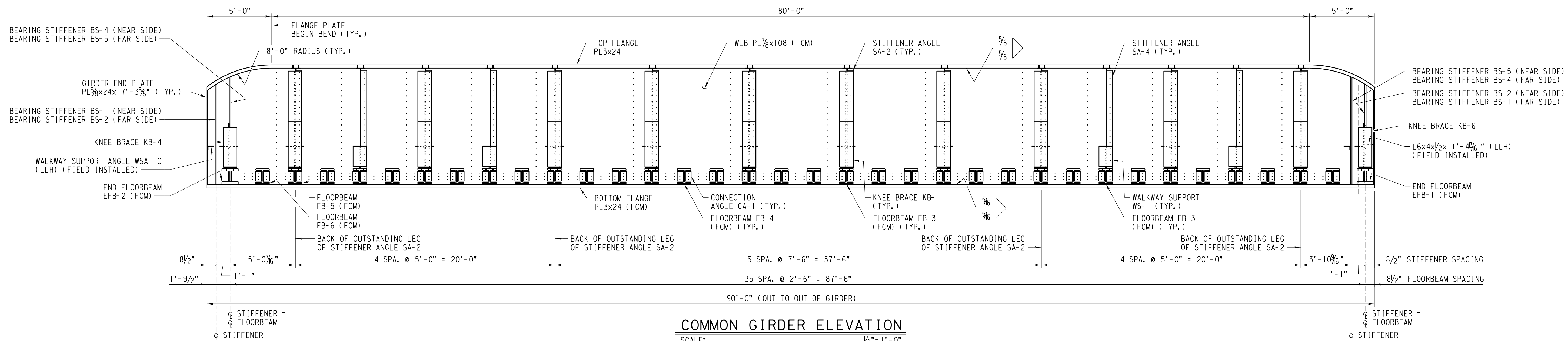


DESIGNED BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design
DRAWN/CHECKED BY: RR /MFB	
UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION
SHEET NO.: E22 of E43	1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)
	SHEET TITLE: TPG BALLAST PAN AND GRATING PLAN

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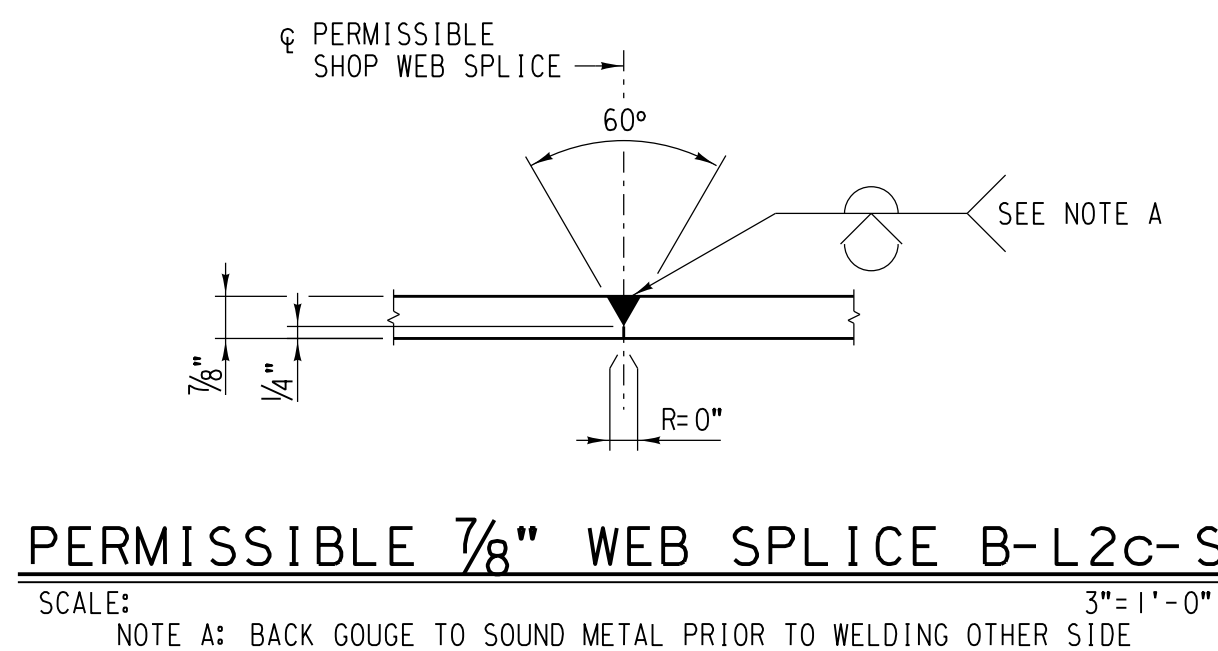
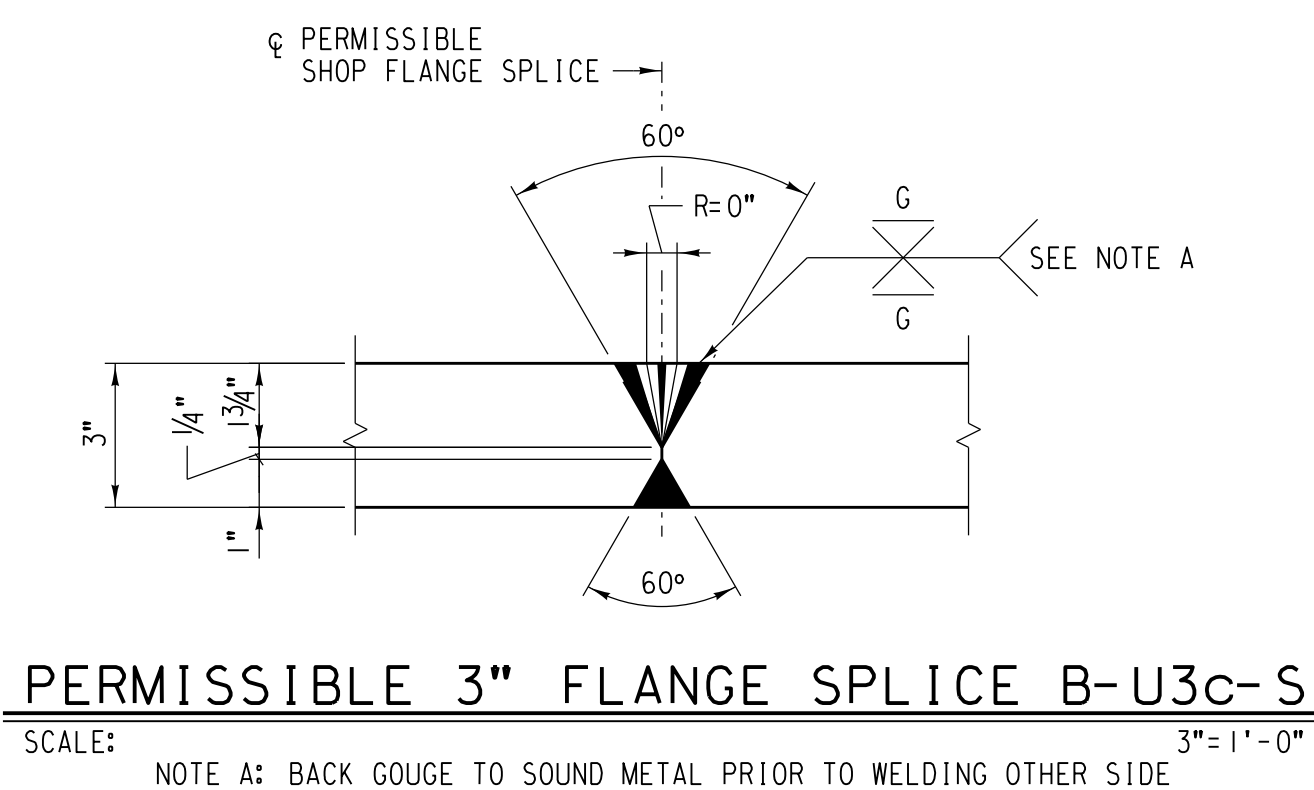


SIDE GIRDER INSIDE ELEVATION
SCALE: 1/4" = 1'-0"



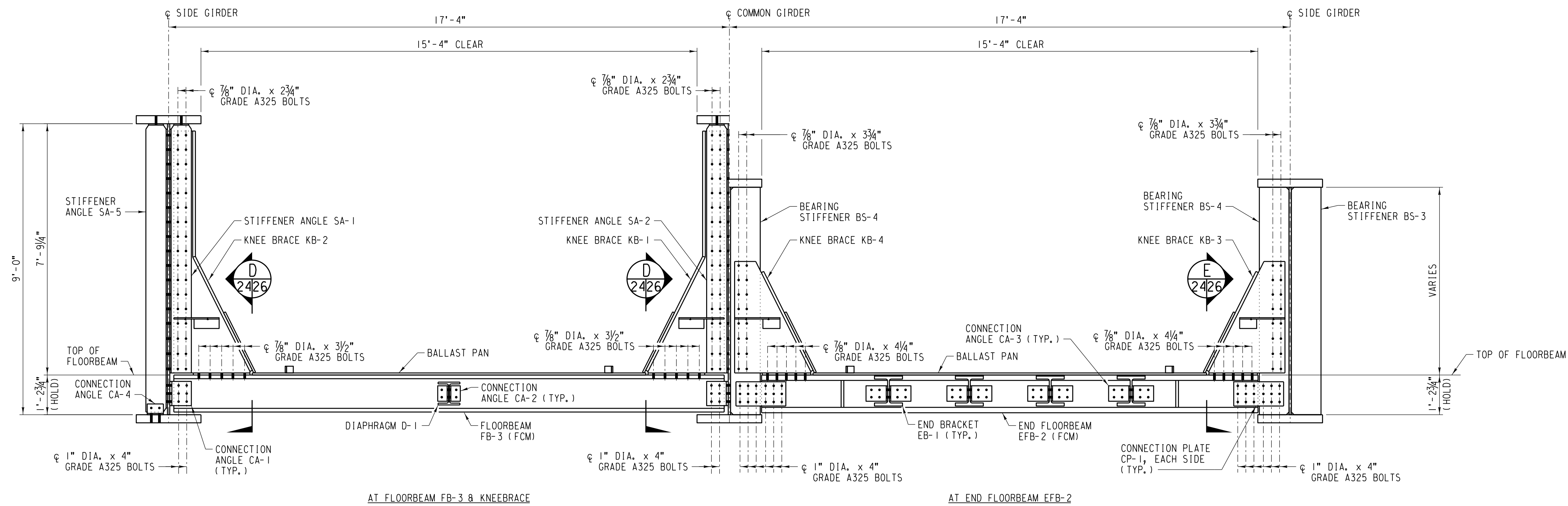
COMMON GIRDER ELEVATION
SCALE: 1/4" = 1'-0"

- NOTES:
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 - FCM = FRACTURE CRITICAL MEMBER
IF = INSIDE FACE
LLH = LONG LEG HORIZONTAL
OF = OUTSIDE FACE
 - NO CAMBER IN PROPOSED GIRDERS.



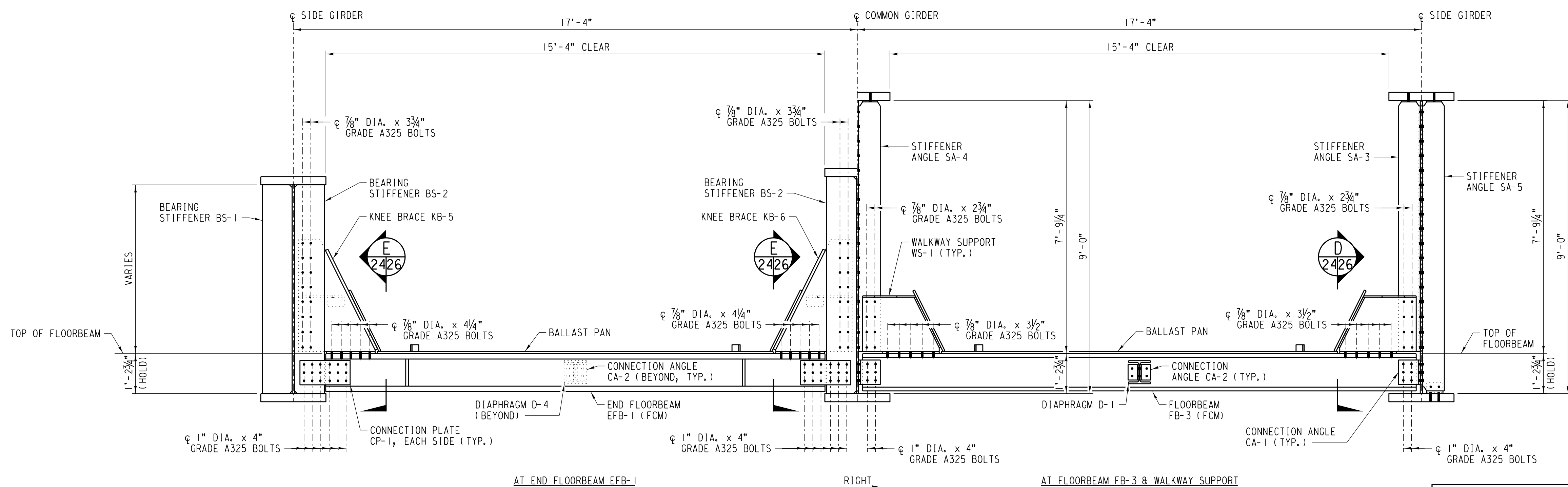
NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
UNION PACIFIC	DSNCHK BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHK BY:	Office of Director Structures Design	
	RR /MFB	LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E23 of E43		SHEET TITLE:	
		TPG GIRDER ELEVATIONS AND DETAILS	



RIGHT
PARTIAL SECTION
SCALE: 1/2" = 1'-0"

NOTE:
STEEL GRATING NOT
SHOWN FOR CLARITY.



RIGHT
PARTIAL SECTION
SCALE: 1/2" = 1'-0"

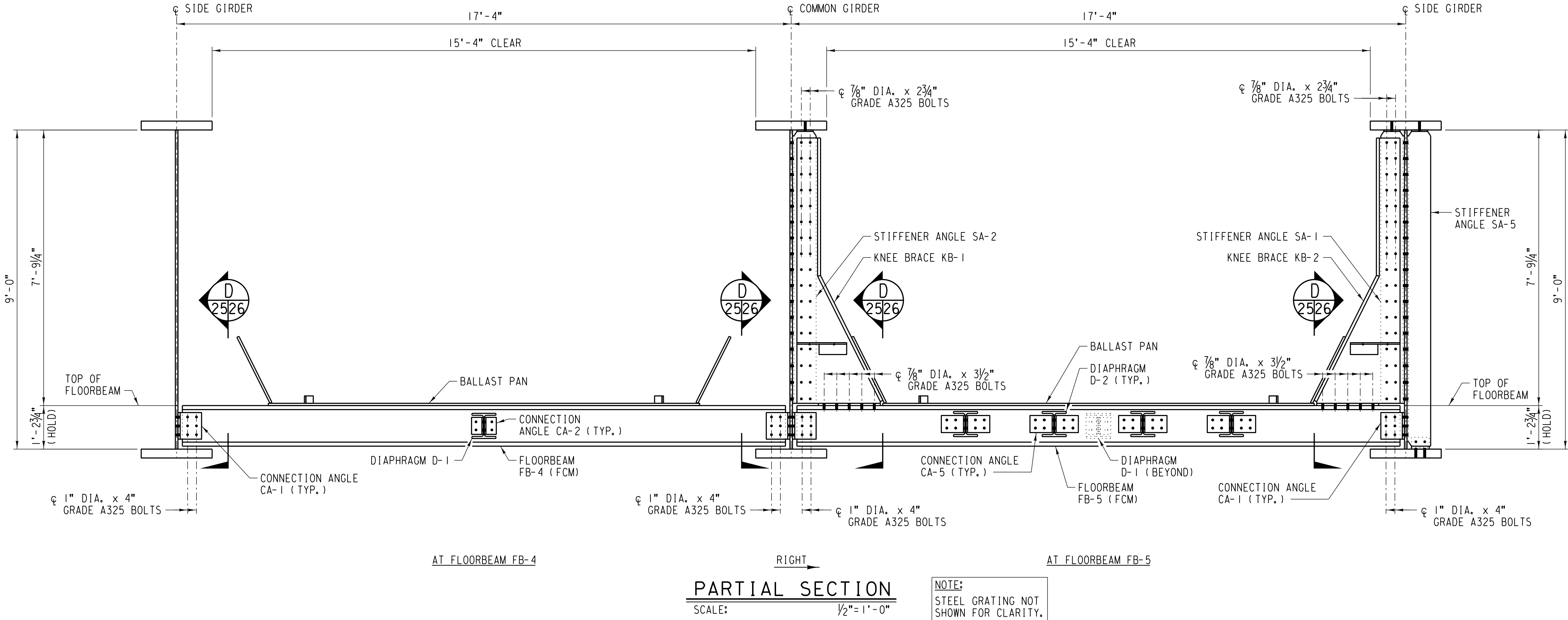
NOTE:
STEEL GRATING NOT
SHOWN FOR CLARITY.

- NOTES:
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 - FCM = FRACTURE CRITICAL MEMBER

NO.	DATE	REVISIONS
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FINAL		05/28/2021
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MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DESIGNED BY:	UNION PACIFIC RAILROAD	
	FN/MFB		
	DRAWN/CHECK BY:	Office of Director Structures Design	
	RR /MFB	LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E24 of E43		SHEET TITLE:	
		TPG TYPICAL SECTIONS (SHEET 1 OF 2)	

FILE NAME: C:\Users\skroml\OneDrive\Documents\TPG\020093.dwg



TPG STRUCTURAL STEEL NOTES

- Materials, fabrication and erection shall be in accordance with Chapter 15: Steel Structures of the AREMA Manual for Railway Engineering.
- Fabrication of structural steel shall be performed by a Fabricator certified under AISC Quality Certification Program for Major Steel Bridges (CBR) and Fracture Critical Endorsement (FCE).
- Fabrication of bearings shall be performed by a Fabricator certified under AISC Quality Certification Program for Certified Metal Component Manufacturer (CPT).
- Material shall conform to the following requirements:

Girder Web, Bottom Flange and Floorbeams	ASTM A709 Grade 50W F2
Girder Top Flanges and Bearing Stiffeners	ASTM A709 Grade 50W T2
Side and Floor Plates	ASTM A709 Grade 50 T2
All Other Structural Steel	ASTM A588
Anchor Rods	ASTM F1554 Grade 105
TPG Bearings	Per notes, Sheet No. 39 thru 42
- Girders shall not be cambered. Floorbeams shall not be cambered, but shall be fabricated with natural camber up.
- Steel designated as fracture critical (FCM) shall comply with the requirements of AREMA Chapter 15, Section 1.14. Testing shall be performed for a minimum service temperature corresponding to Zone 2. Killed fine grain practice shall be used in the manufacture of FCM steel. Supplementary requirements S84 and S93 of ASTM A709 shall be mandatory.
- Structural steel shall be of the type and quality as designated on the drawings. Material supplied shall meet the longitudinal Charpy V-notch requirements for Zone 2 as specified in the AREMA Manual for Railway Engineering.
- All shop and field bolted connections shall use 7/8" or 1" dia. high strength bolts with nut and hardened steel washer. High strength bolts shall conform to ASTM F3125 Grade A325, Type 3. Nuts shall conform to ASTM A563, lubricated. Washers shall conform to ASTM F436 and shall be placed under the element to be turned. Diameter of bolt holes shall be 1/16" larger than the nominal bolt diameter, unless noted otherwise.
- High strength steel bolts shall be installed in accordance with the "Turn of Nut Method". The procedure for installation is as specified by the Research Council on Structural Connections. Alternative bolt installation methods are subject to approval by the UPRR Office of Director Structures Design with verification performed in accordance with AREMA Chapter 15, Articles 3.2.2.h and 3.2.2.k.
- Bolts shall be installed so that the bolt heads are on the outside (exposed) surface of the member unless shown otherwise on the drawings. Threads shall be excluded from the shear plane in all connections.
- Any machine bolts required for shipment shall be ASTM A307.
- All welding shall be in accordance with the Bridge Welding Code, AWS D1.5. Welding of fracture critical members shall also conform to the applicable provisions of AREMA Manual for Railway Engineering, Chapter 15: Steel Structures. Welding to be allowed only as shown on the drawings and approved shop drawings. Weld metal material properties shall match structural steel.
- Welded joints are to be AWS prequalified. Alternate joint details are subject to approval by the UPRR Office of Director Structures Design. All welding shall be done to minimize distortion. The welding sequence and procedures to be used shall be submitted for approval to the UPRR Office of Director Structures Design.

TPG STRUCTURAL STEEL NOTES - CONT.

14. Fully automatic submerged arc welding shall be required for this project unless otherwise noted. Manual shielded arc welding or semi-automatic submerged arc welding shall be allowed only if fully automatic submerged arc welding is not practical. Alternate welding methods are subject to approval by the UPRR Office of Director Structures Design.
15. When welding ASTM A709 Grade 50W or ASTM A588 steel, weld metal shall be equivalent to ASTM A709, Grade 50W or ASTM A588 steel in corrosion resistance and weathered appearance.
16. The Fabricator shall submit copies of welders' certificates for all welding processes. Welders shall possess valid qualifications.
17. Permissible Shop Splice - Steel fabricator may provide welded, shop flange and web splices where required in accordance with the following:
 - Splices shall be located 1'-0" minimum clear from all bolted and/or welded attachments to the girders.
 - Splices shall not be located within the middle third of the girder span, nor any location where weld metal may be subject to drilling.
 - One shop splice shall be permitted in the bottom flange plate at a minimum distance of 3'-0" from nearest top flange splice.
 - Web plate splices shall be located a minimum of 1'-0" from back face of nearest intermediate web stiffener angle and a minimum of 2'-0" from centerline of nearest flange splice.
 - All shop splice welds for flanges shall be ground smooth in the direction of stress.
 - Plates shall be provided in maximum lengths as practical to minimize the number of splices required.
 - Final locations of shop splices in flanges to be approved by the UPRR Office of Director Structures Design.
 - Electroslag and electrogas welding process shall not be used.
18. Nondestructive testing of welds shall be performed in accordance with the AREMA Manual for Railway Engineering Chapter 15: Steel Structures, the Bridge Welding Code, AWS D1.5, Sections 6.7 and 12.16 and as follows:
 - 100% RT inspection of full penetration welds in girder webs and bottom flanges.
 - 100% UT inspection of full penetration groove welds in top flanges.
 - 100% MP inspection of all flange to web welds.
 - 100% MP inspection of fillet welds on bearing stiffeners.
 - 25% MP inspection of all other welds. If any defects are found, then 100% MP inspection shall be required.Test results shall be furnished to the UPRR Office of AVP Engineering Design.
19. All bearing stiffeners shall be vertical under final dead loads. All intermediate stiffeners shall be placed normal to the girder bottom flange. All stiffeners and girder end plates shall be placed perpendicular to the web.
20. Flame cutting of girder webs and flanges shall be performed in accordance with the AREMA Manual for Railway Engineering Chapter 15, Article 3.1.6: Thermal Cutting, Copes and Access Holes. Preheating is required. Preheating shall be in accordance with the following:

Thickness	Minimum Preheat Temperature
Up to 1"	50° F
Over 1" to 2"	100° F
Over 2"	200° F

The piece shall be preheated in such a manner that it is heated through 1/3 its thickness and that both surfaces are above the minimum temperature for a distance equal to the thickness of the part being cut, but not less than 3", both laterally and in advance of the cut. The Fabricator shall furnish a calibrated electrical contact pyrometer to verify compliance with the preheat requirements during fabrication.

TPG STRUCTURAL STEEL NOTES - CONT.

21. For welds designated to be ground flush, grinding shall be in the direction of applied stress.
22. All joints and edge preparation, removal of unacceptable weld or base metal, and backgouging shall be completed by machining. Rough removals may be completed by non-mechanical means.
23. The Fabricator shall submit detailed shop drawings prior to beginning fabrication. Fabrication shall not begin until shop drawings are approved.
24. All Fabricator questions and correspondence shall be addressed to Adam Studts:

Union Pacific Railroad
1400 Douglas St., STOP 0910
Omaha, NE 68179
(402) 544-3541
adstudts@up.com
25. The Fabricator shall shop assemble the entire span prior to shipping. During assembly and reaming, all bolts shall be placed in holes as work progresses to assure proper fit.
26. Shop assembled span shall be made available for inspection by the Railroad at the Fabricator's plant before the steel is disassembled and shipped to the erection site at the Railroad's discretion. Pieces shall be match-marked as required.
27. Reaming of holes during field erection is not allowed, unless approved by the Railroad.
28. All structural steel shall be blast cleaned prior to shipment as follows, unless noted otherwise. All ASTM A709 steel, other surfaces visible from sides and all faying surfaces regardless of location: Minimum SSPC-SP6, Commercial Blast Cleaning. All remaining steel surfaces: SSPC-SP1, Solvent Cleaning. All steel members shall be clearly marked after blast cleaning has been completed.
29. All steel components shall be inspected before shipment. Photographs of Fabricator's progress shall be submitted to the UPRR Office of the Director Structures Design.
30. Bearing pads shall be shipped flat.
31. All material certifications and quality control test results shall be submitted to Union Pacific Railroad at project completion.
32. Walkway grating shall be type WB as manufactured by IKG Borden with 1/4"x3/8" serrated bearing bars (galvanized). Any cuts made to grating after galvanizing process shall be treated with ZRC Cold-Galvanizing compound.

TPG DESIGN NOTES

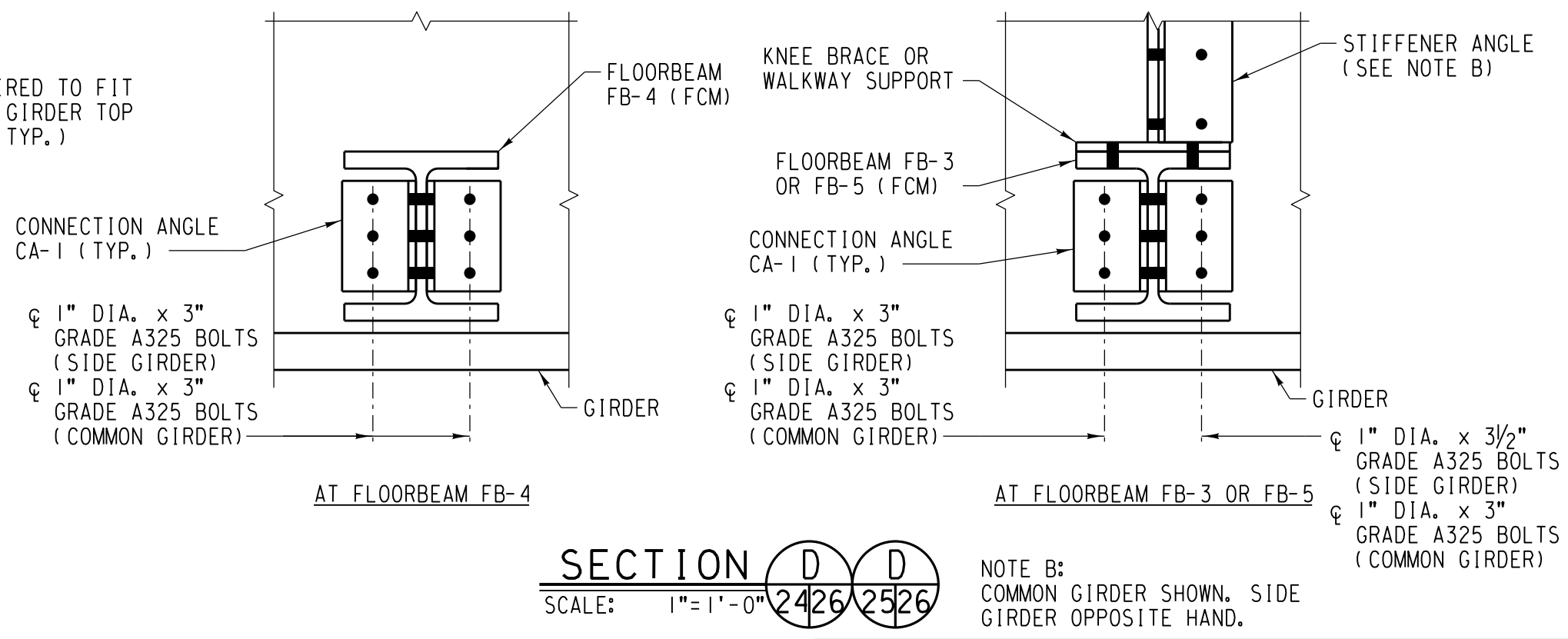
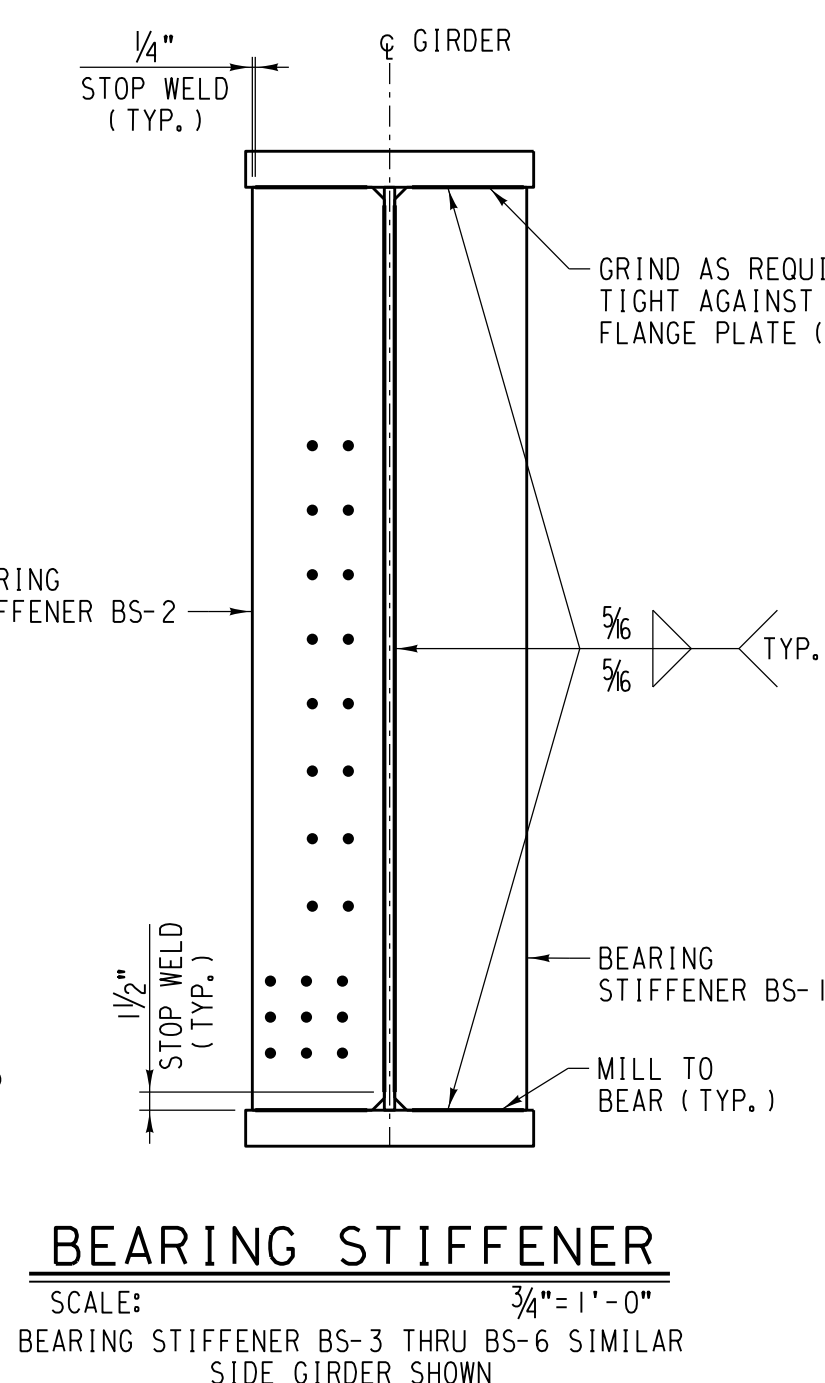
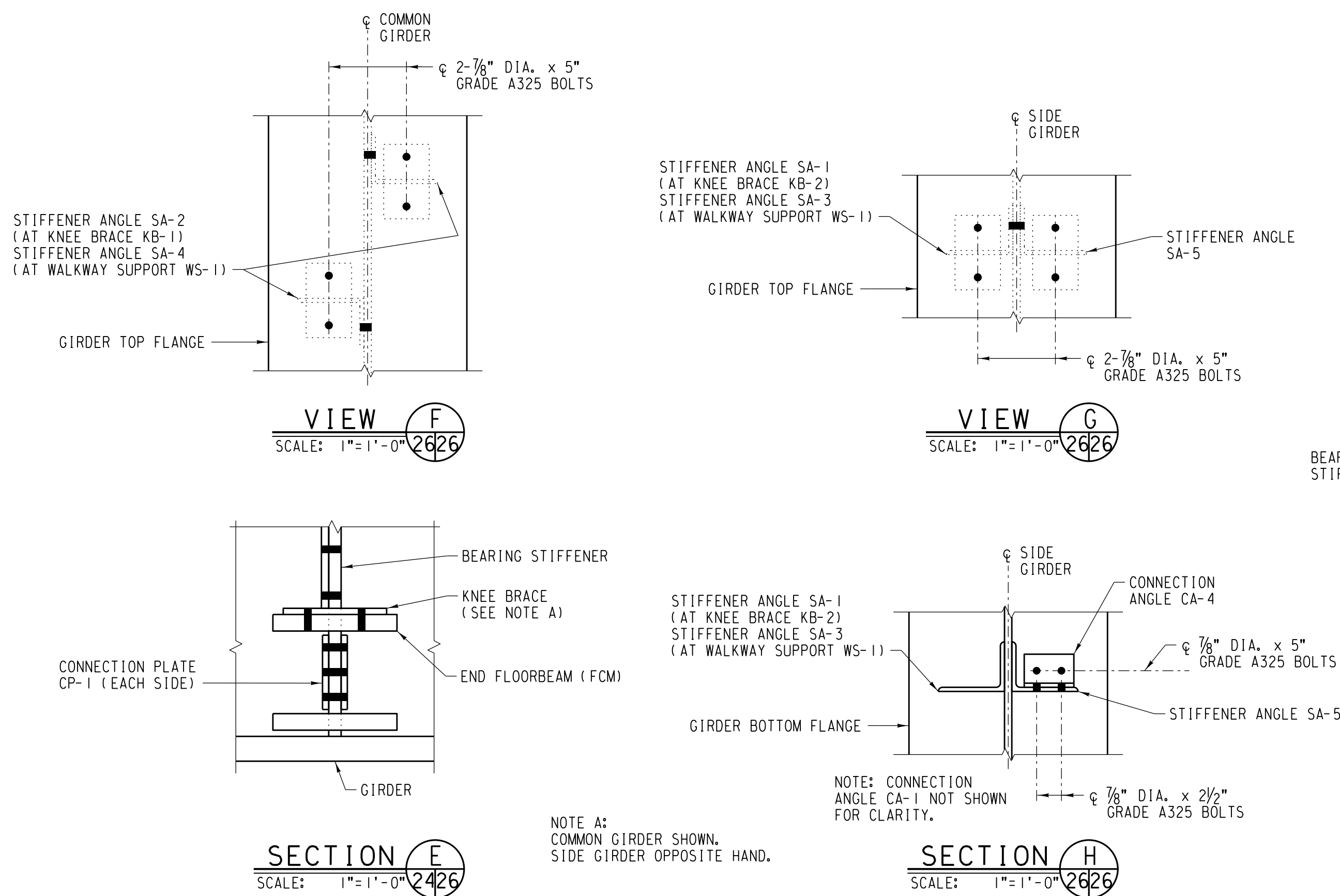
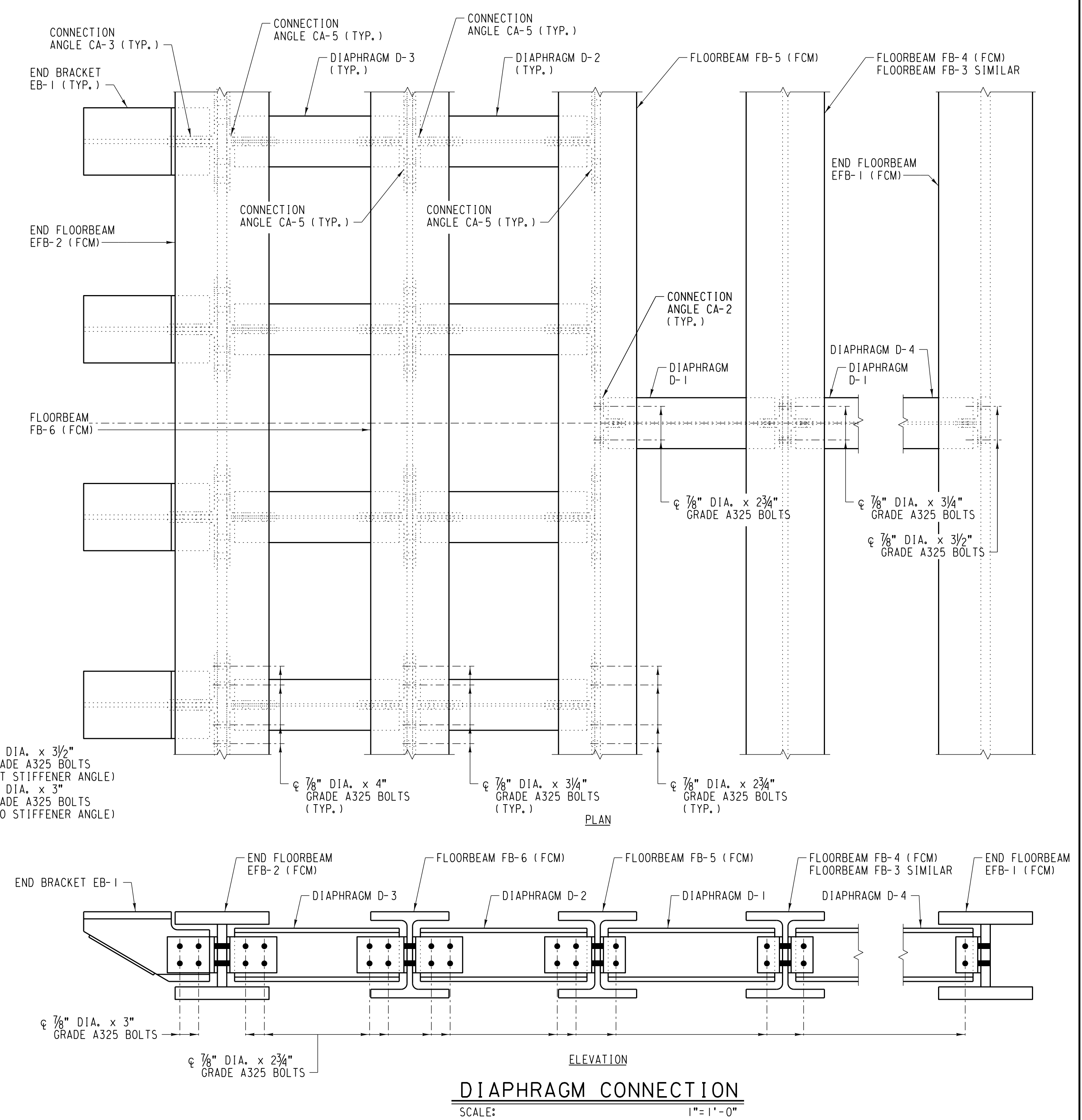
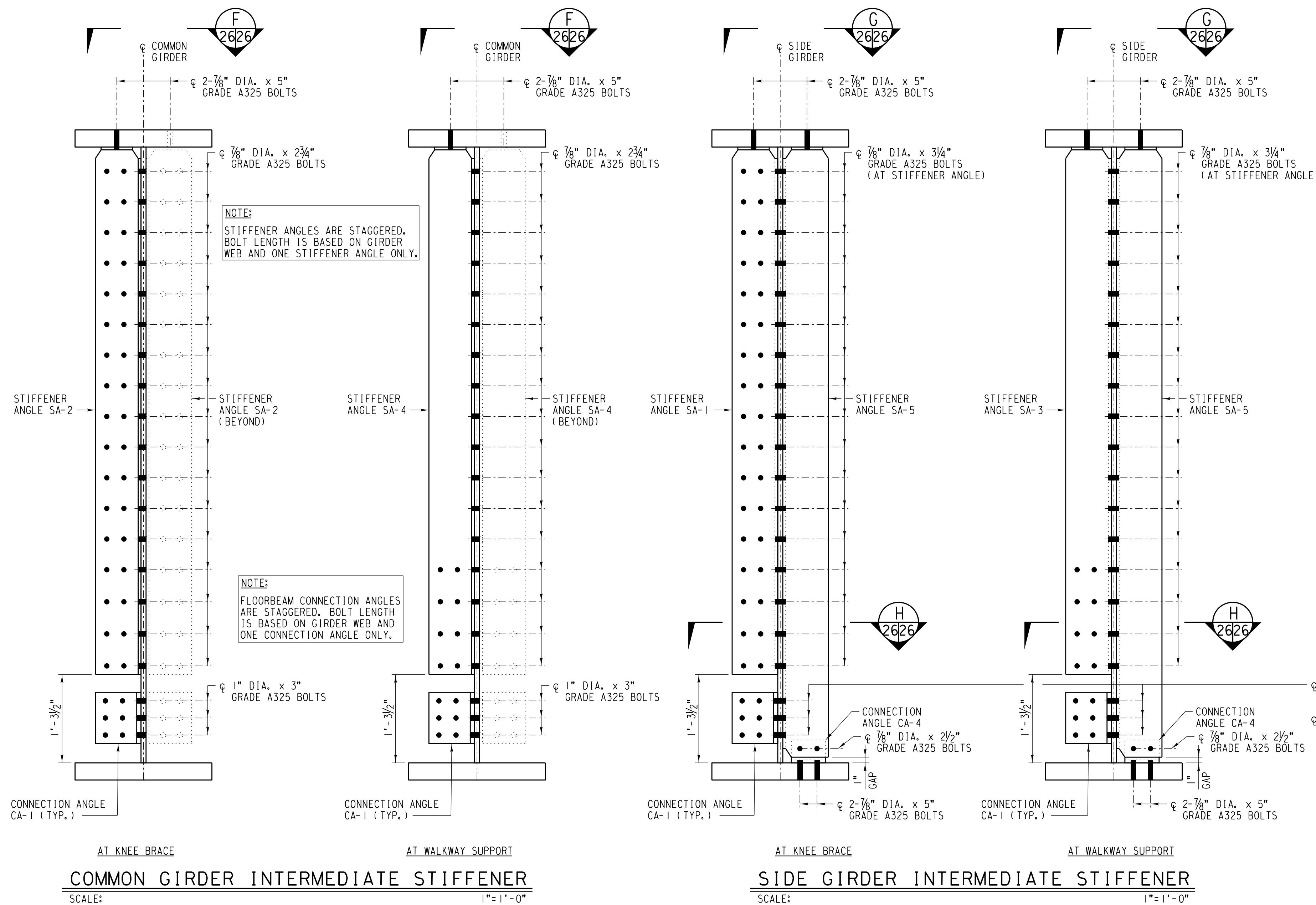
- The TPG span has been designed in accordance with the AREMA Manual for Railway Engineering, Chapter 15: Steel Structures.
- The TPG span has been designed for Cooper E80 Live Load plus impact with a 30" maximum total depth of ballast.
- The floorbeams have been designed for Alternate Live Load plus impact with a 30" maximum total depth of ballast.

NOTE:
FCM = FRACTURE CRITICAL MEMBER

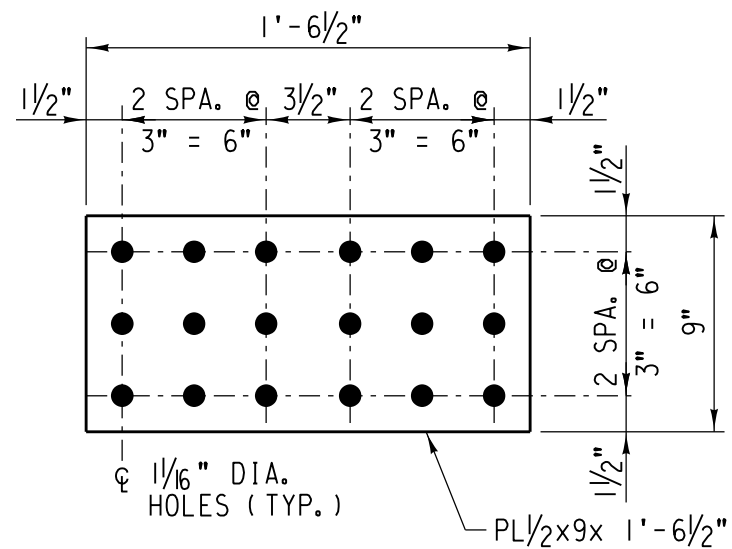
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COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
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MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION			LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSN/CHK BY:	FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design	
	DRAWN/CHK BY:	RR /MFB		
	UPRR ENGINEER:	DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION	
	SHT NO:	E25 of E43	1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
			SHEET TITLE: TPG TYPICAL SECTIONS AND NOTES (SHEET 2 OF 2)	

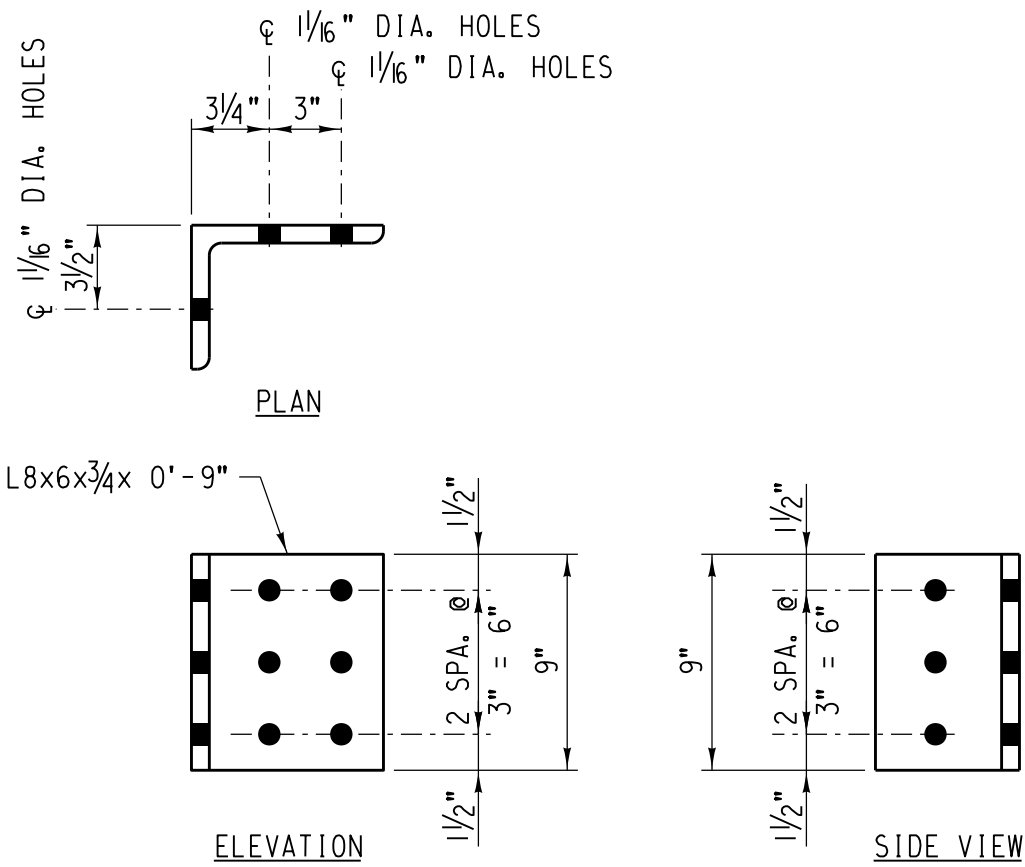
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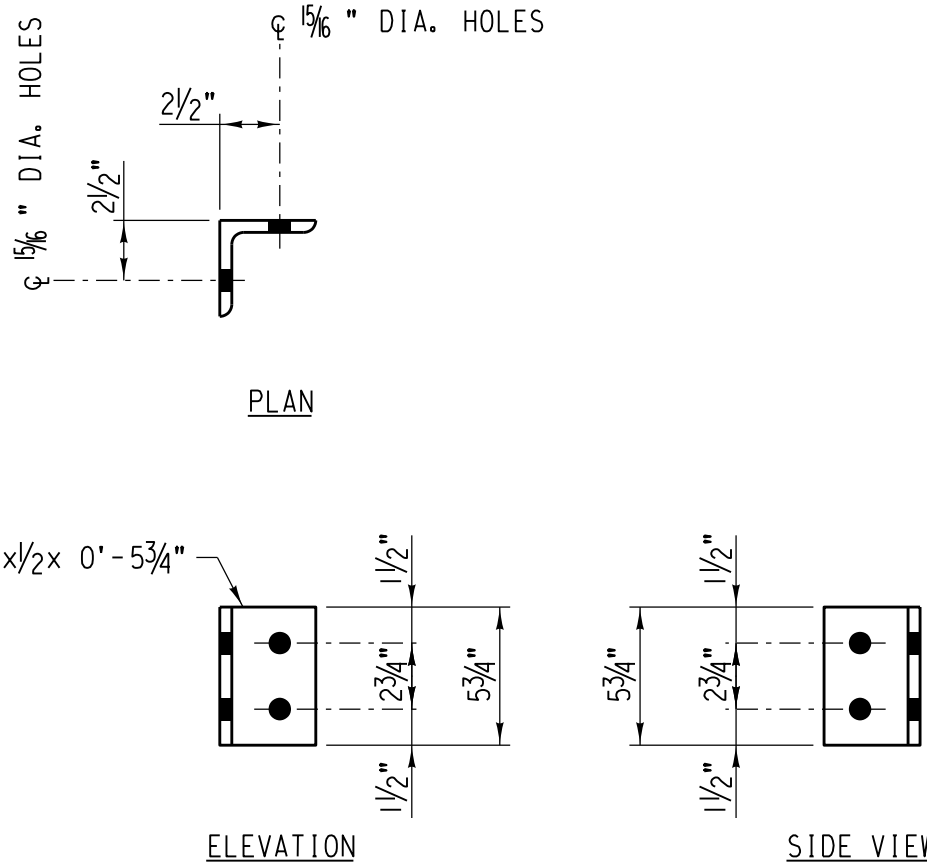
NOTES: 1. FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25. 2. FCM = FRACTURE CRITICAL MEMBER		
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APPROVED FOR UNION PACIFIC RAILROAD BY: MATTHEW BECKER CONSULTANT ENGINEER		
PROJECT ID:		05/28/2021 DATE
WORK ORDER:		31876
C.E. NUMBER:		122525
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		
LATITUDE: 41.88670°N LONGITUDE: 87.69144°W		
UNION PACIFIC RAILROAD Office of Director Structures Design		
LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPG x 70' (2 TRACKS)		
SHEET TITLE: TPG ASSEMBLY DETAILS		
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DRAWN/CHK BY: RR /MFB		
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SHT NO.: E26 of E43		



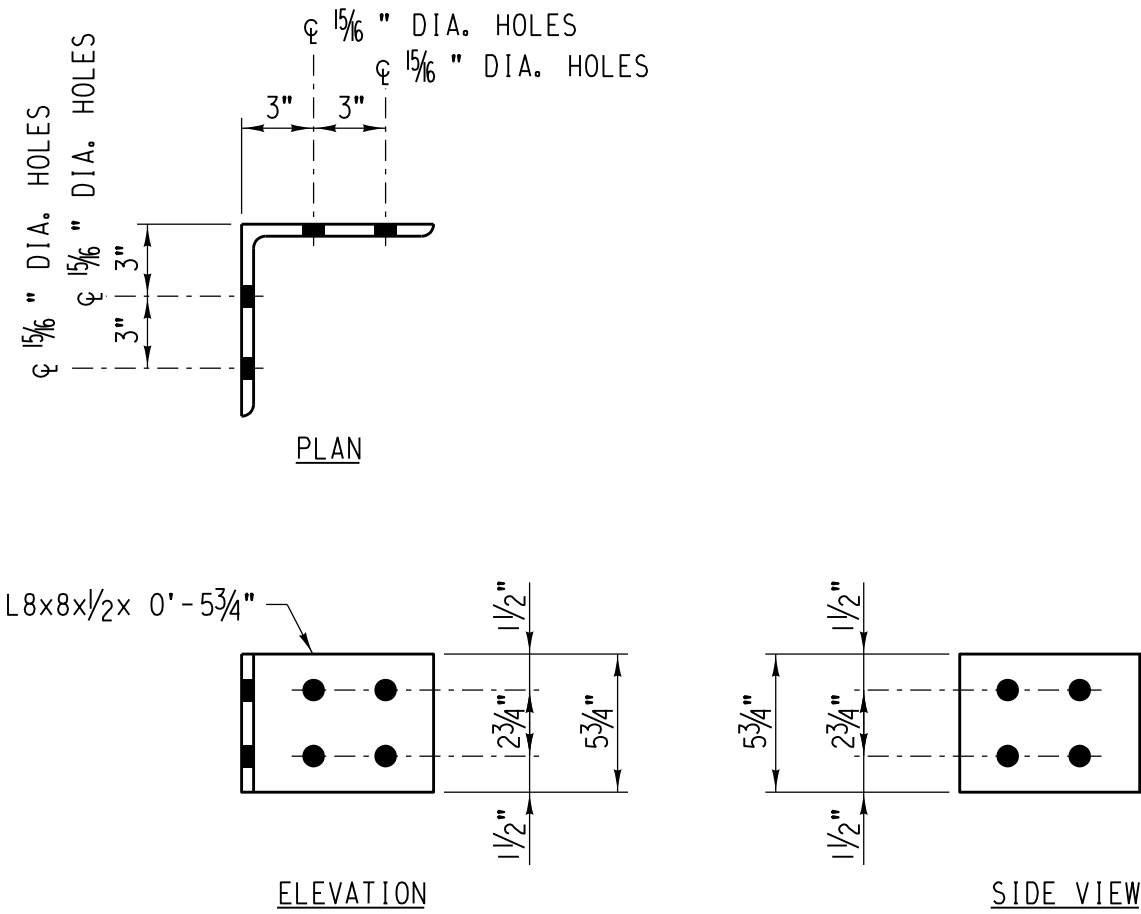
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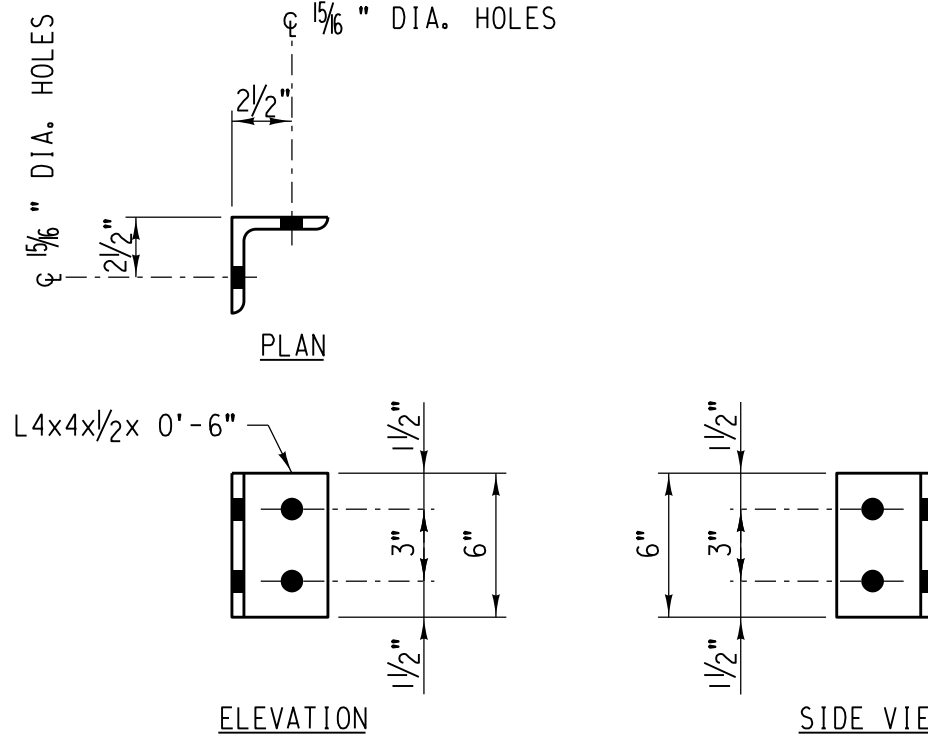
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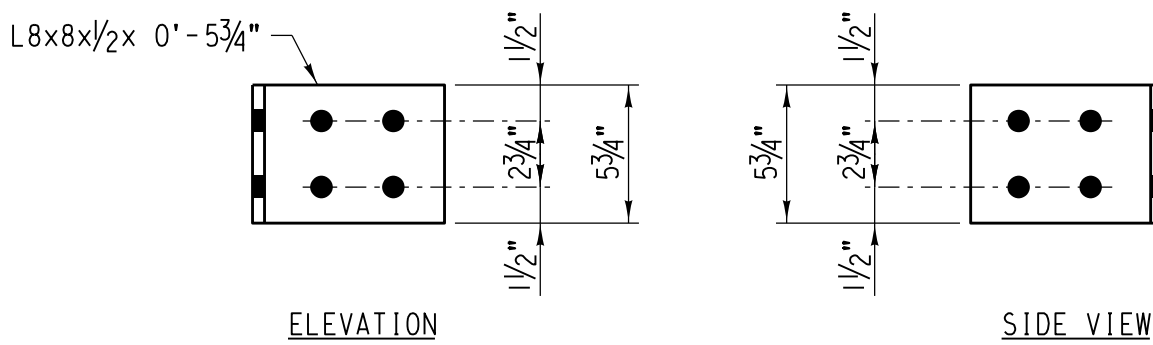
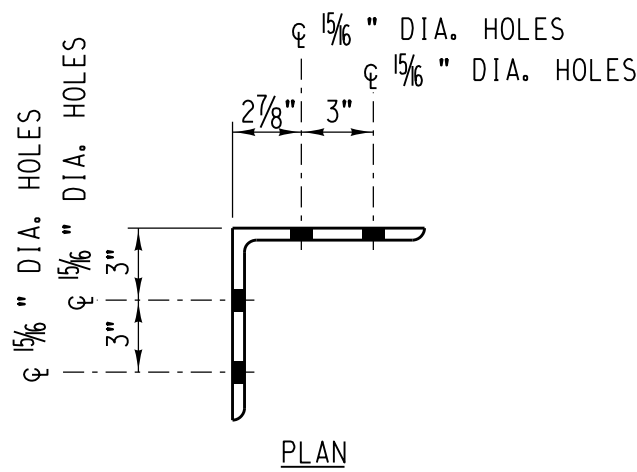
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CONNECTION ANGLE CA-5
SCALE: EST. WT. = 12.6 LB. EA. 1/2"=1'-0"



CONNECTION ANGLE CA-4
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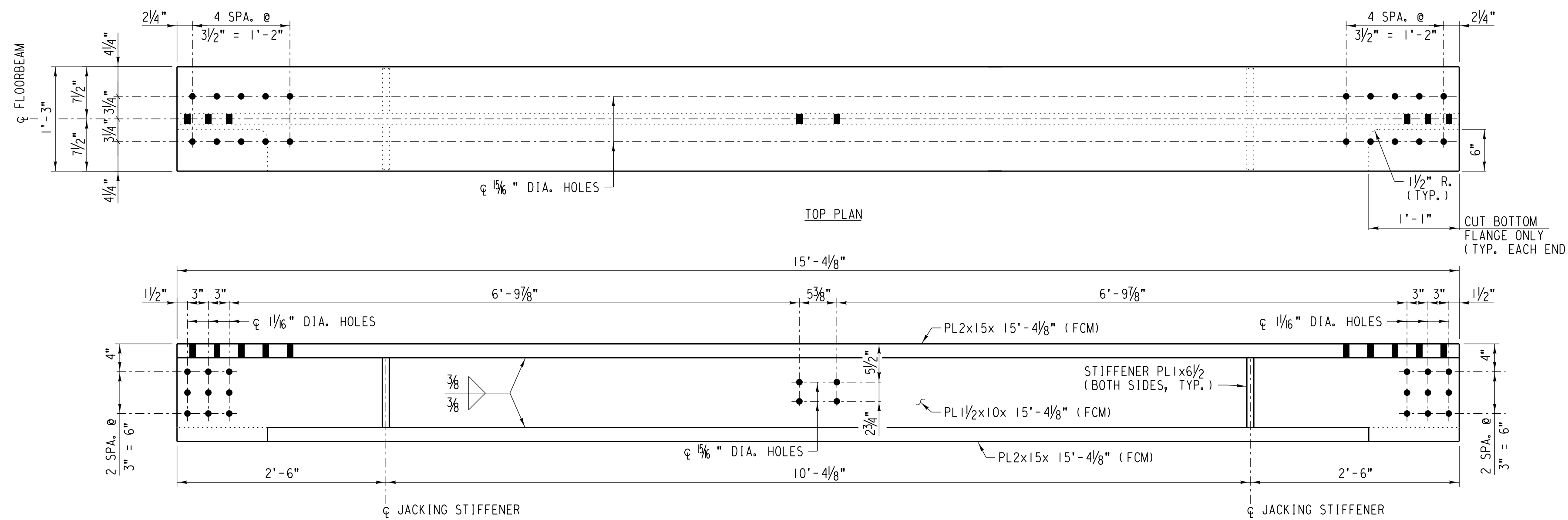


CONNECTION ANGLE CA-3
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- NOTES:
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 - FCM = FRACTURE CRITICAL MEMBER
LLH = LONG LEG HORIZONTAL

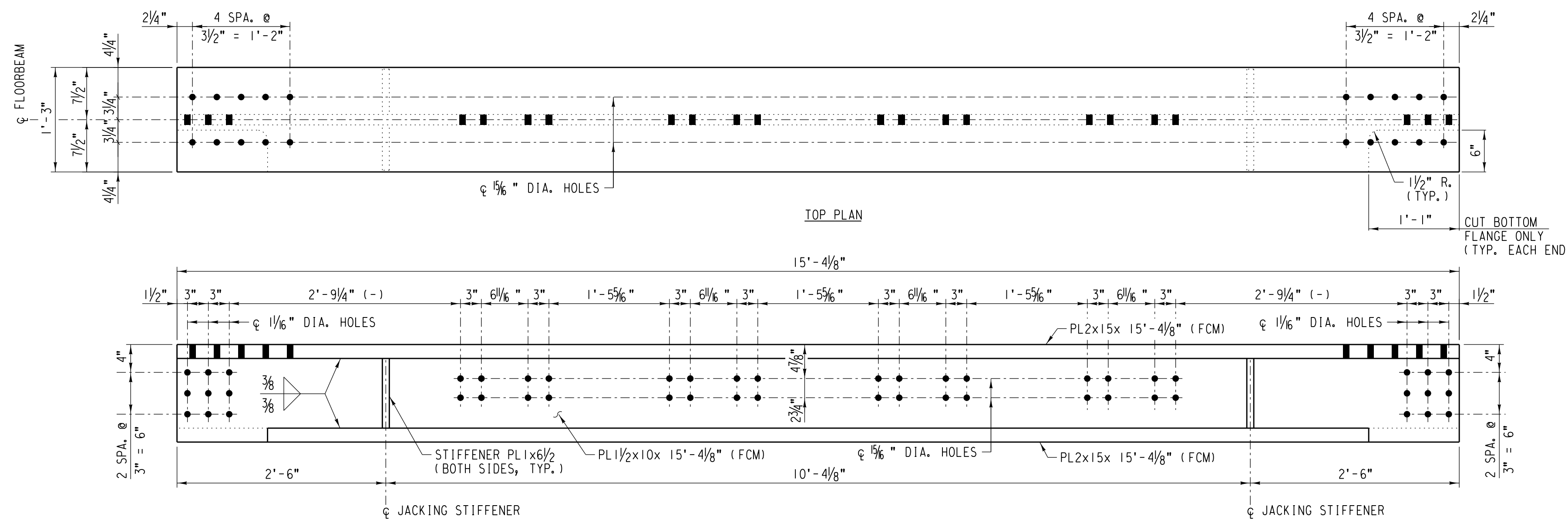
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STATUS		DATE	
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MATTHEW BECKER		05/28/2021	
CONSULTANT ENGINEER		DATE	
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	31876	122525	

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSNCHK BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHK BY:	Office of Director Structures Design	
	RR /MFB	LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E29 of E43		SHEET TITLE:	
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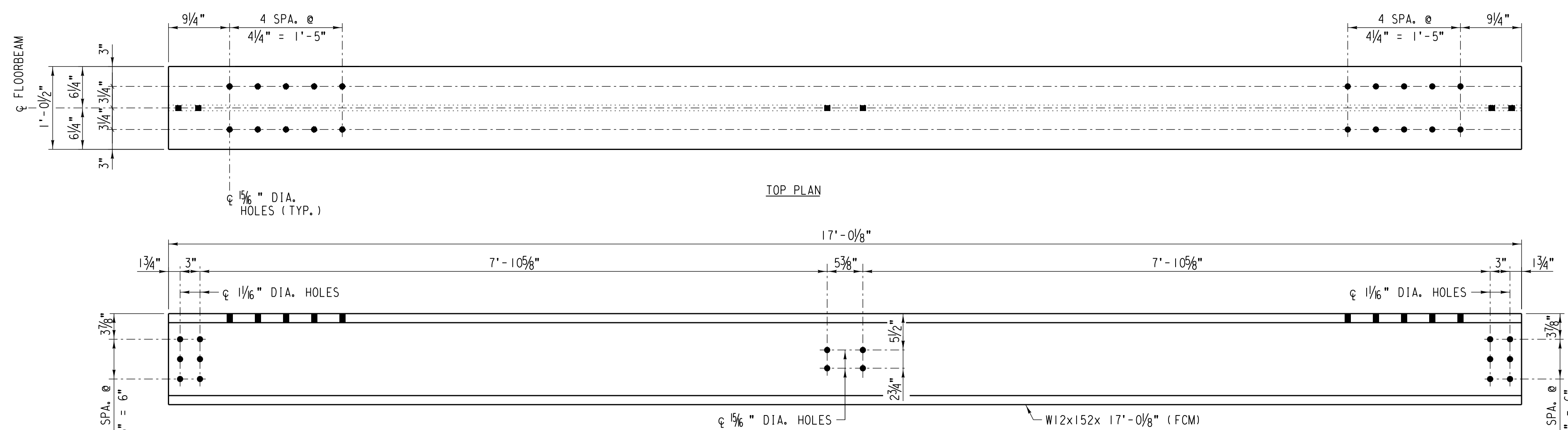
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SCALE: 1"=1'-0" EST. WT. = 3,990 LB. EA.



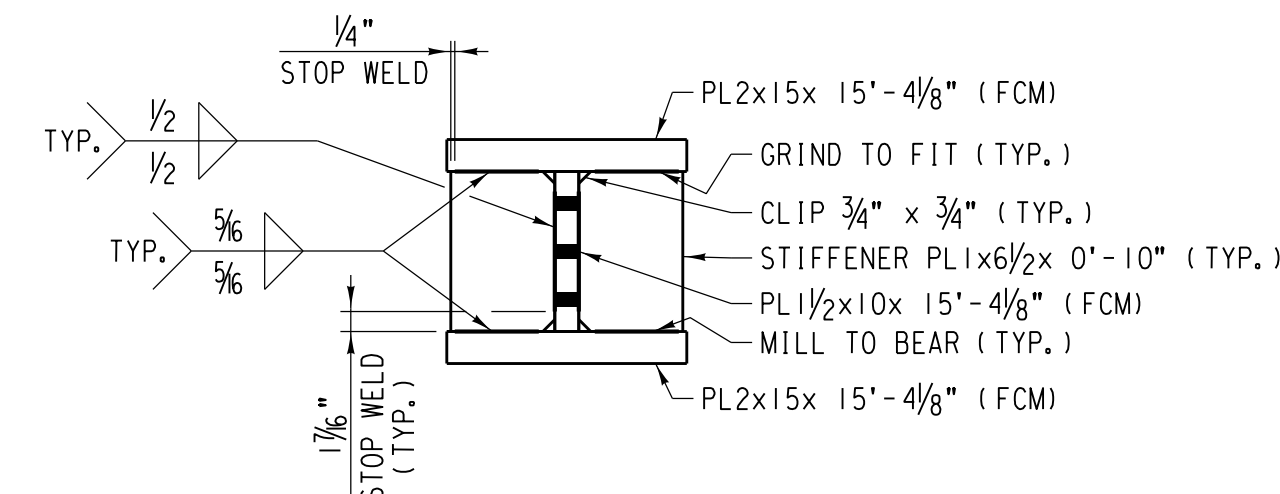
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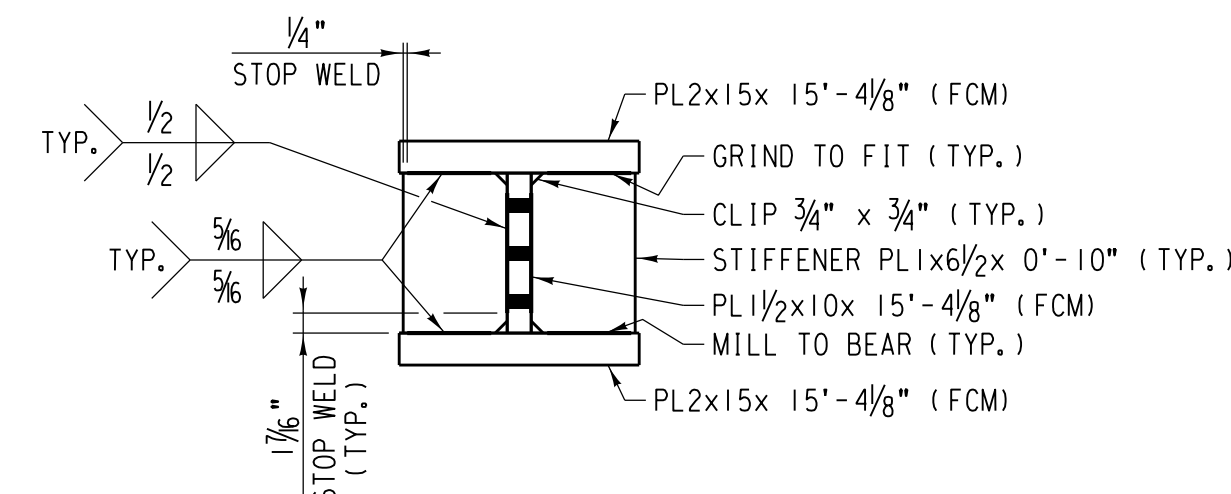


FLOORBEAM FB-3

SCALE: 1"=1'-0" EST. WT. = 2,586 LB. EA.



SECTION



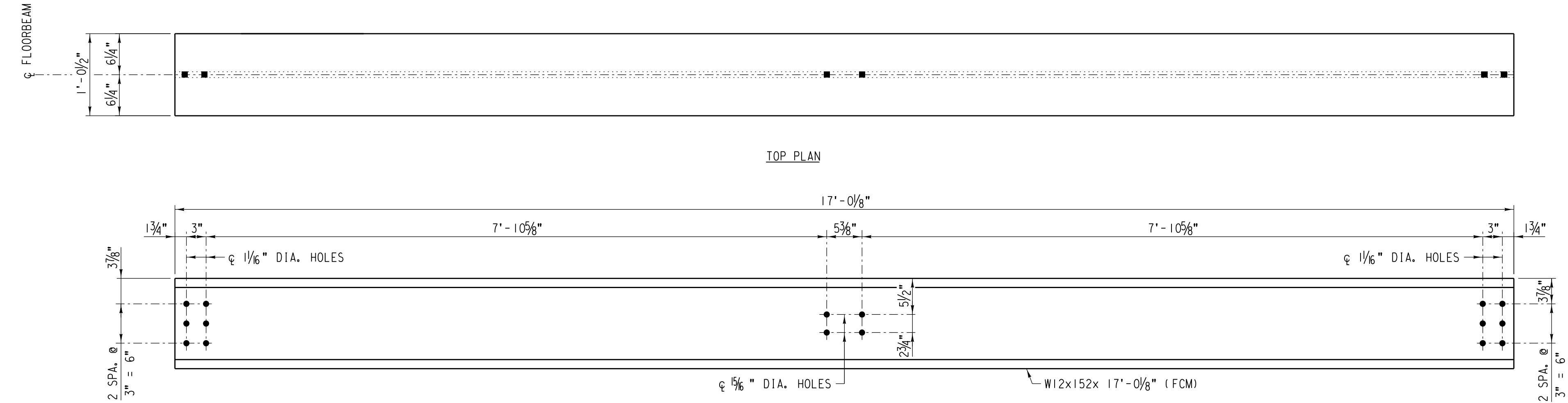
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- NOTES:
1. FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 2. FCM = FRACTURE CRITICAL MEMBER

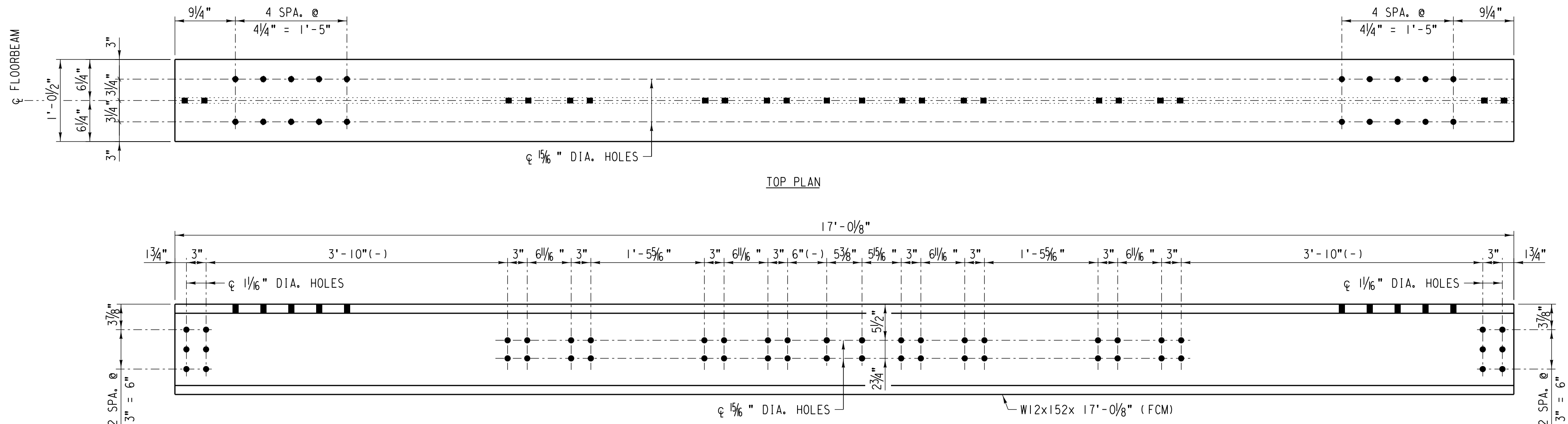
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MATTHEW BECKER		05/28/2021
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	31876	122525

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	DESIGNED BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHECKED BY:	Office of Director Structures Design	
RR /MFB		LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		REPLACING 1 SPAN TPG x 90'	
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E31 of E43		TPG FLOORBEAM DETAILS (SHEET 1 OF 2)	

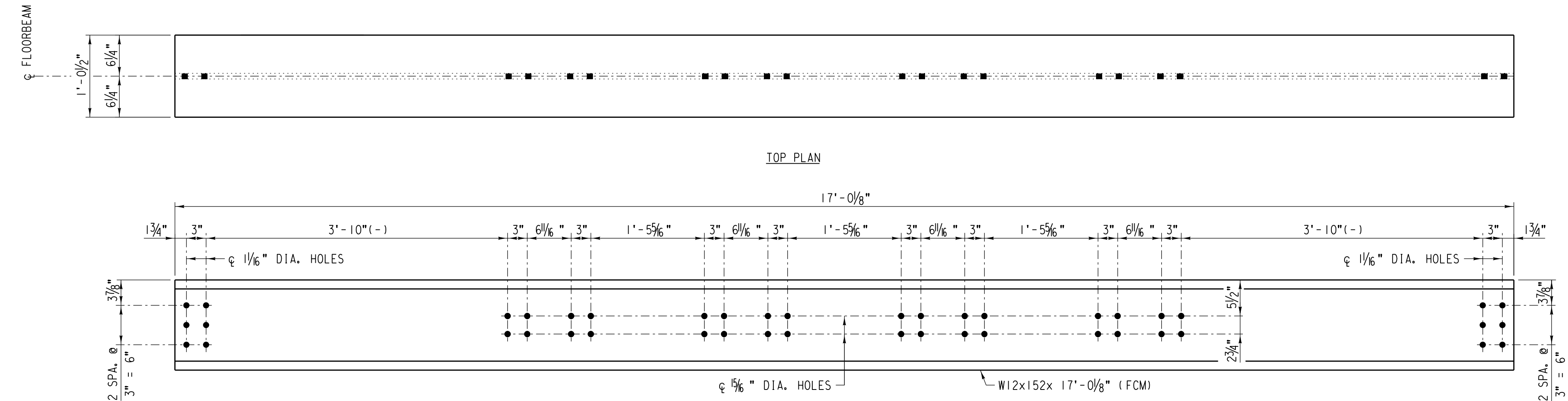
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ELEVATION
FLOORBEAM FB-4
SCALE: 1"=1'-0"
EST. WT. = 2,586 LB. EA.



ELEVATION
FLOORBEAM FB-5
SCALE: 1"=1'-0"
EST. WT. = 2,586 LB. EA.

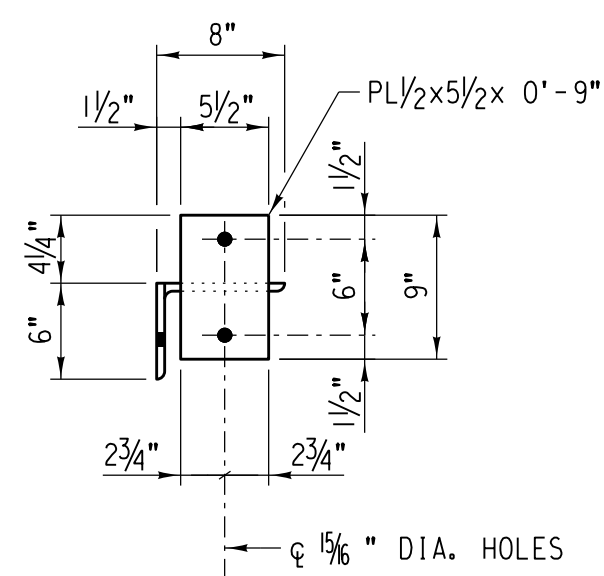


ELEVATION
FLOORBEAM FB-6
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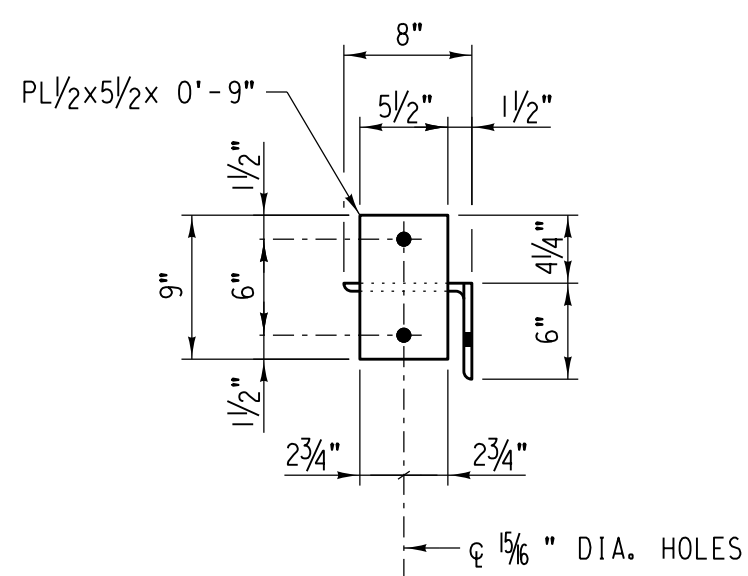
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2. FCM = FRACTURE CRITICAL MEMBER

NO.	DATE	REVISIONS
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MATTHEW BECKER		05/28/2021
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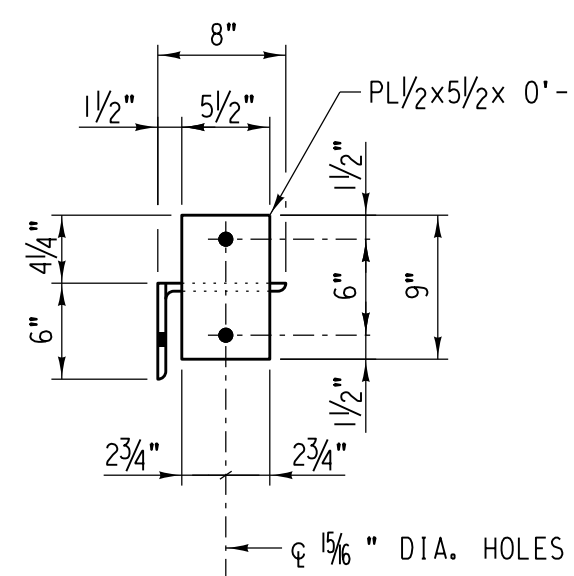
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UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E32 of E43		SHEET TITLE:	
		TPG FLOORBEAM DETAILS (SHEET 2 OF 2)	



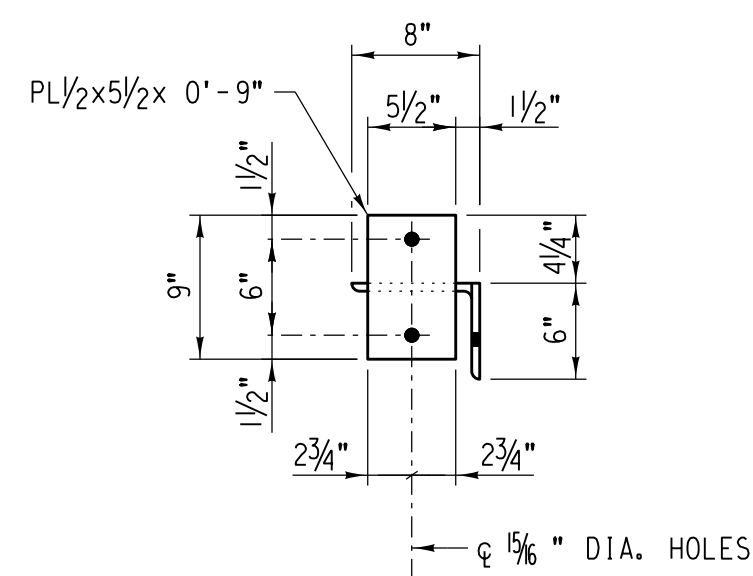
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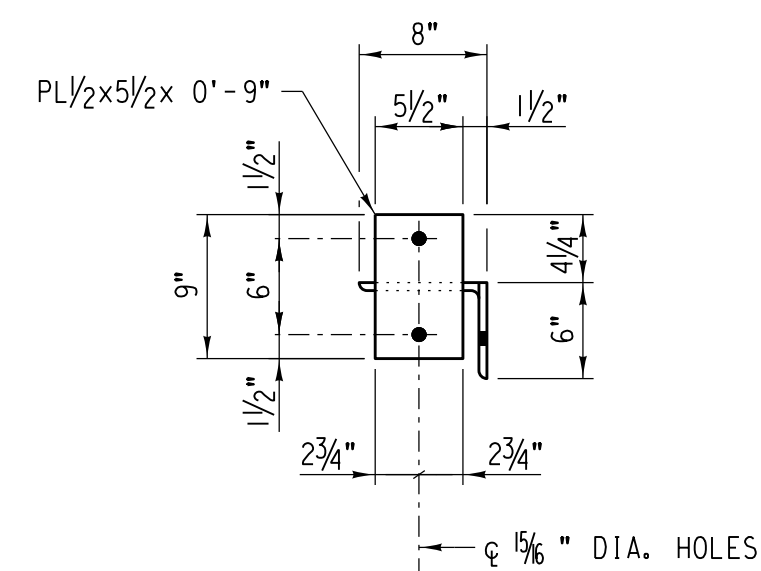
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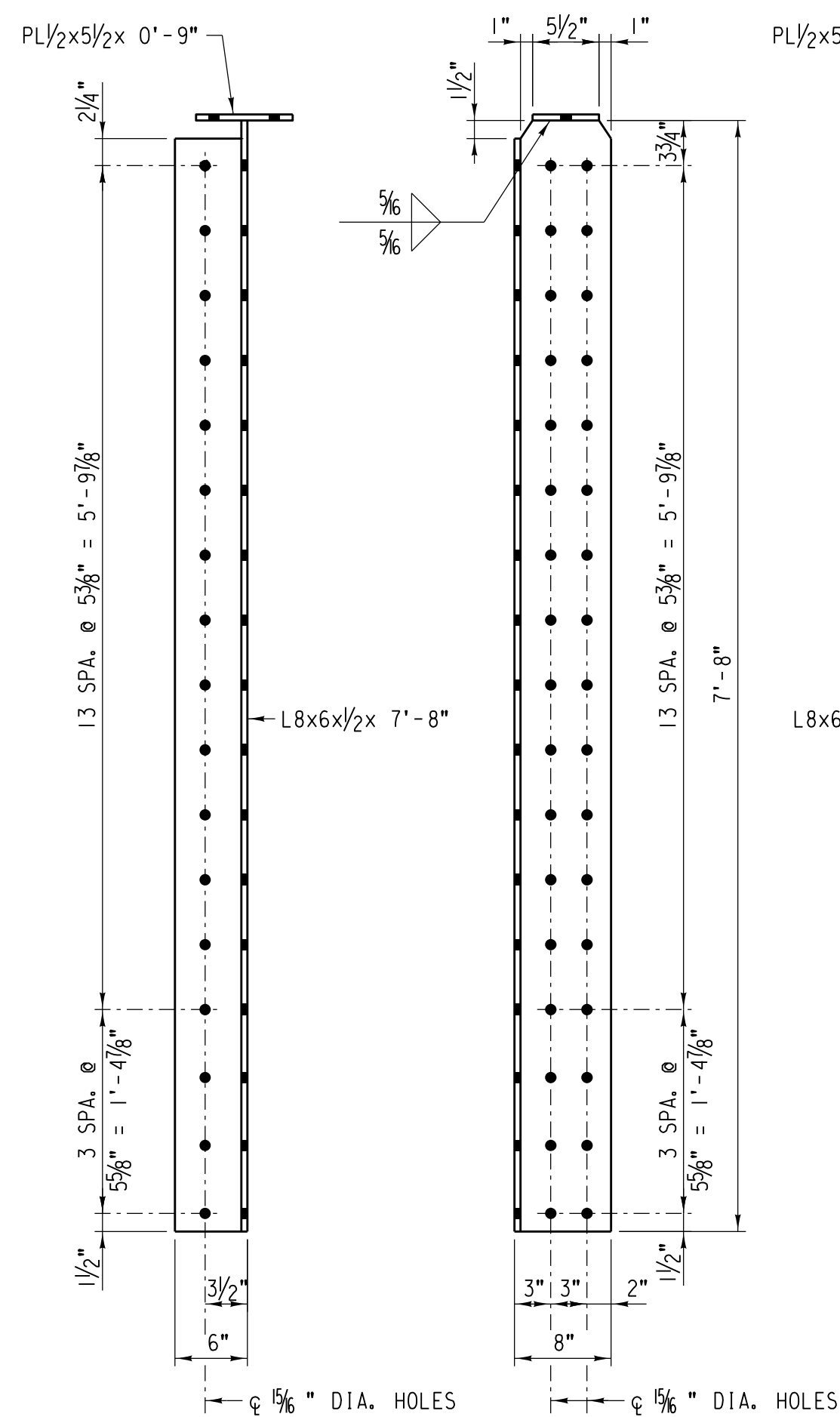
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PLAN



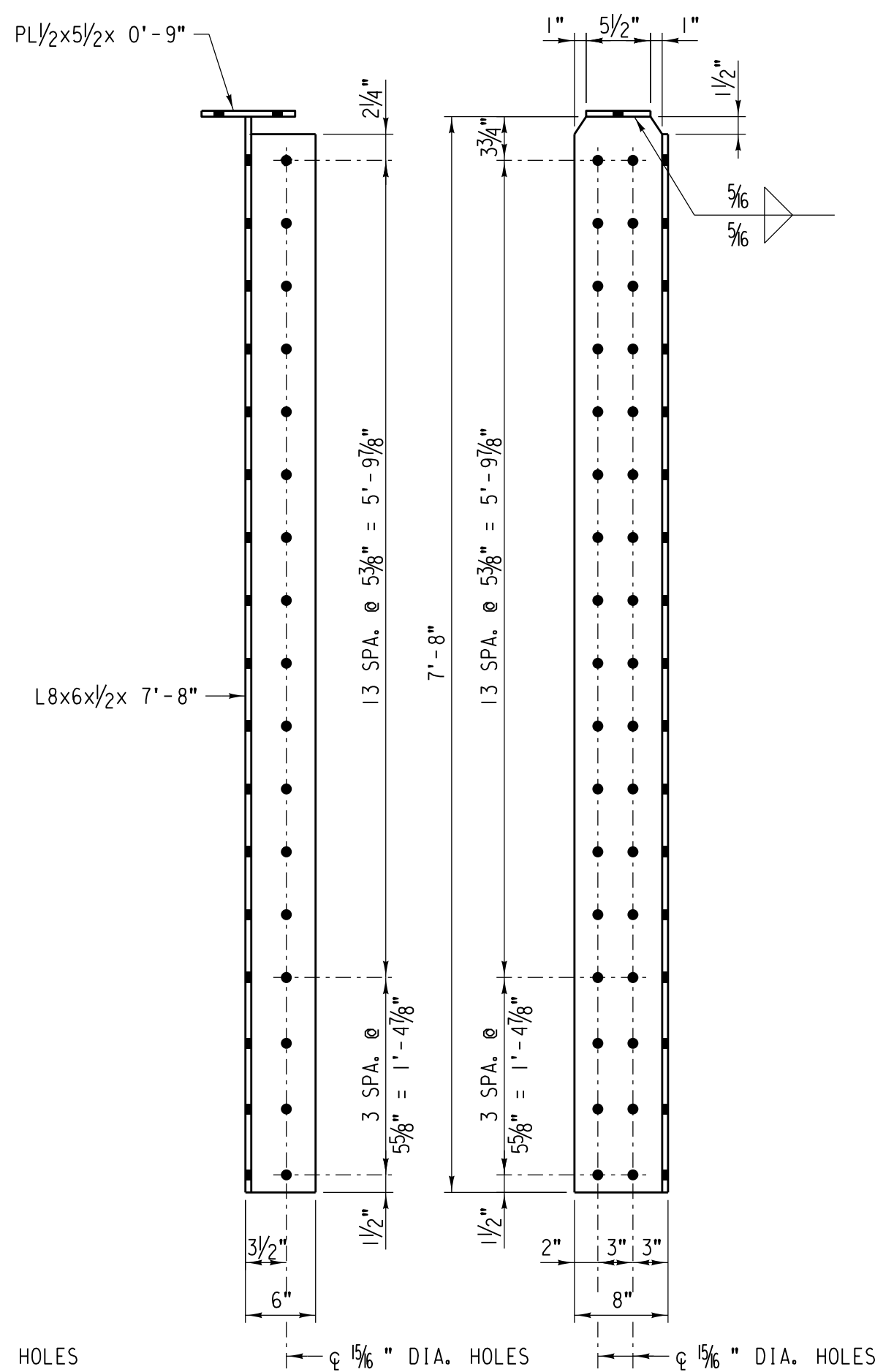
PLAN

ELEVATION

SIDE VIEW

STIFFENER ANGLE SA-1

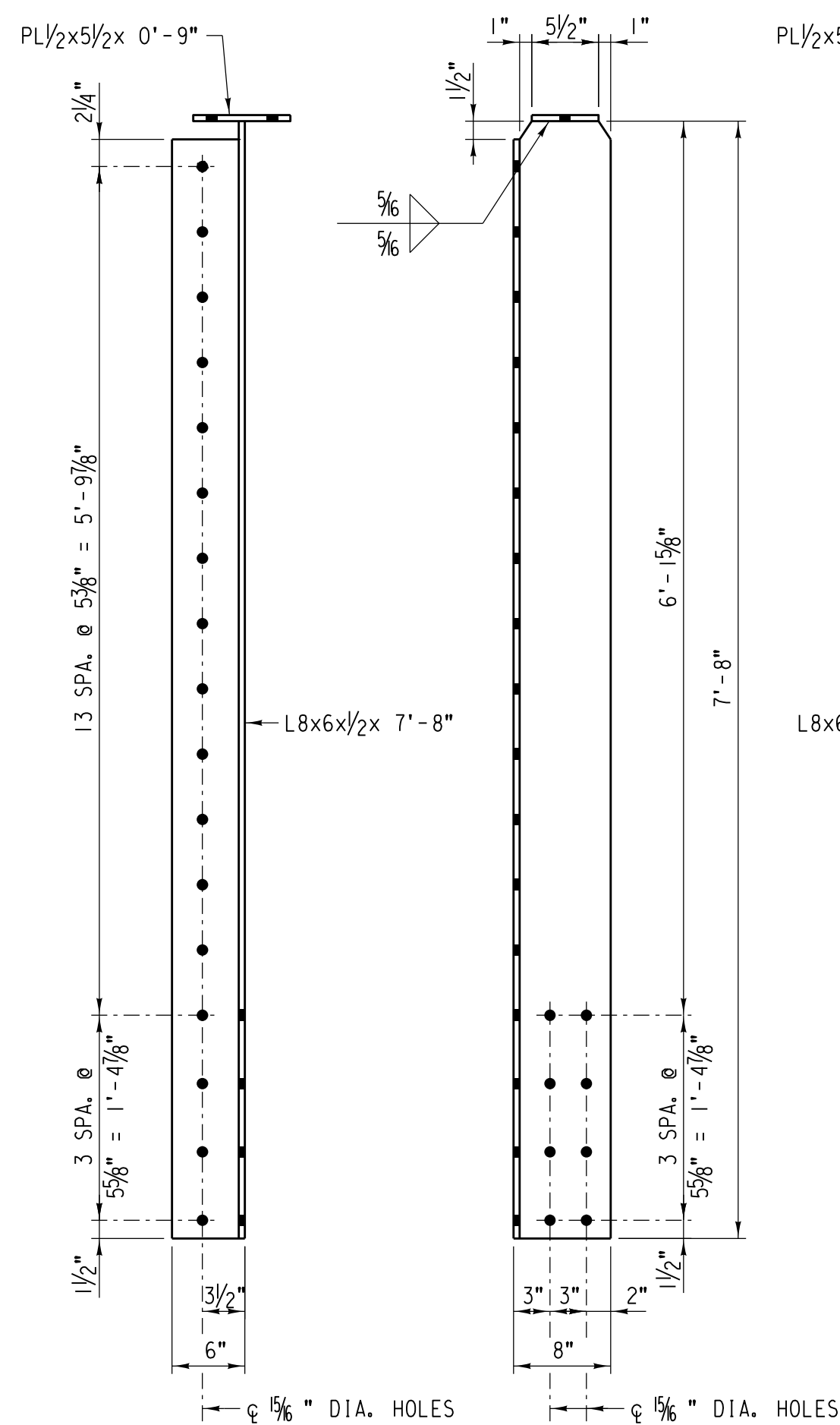
SCALE: EST. WT. = 184 LB. EA. 1" = 1'-0"

ELEVATION

SIDE VIEW

STIFFENER ANGLE SA-2

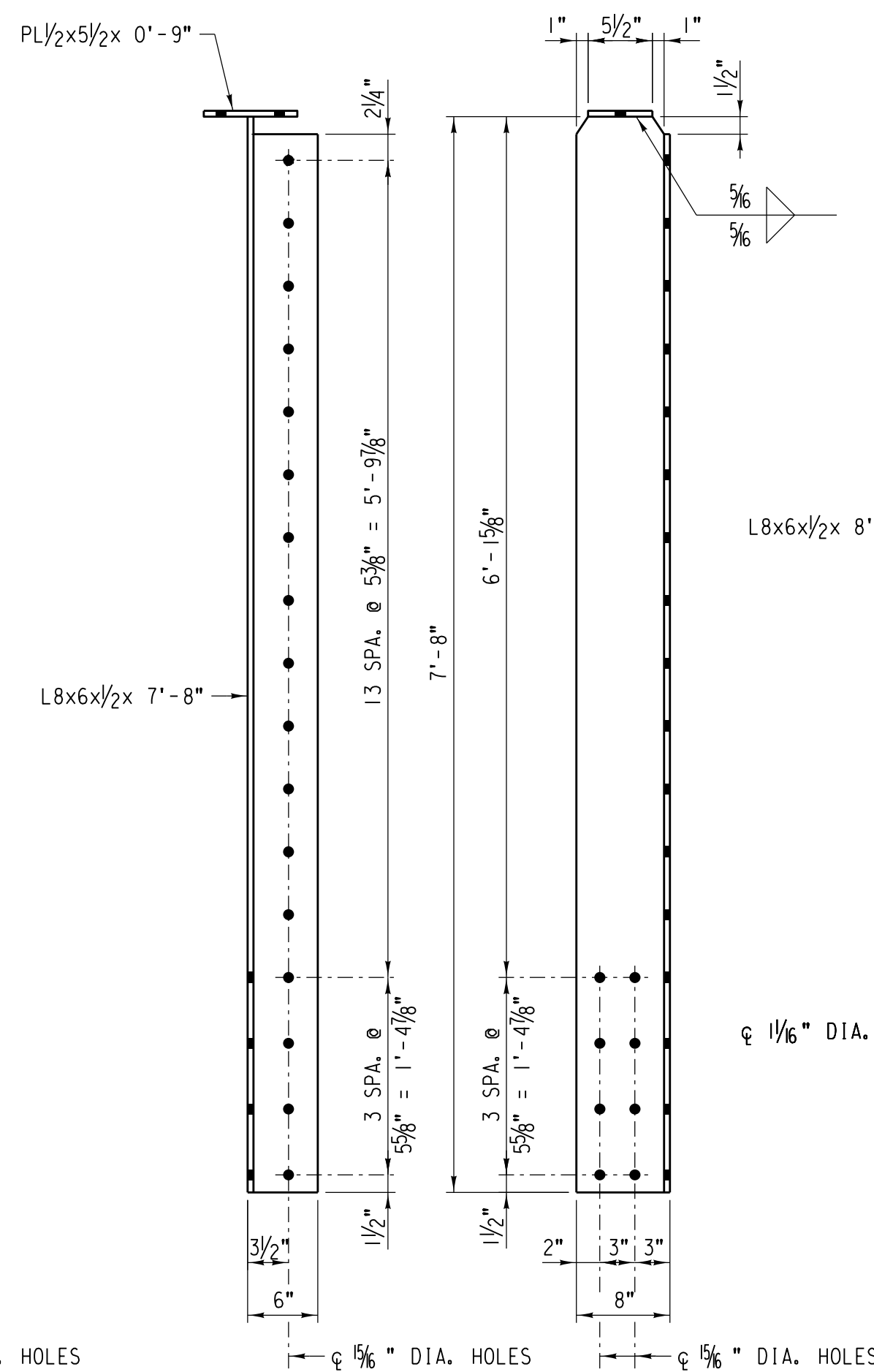
SCALE: EST. WT. = 184 LB. EA. 1" = 1'-0"

ELEVATION

SIDE VIEW

STIFFENER ANGLE SA-3

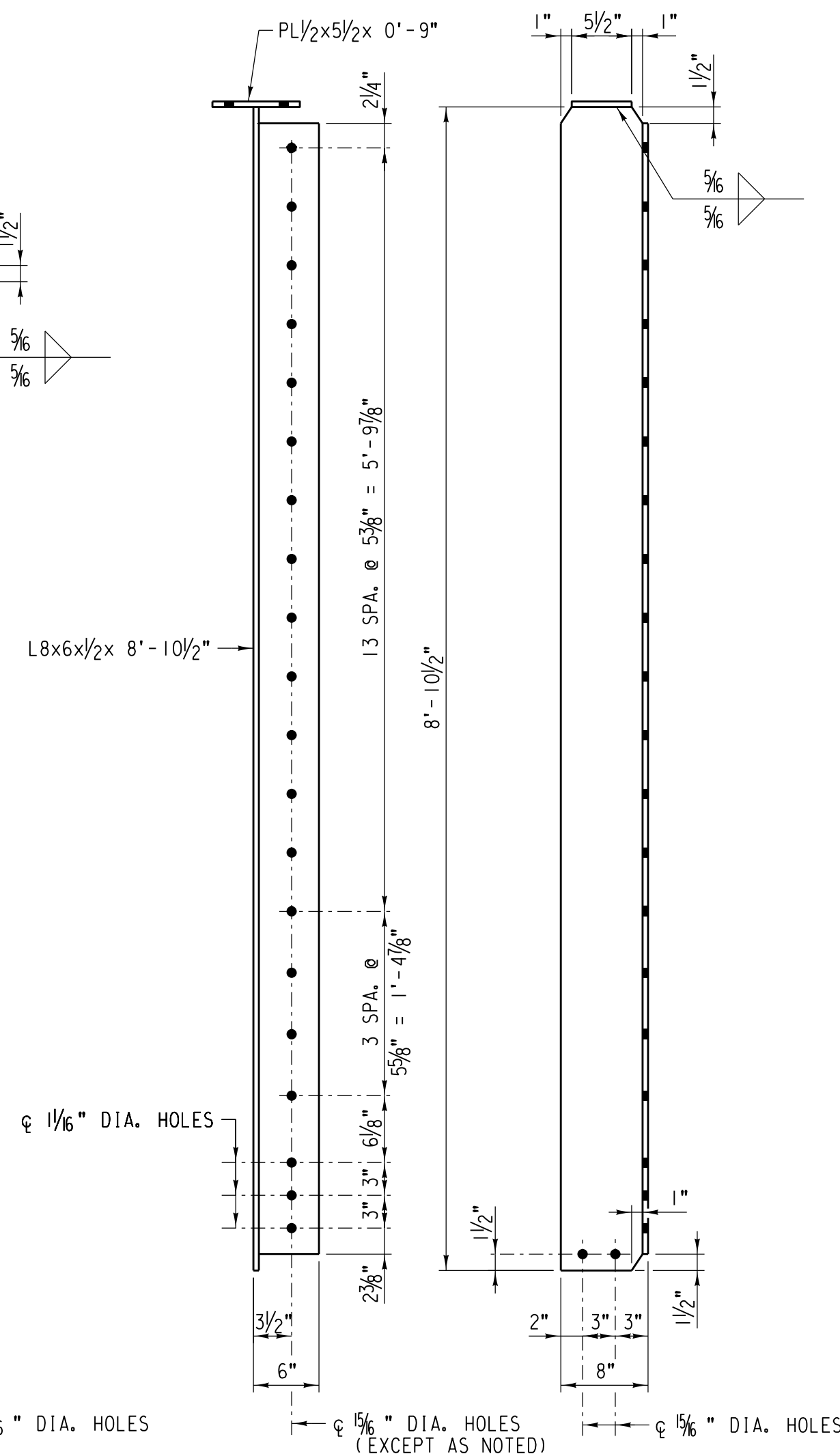
SCALE: EST. WT. = 184 LB. EA. 1" = 1'-0"

ELEVATION

SIDE VIEW

STIFFENER ANGLE SA-4

SCALE: EST. WT. = 184 LB. EA. 1"=1'-0"

ELEVATION

SIDE VIEW

STIFFENER ANGLE SA-5

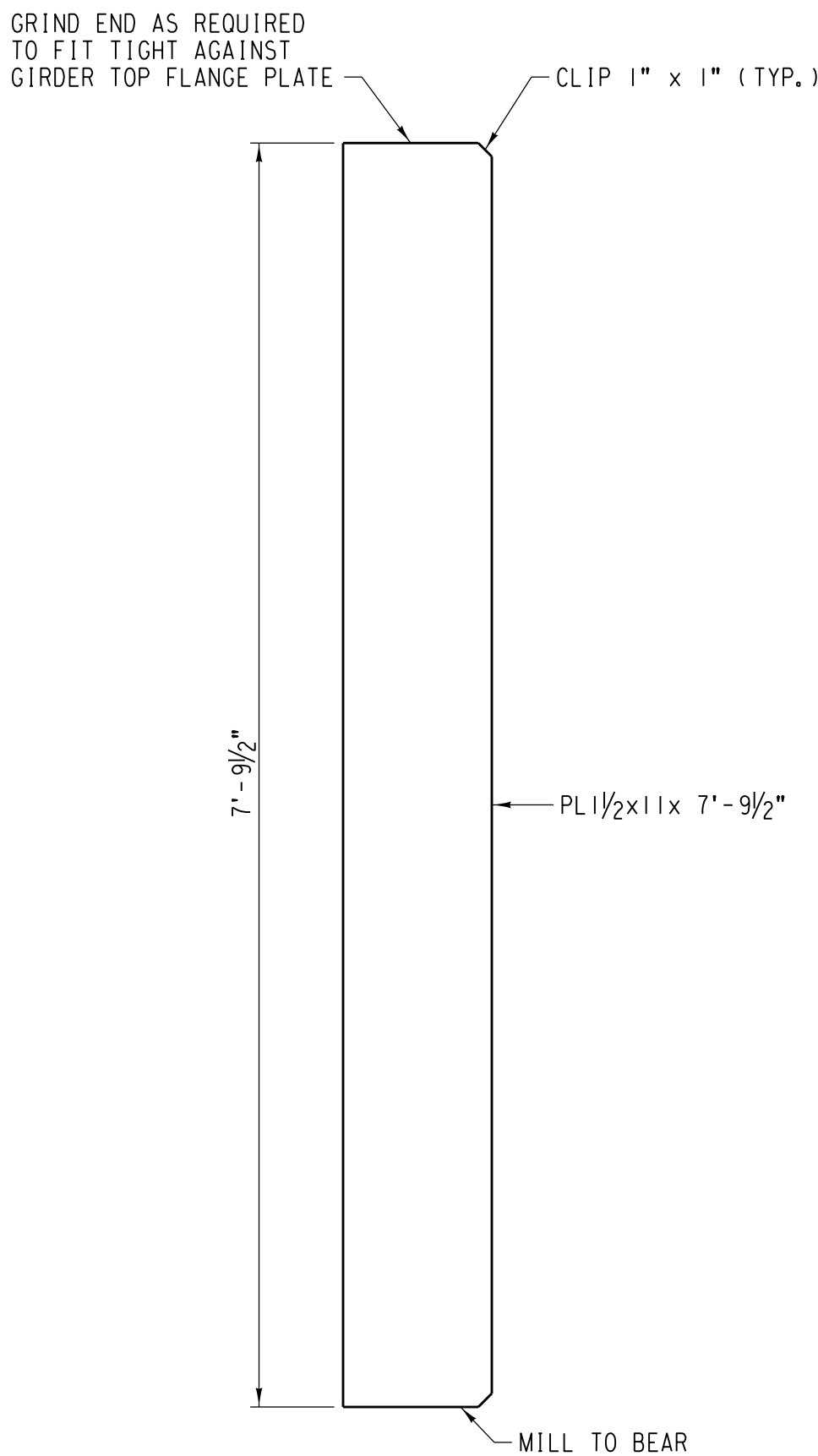
SCALE: EST. WT. = 212 LB. EA. 1" = 1'-0"

NOTE:
FOR TPG STRUCTURAL STEEL NOTES,
SEE SHEET NO. E25.

	/ /	
	/ /	
NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		<u>05/28/2021</u>
STATUS		DATE
 benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		<u>05/28/2021</u>
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C E NUMBER:
	31876	122525

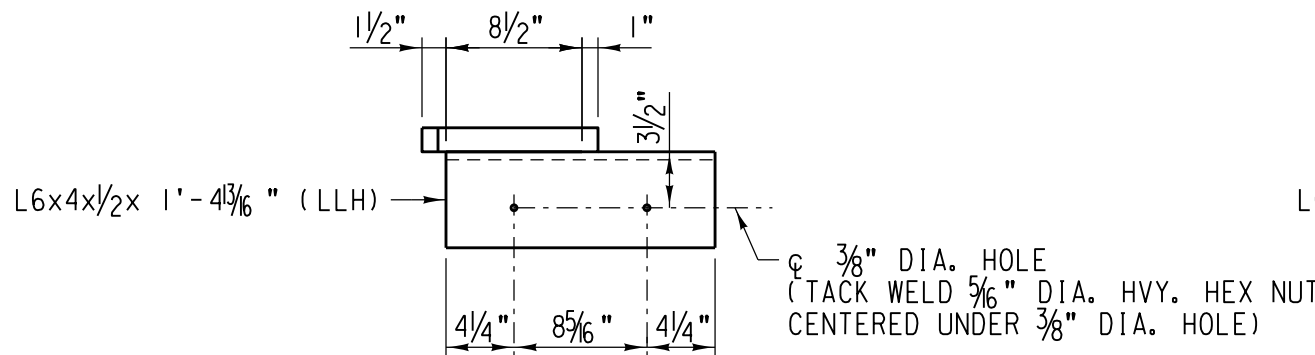
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: RR /MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E33 of E43	SHEET TITLE: TPG STIFFENER ANGLE DETAILS			

FILE NAME: C:\user\srini\ez\desk\tp\000093.d11.dgn

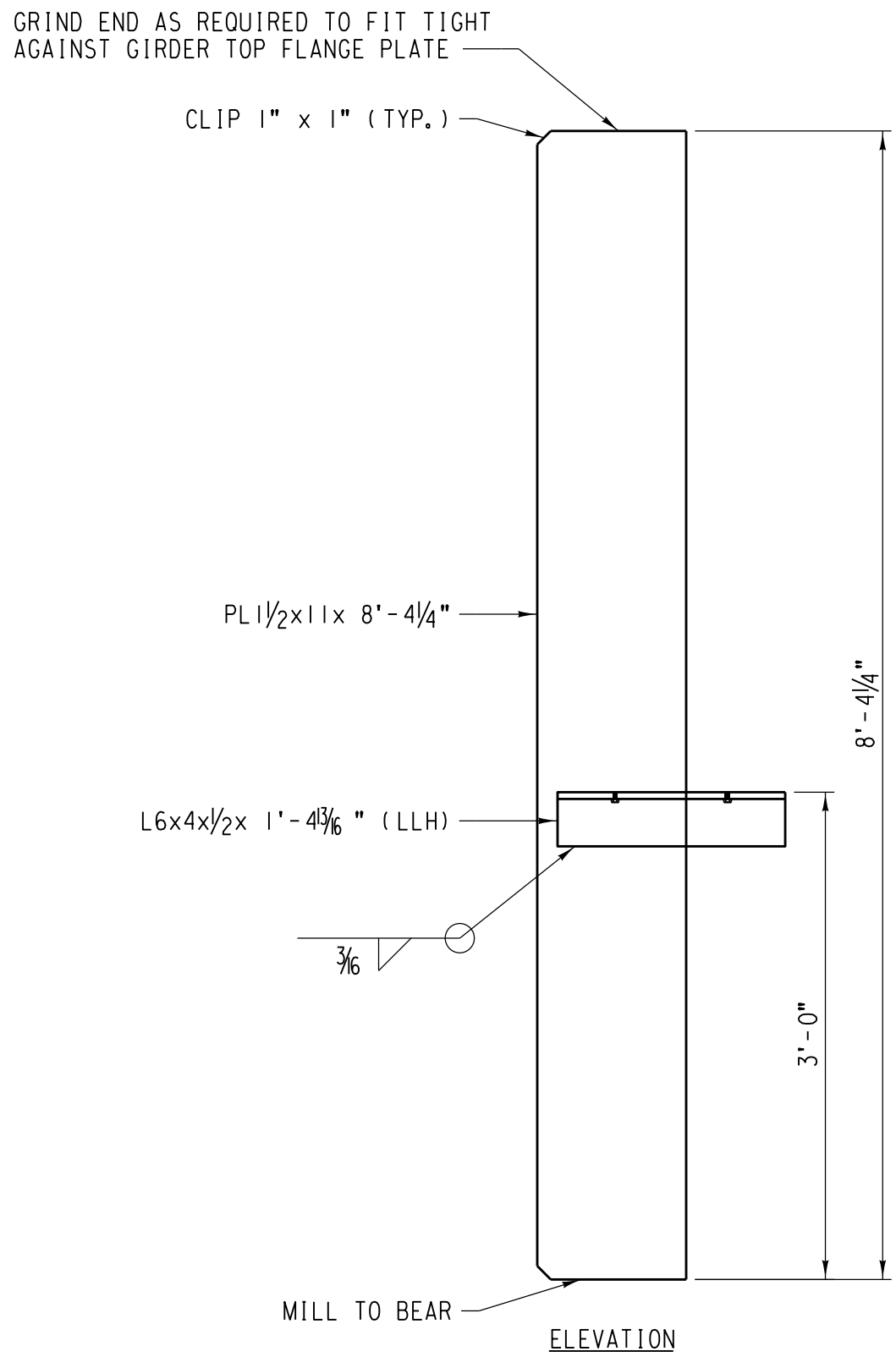


BEARING STIFFENER BS-1

SCALE: EST. WT. = 438 LB. EA. 1"=1'-0"

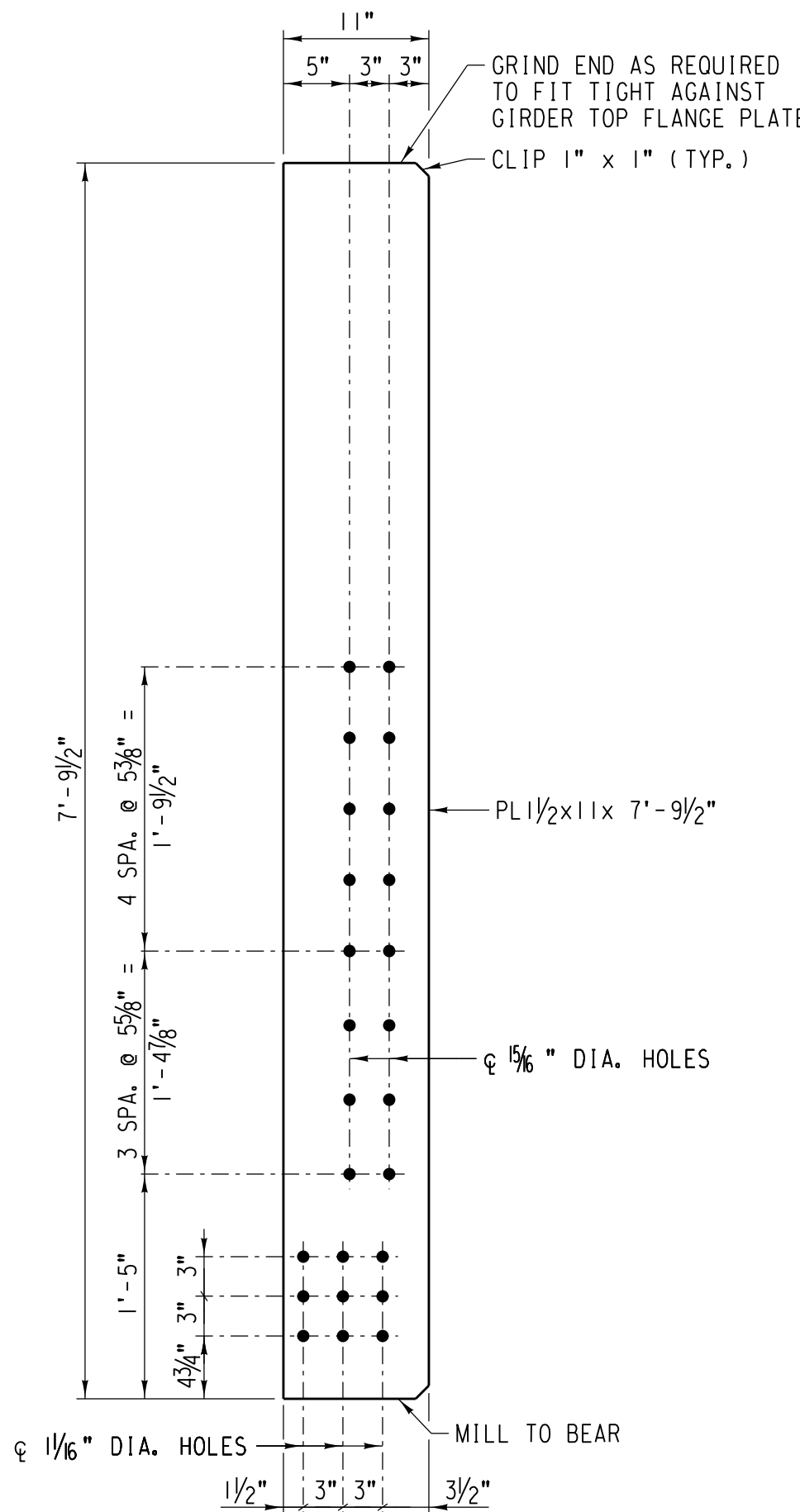


SECTION



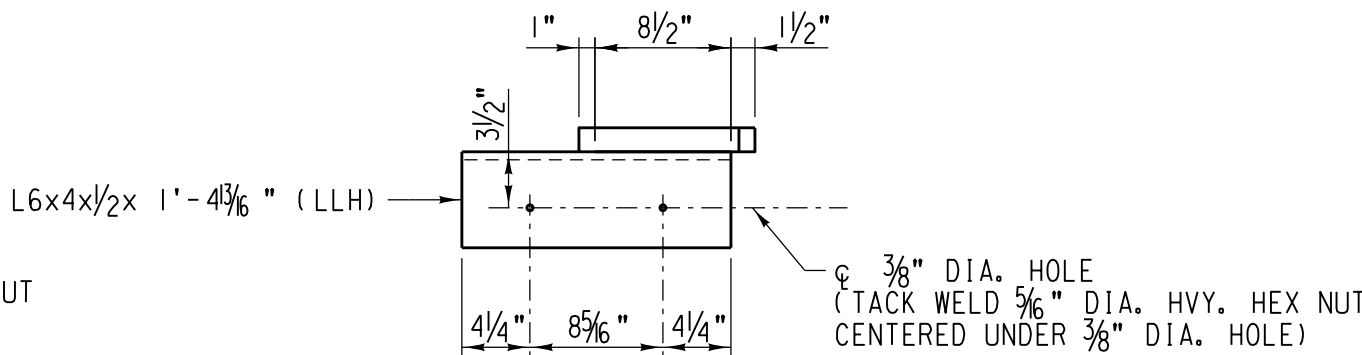
BEARING STIFFENER BS-5

SCALE: EST. WT. = 492 LB. EA. 1"=1'-0"

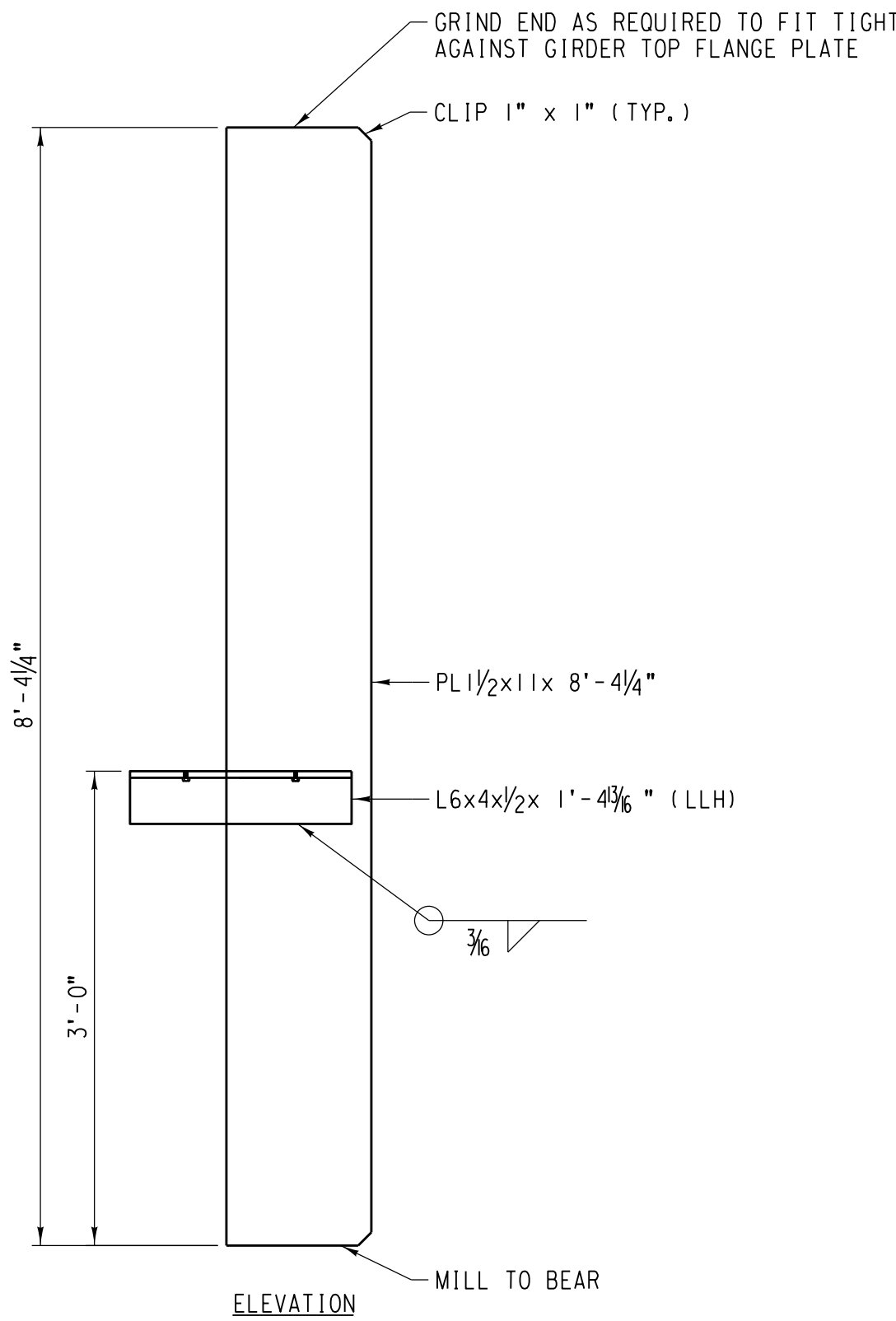


BEARING STIFFENER BS-2

SCALE: EST. WT. = 438 LB. EA. 1"=1'-0"

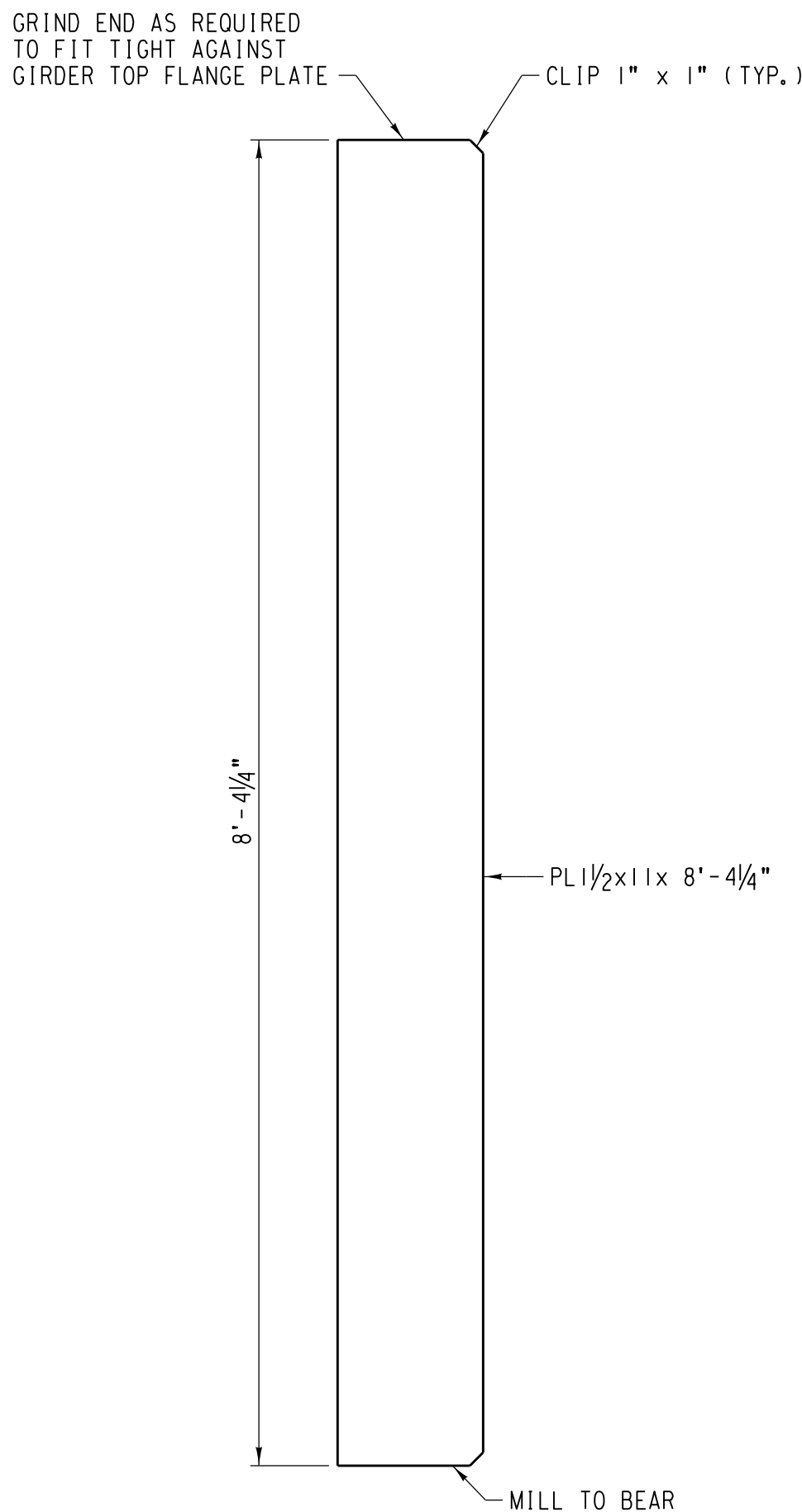


SECTION



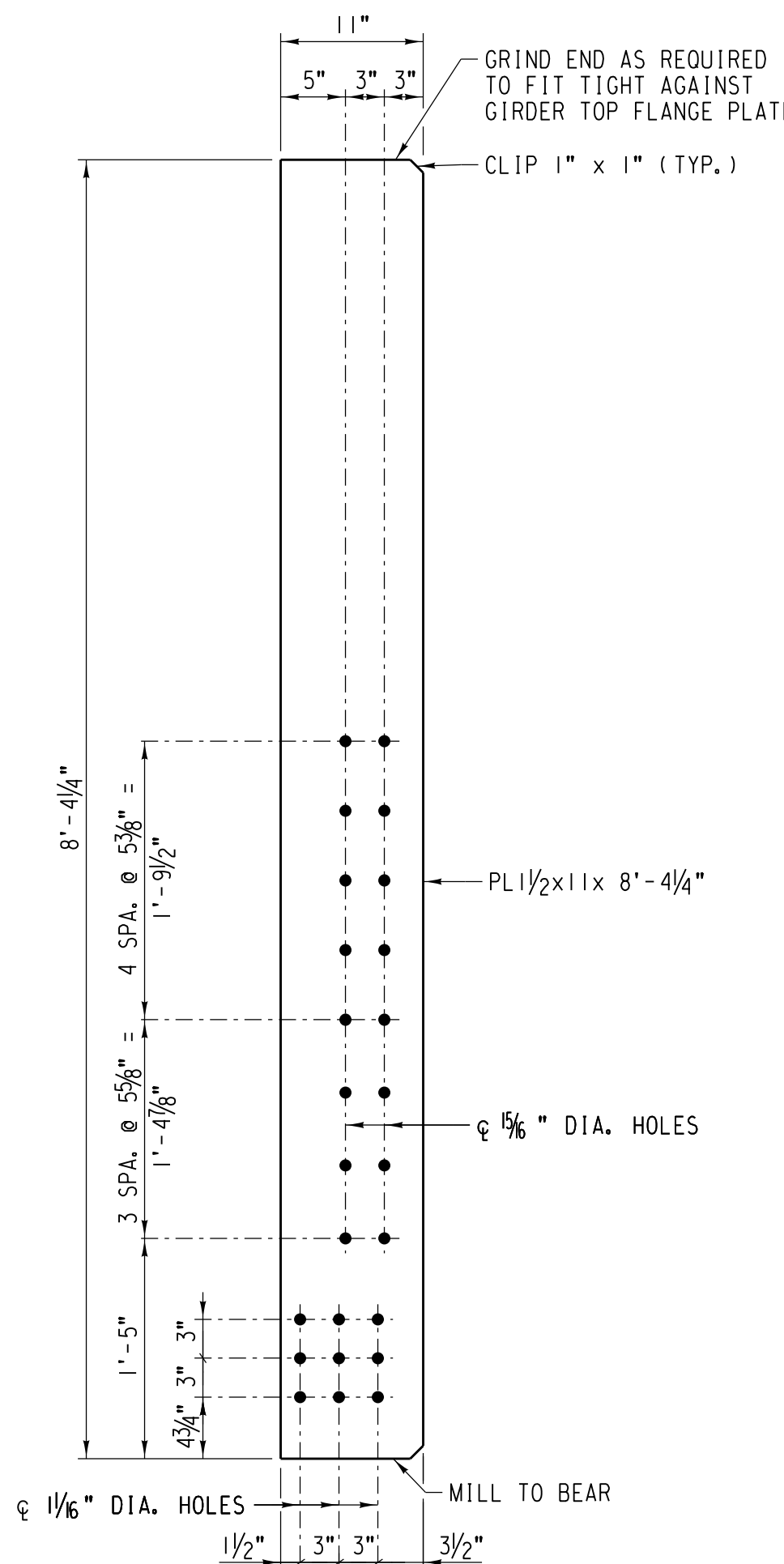
BEARING STIFFENER BS-6

SCALE: EST. WT. = 492 LB. EA. 1"=1'-0"



BEARING STIFFENER BS-3

SCALE: EST. WT. = 469 LB. EA. 1"=1'-0"



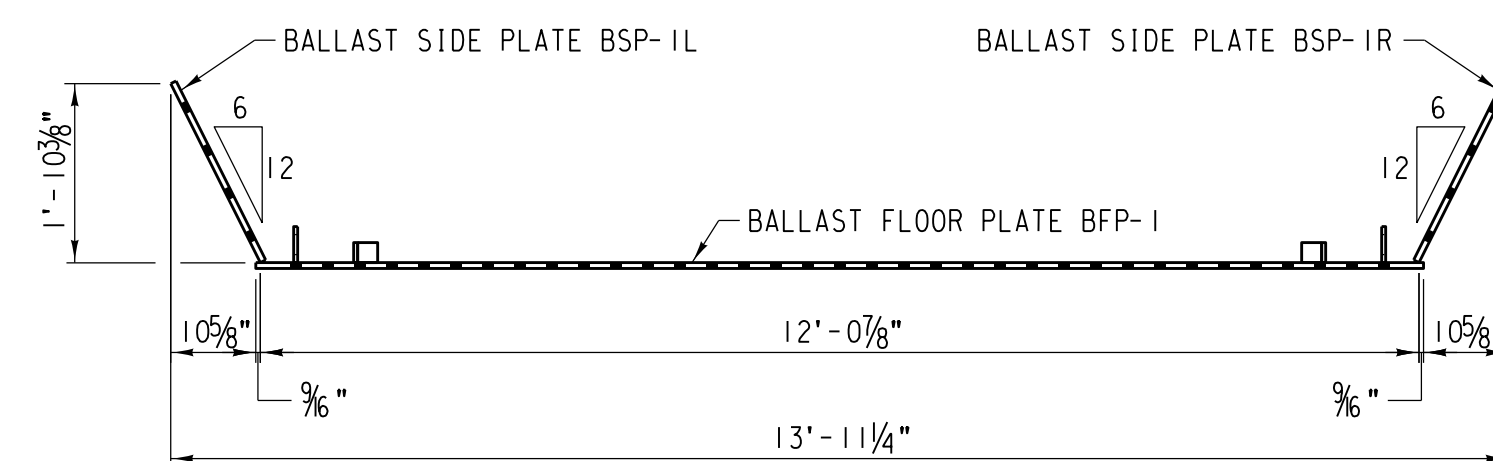
BEARING STIFFENER BS-4

SCALE: EST. WT. = 469 LB. EA. 1"=1'-0"

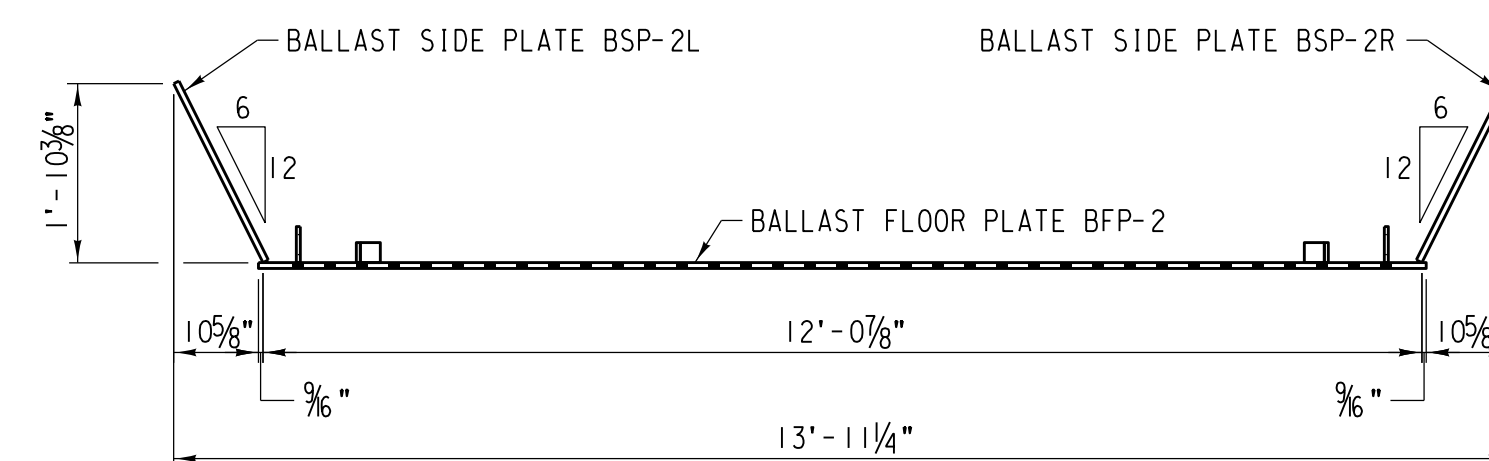
- NOTES:
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. 25.
 - LLH = LONG LEG HORIZONTAL

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

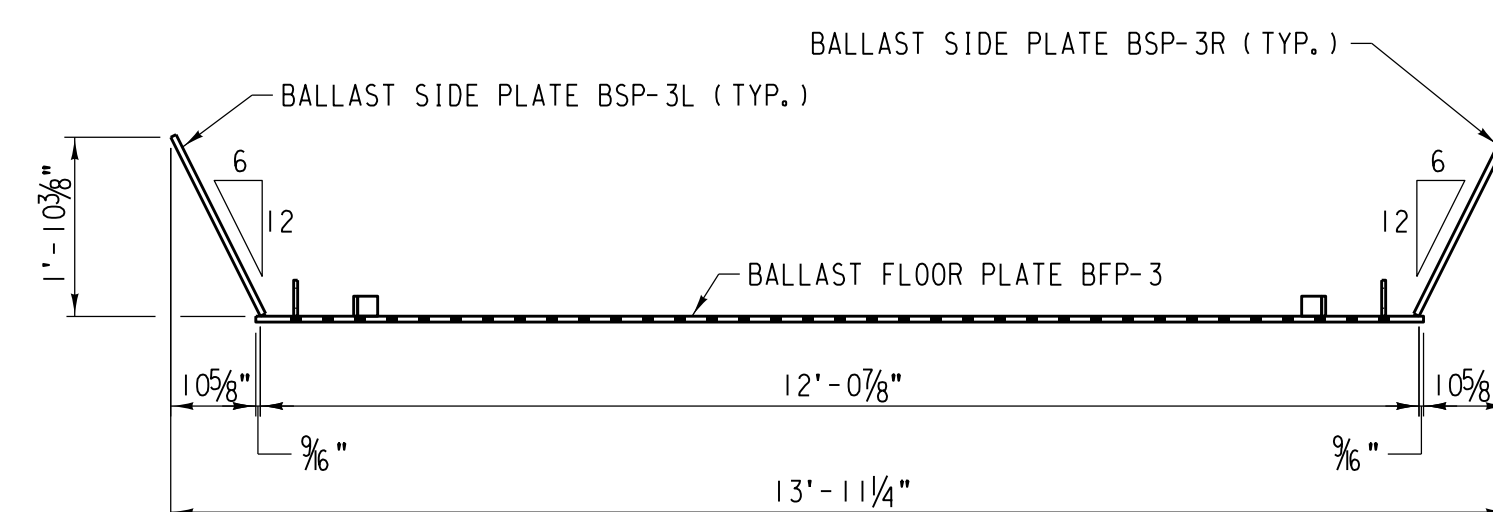
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSNCHK BY:	UNION PACIFIC RAILROAD	
	FNF/MFB		
	DRAWN/CHK BY:	Office of Director Structures Design	
	RR /MFB	LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E34 of E43		SHEET TITLE:	
		TPG BEARING STIFFENER DETAILS	



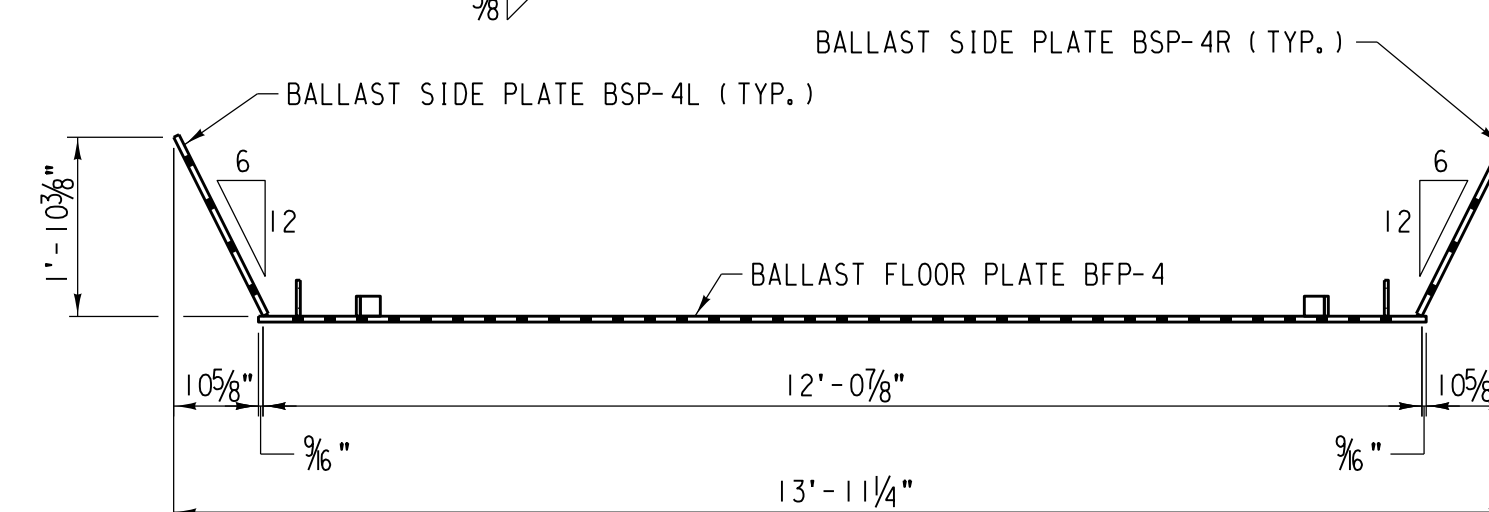
BALLAST PAN BP- I



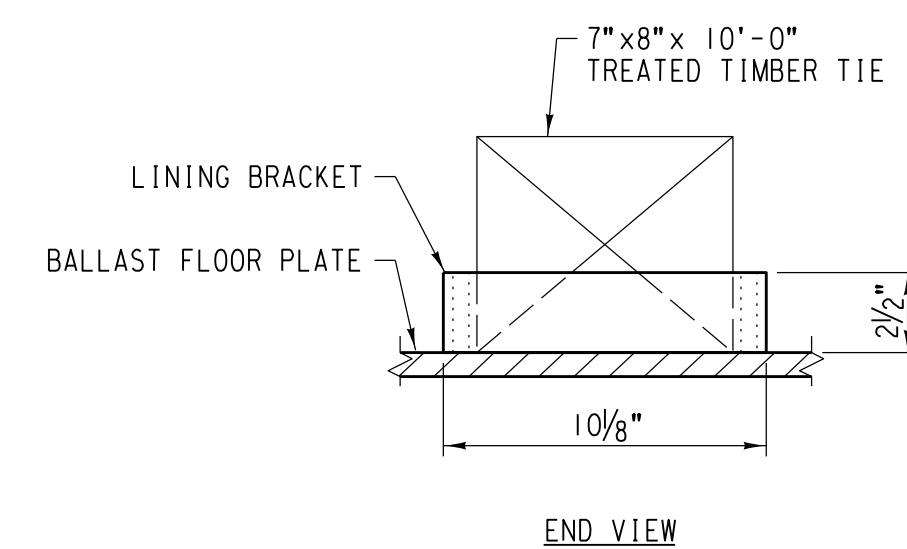
BALLAST PAN BP-2



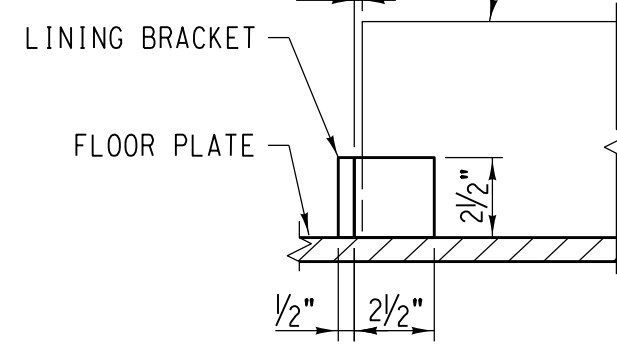
BALLAST PAN BP-3



BALLAST PAN BP-4



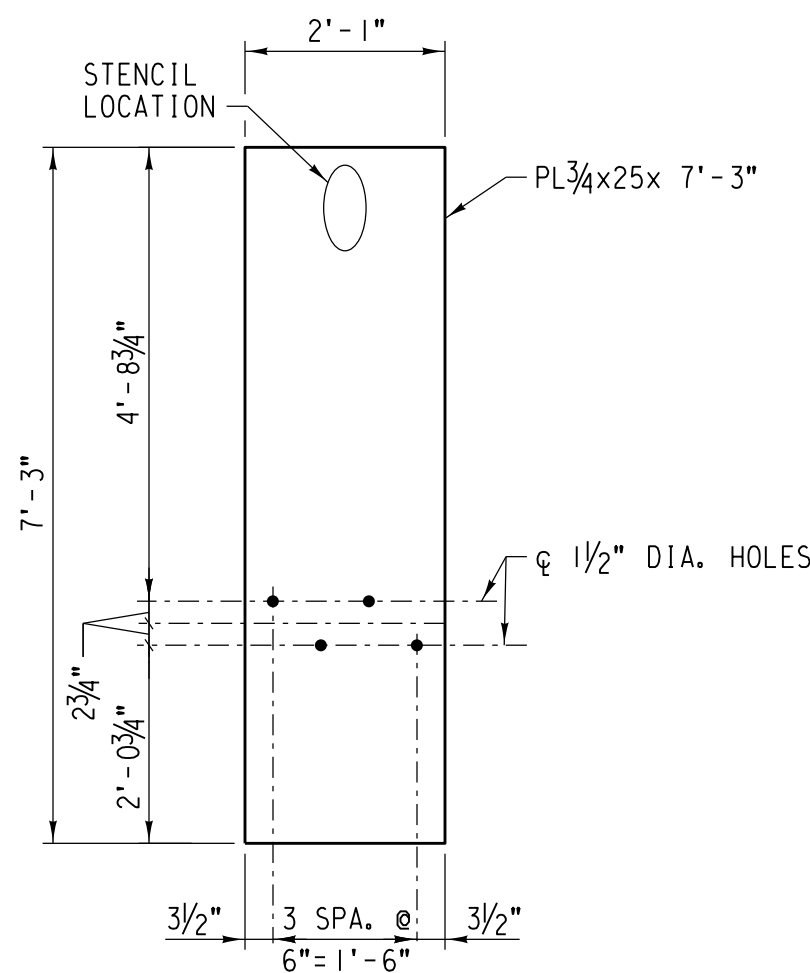
LINING BRACKET DETAIL
 SCALE: 3"=1'-0"



- NOTES:**
1. FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. E25.
 2. ESTIMATED WEIGHT OF THE BALLAST P EXCLUDE THE WEIGHT OF THE TIE LIN BRACKETS.

	/	/	
	/	/	
NO.	DATE	REVISIONS	
COMPLETION STATUS:			
FINAL		<u>05/28/2021</u>	
STATUS		DATE	
			
APPROVED FOR UNION PACIFIC RAILROAD BY:			
MATTHEW BECKER		<u>05/28/2021</u>	
CONSULTANT ENGINEER		DATE	
PROJECT ID:	WORK ORDER:	C E NUMBER:	
	31876	122525	

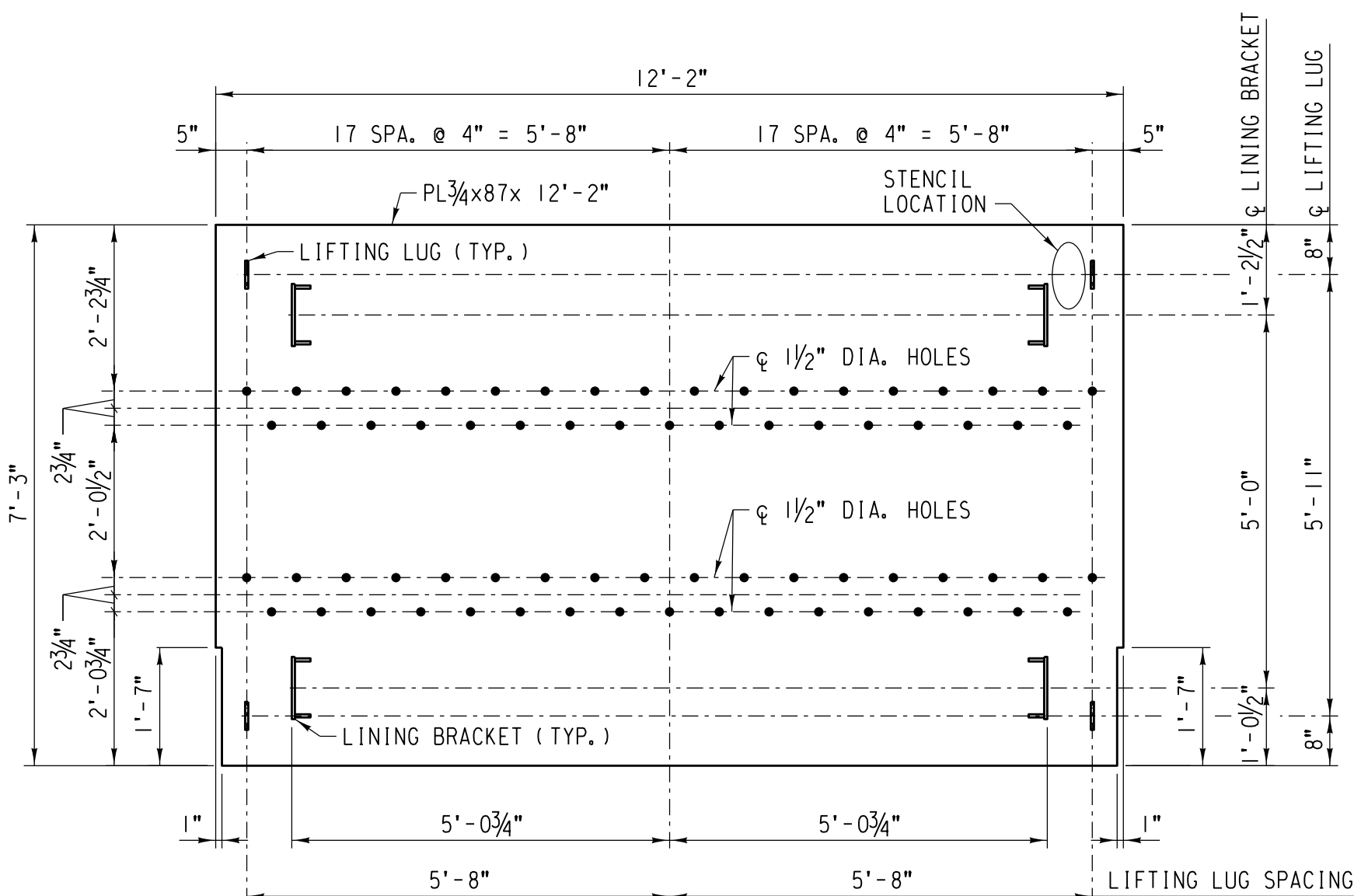
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: RR /MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO: E35 of E43	SHEET TITLE: TPG BALLAST PAN DETAILS			



PLAN

BALLAST SIDE PLATE BSP-1L/1R

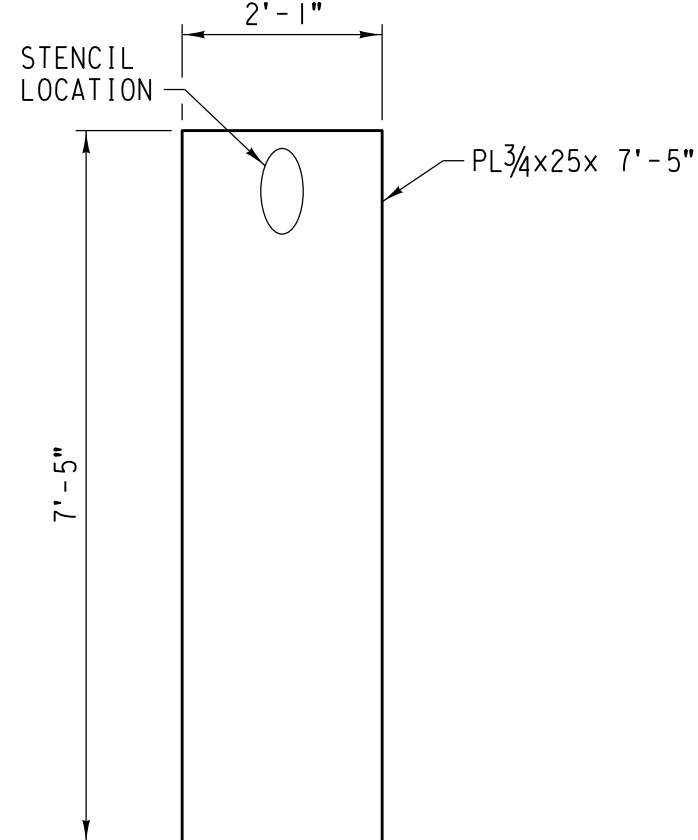
SCALE: EST. WT. = 463 LB. EA.
(BSP-1L SHOWN, BSP-1R OPPOSITE HAND)



PLAN

BALLAST FLOOR PLATE BFP-1

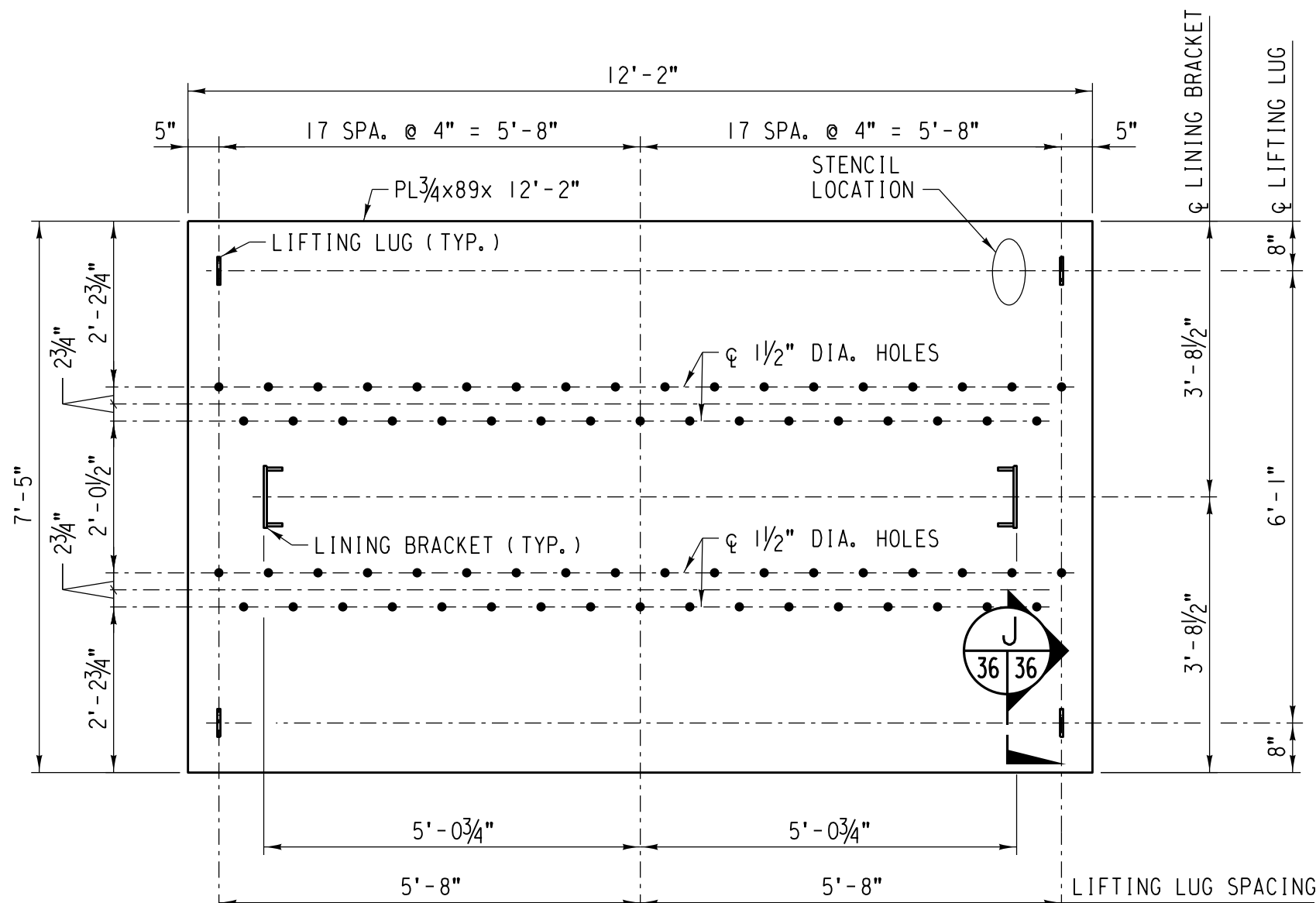
SCALE: EST. WT. = 2,713 LB. EA.



PLAN

BALLAST SIDE PLATE BSP-3L/3R

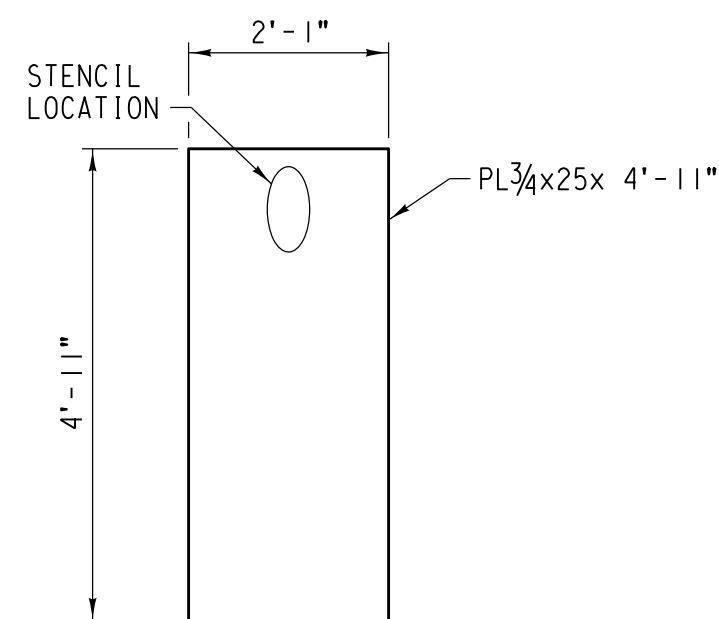
SCALE: EST. WT. = 473 LB. EA.
(BSP-3L SHOWN, BSP-3R OPPOSITE HAND)



PLAN

BALLAST FLOOR PLATE BFP-3

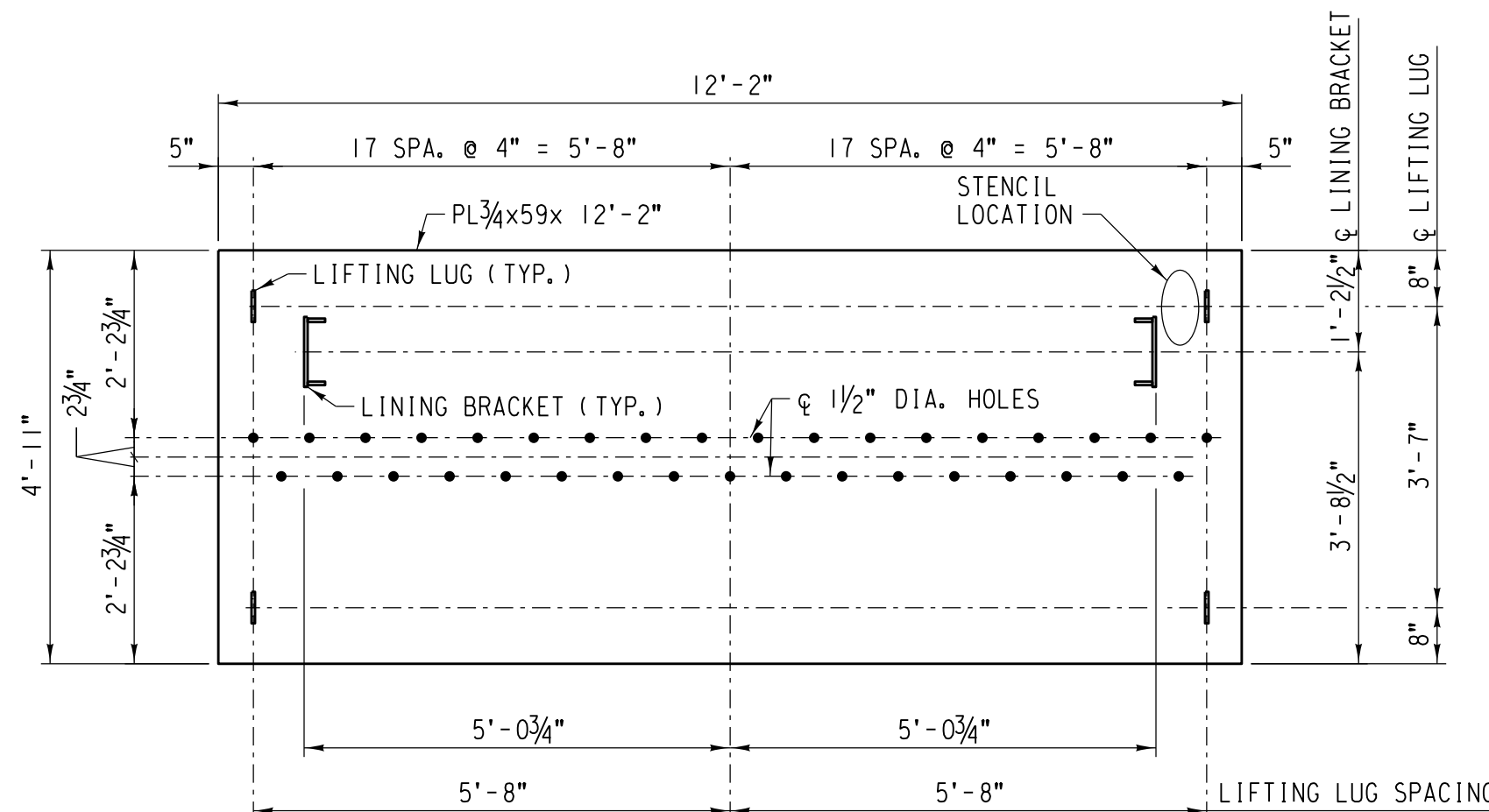
SCALE: EST. WT. = 2,776 LB. EA.



PLAN

BALLAST SIDE PLATE BSP-2L/2R

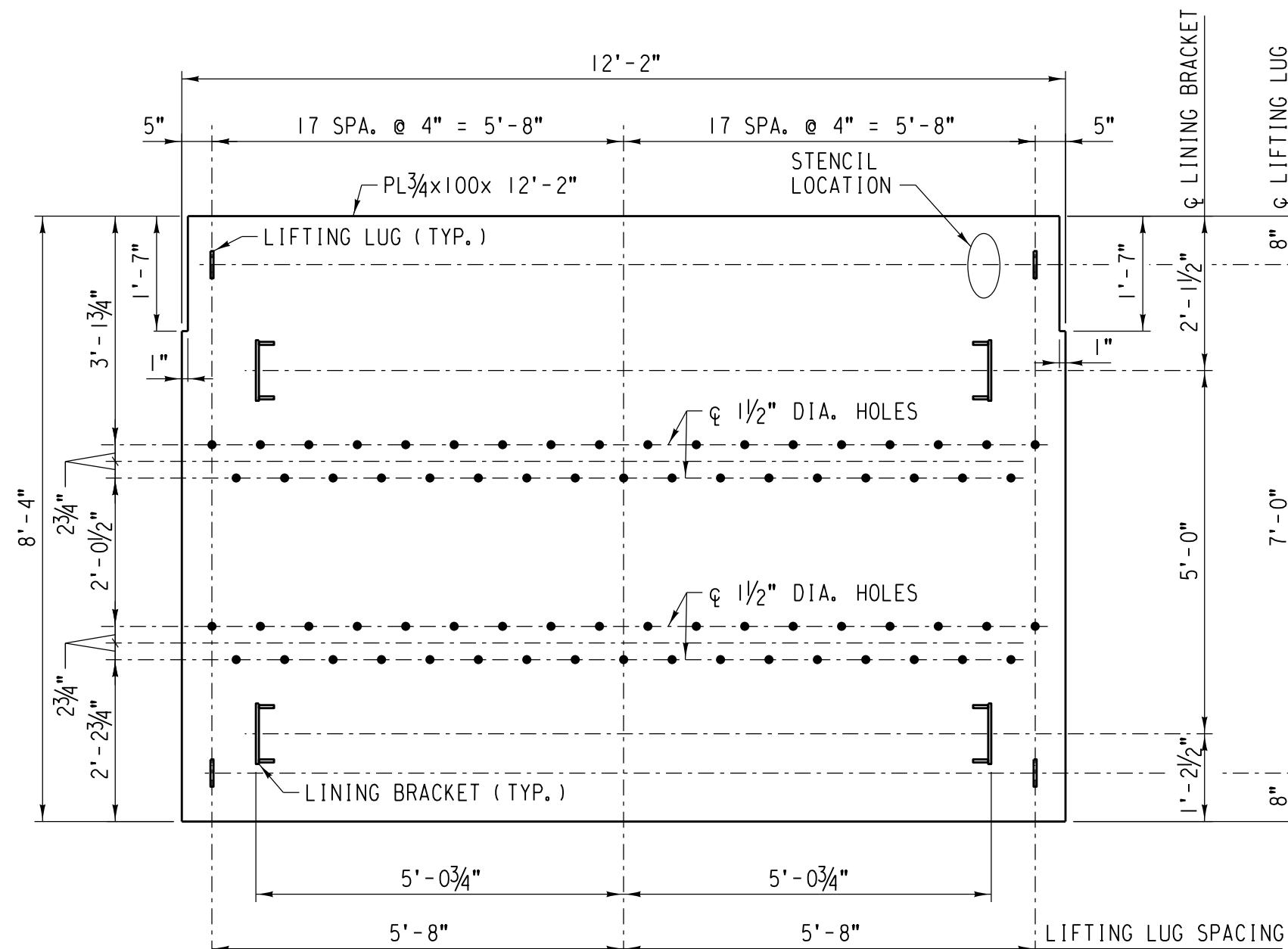
SCALE: EST. WT. = 314 LB. EA.
(BSP-2L SHOWN, BSP-2R OPPOSITE HAND)



PLAN

BALLAST FLOOR PLATE BFP-2

SCALE: EST. WT. = 1,844 LB. EA.

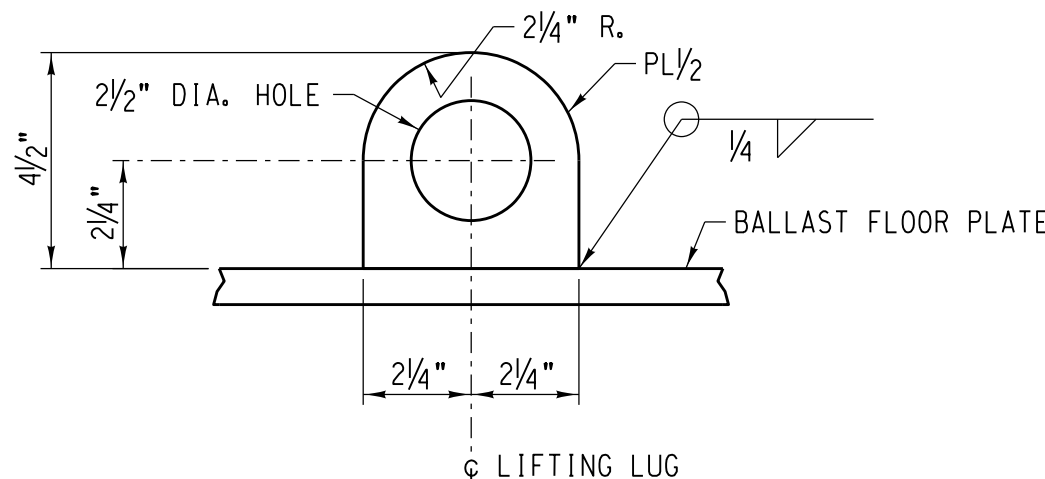


PLAN

BALLAST FLOOR PLATE BFP-4

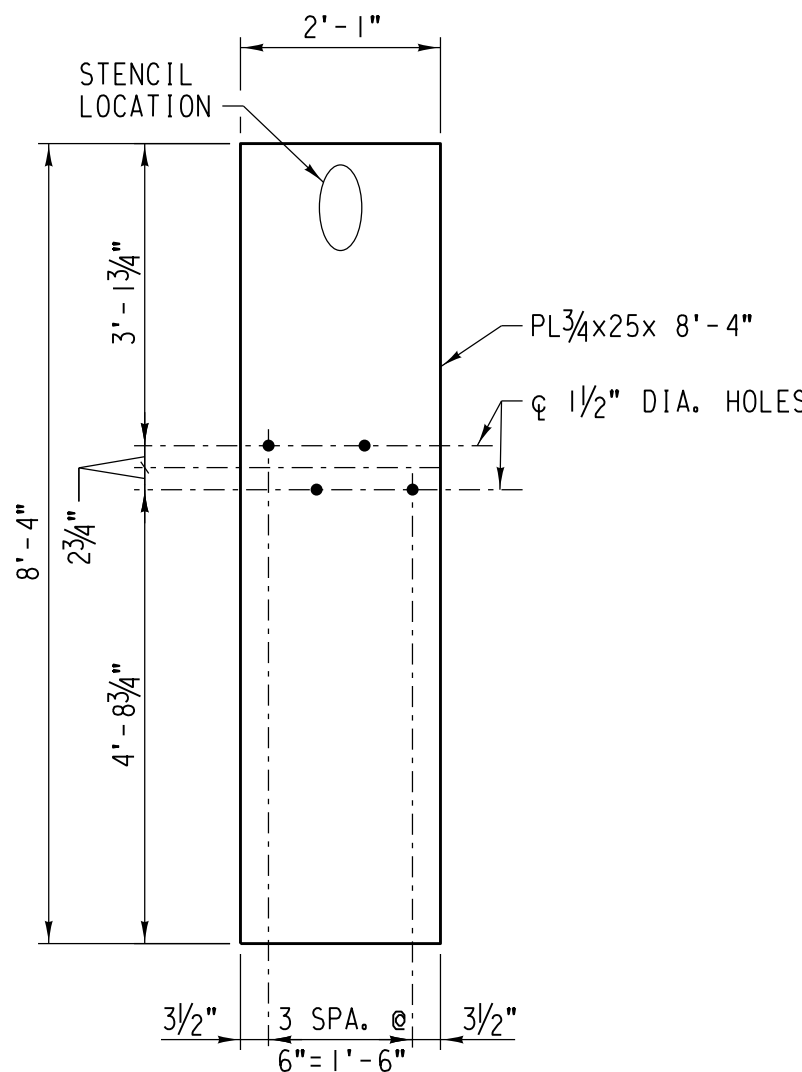
SCALE: EST. WT. = 3,117 LB. EA.

- NOTES:
- FOR TPG STRUCTURAL STEEL NOTES, SEE SHEET NO. E25.
 - ESTIMATED WEIGHT OF THE BALLAST PANS EXCLUDE THE WEIGHT OF THE TIE LINING BRACKETS.



SECTION J

SCALE: 3" = 1'-0"



PLAN

BALLAST SIDE PLATE BSP-4L/4R

SCALE: EST. WT. = 431 LB. EA.
(BSP-4L SHOWN, BSP-4R OPPOSITE HAND)

NO.	DATE	REVISIONS
COMPLETION STATUS:		
FINAL		05/28/2021
STATUS		DATE
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		05/28/2021
CONSULTANT ENGINEER		DATE
PROJECT ID:	WORK ORDER:	C/E NUMBER:
	31876	122525

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DESIGNED BY:	UNION PACIFIC RAILROAD	
	FN/MFB		
	DRAWN/CHECK BY:	Office of Director Structures Design	
RR /MFB		LOCATION & DESCRIPTION:	
UPRR ENGINEER:		BRIDGE 0.93 ROCKWELL SUBDIVISION	
DEH/ADS		1 SPAN TPG x 90'	
SHT NO:		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
E36 of E43		TPG BALLAST PLATE DETAILS	

FILE NAME: C:\user\sr\cmr\ez\Bask\tp\00000931.d11.dgn

GIRDER STRESS TABLE																					
SPAN LENGTH (L)	GIRDER LOCATION	TOP FLANGE SIZE	BOTTOM FLANGE SIZE	WEB SIZE	SHEAR								MOMENT						LIVE LOAD + IMPACT DEFLECTION		
					DEAD LOAD (k)	LIVE LOAD		IMPACT (k)	TOTAL (k)	WEB AREA (Aw)		DEAD LOAD (k-ft)	LIVE LOAD		IMPACT (k-ft)	TOTAL (k-ft)	SECTION MODULUS (S)		ALLOWABLE (in)	MOMENT OF INERTIA (I)	
						E 80 (k)	ALTERNATE (k)			REQUIRED (in ²)	PROVIDED, GROSS (in ²)		E 80 (k-ft)	ALTERNATE (k-ft)			REQUIRED (in ³)	PROVIDED, NET (in ³)		REQUIRED (in ⁴)	PROVIDED, GROSS (in ⁴)
90'-0"	***COMMON	3" x 24"	3" x 24"	**7/8" x 963/4"	294.0	535.9	-	155.3	985.2	56.3	84.7	6,432	10,168	-	2,947	19,547	8,530	9,065	1.64	379,888	535,518

NOTES:
L = LONGITUDINAL DIMENSION, OUT TO OUT OF GIRDERS
** WEB SIZE AT LOCATION OF MAXIMUM SHEAR (ϕ BEARING)
*** ALL GIRDERS ARE DESIGNED AS COMMON GIRDERS

FLOORBEAM STRESS TABLE																				
LENGTH (L _f)	FLOORBEAM LOCATION	BEAM SIZE OR FLANGE SIZE	BEAM SIZE OR WEB SIZE	SHEAR							MOMENT							LIVE LOAD + IMPACT DEFLECTION		
				DEAD LOAD (k)	LIVE LOAD		IMPACT (k)	TOTAL (k)	WEB AREA (A _w)		DEAD LOAD (k-ft)	LIVE LOAD		IMPACT (k-ft)	TOTAL (k-ft)	SECTION MODULUS (S)		ALLOWABLE (in)	MOMENT OF INERTIA (I)	
					E 80 (k)	ALTERNATE (k)			REQUIRED (in ²)	PROVIDED, GROSS (in ²)		E 80 (k-ft)	ALTERNATE (k-ft)			REQUIRED (in ³)	PROVIDED, NET (in ³)		REQUIRED (in ⁴)	PROVIDED, GROSS (in ⁴)
17'-4"	INTERIOR	W12x152	W12x152	7.0	-	28.8	11.7	47.5	2.8	9.5	36.9	-	177.3	72.1	286.3	***128.6	***209.0	0.33	1,428	1,430
17'-4"	END	2" x 15"	1½" x 10"	7.9	-	28.8	11.7	48.4	2.8	15.0	40.8	-	177.3	72.1	290.2	***129.1	***329.3	0.33	1,428	2,305
17'-4"	**END	2" x 15"	1½" x 10"	171.9	-	-	-	171.9	6.6	15.0	429.7	-	-	-	429.7	***127.4	***329.3	0.33	1,428	2,305

NOTES:
Lf = LENGTH OF FLOORBEAM, ϕ TO ϕ OF GIRDERS
** END FLOORBEAM DESIGN STRESSES ARE BASED ON JACKING LOAD, UTILIZING 150% ALLOWABLE OVERSTRESS.
*** COMPRESSION FLANGE CONTROLS CAPACITY

ONE GIRDER MATERIAL LIST																					
SPAN LENGTH (L)	GIRDER LOCATION	TOP FLANGE	BOTTOM FLANGE	WEB PL $\frac{7}{8}$ x108 (Lw)	BEARING STIFFENER						STIFFENER ANGLE					CONNECTION	GIRDER END PLATE (Eq.)	$\frac{7}{8}$ " DIA. GRADE A325 BOLTS			
		PL3x24x 90"-9 $\frac{5}{8}$ " (Eq.)	PL3x24x 90"-0" (Eq.)		BS-1 (Eq.)	BS-2 (Eq.)	BS-3 (Eq.)	BS-4 (Eq.)	BS-5 (Eq.)	BS-6 (Eq.)	SA-1 (Eq.)	SA-2 (Eq.)	SA-3 (Eq.)	SA-4 (Eq.)	SA-5 (Eq.)	CA-4 (Eq.)		x 5" (Eq.)	x 3 $\frac{1}{4}$ " (Eq.)	x 2 $\frac{3}{4}$ " (Eq.)	x 2 $\frac{1}{2}$ " (Eq.)
90"-0"	COMMON	I	I	89"-10 $\frac{3}{4}$ "	2	2	-	2	2	-	-	20	-	8	-	-	2	56	-	476	-
90"-0"	SIDE	I	I	89"-10 $\frac{3}{4}$ "	3	I	2	I	-	I	10	-	4	-	14	14	2	84	238	-	28

NOTES:
L = LONGITUDINAL DIMENSION, OUT TO OUT OF GIRDER
Lw = LENGTH OF GIRDER WEB

TOTAL MATERIAL LIST																																
SPAN LENGTH (L)	COMMON GIRDER (Eq.)	SIDE GIRDER (Eq.)	END FLOORBEAM		FLOORBEAM				DIAPHRAGM				END BRACKET EB-1 (Eq.)	KNEE BRACE						WALKWAY SUPPORT WS-1 (Eq.)	WALKWAY SUPPORT ANGLE WSA-10 (Eq.)	WALKWAY SUPPORT ANGLE WSA-11 (Eq.)	L6x4x1/2x 1' - 4 3/8" "	CONNECTION ANGLE				END FLOORBEAM CONNECTION PLATE CP-1 (Eq.)	BALLAST PAN			
			EFB-1 (Eq.)	EFB-2 (Eq.)	FB-3 (Eq.)	FB-4 (Eq.)	FB-5 (Eq.)	FB-6 (Eq.)	D-1 (Eq.)	D-2 (Eq.)	D-3 (Eq.)	D-4 (Eq.)		KB-1 (Eq.)	KB-2 (Eq.)	KB-3 (Eq.)	KB-4 (Eq.)	KB-5 (Eq.)	KB-6 (Eq.)					CA-1 (Eq.)	CA-2 (Eq.)	CA-3 (Eq.)	CA-5 (Eq.)		BP-1 (Eq.)	BP-2 (Eq.)	BP-3 (Eq.)	BP-4 (Eq.)
90'-0"	1	2	2	2	26	38	2	2	64	8	8	2	8	20	20	2	2	2	2	16	2	2	4	272	264	16	64	16	2	16	10	2

TOTAL MATERIAL LIST (CON'T.)																											
SPAN LENGTH (L)	FIXED BEARING ASSEMBLY (COMMON) (Eq.)	FIXED BEARING ASSEMBLY (SIDE) (Eq.)	EXPANSION BEARING ASSEMBLY (COMMON) (Eq.)	EXPANSION BEARING ASSEMBLY (SIDE) (Eq.)	ANCHOR ROD		1" DIA. GRADE A325 BOLTS x 4½" (Eq.)	GRATING PANEL						WALKWAY GRATING 16⅜" W. x 7'-1" L. (Eq.)	WALKWAY GRATING 16⅜" W. x 4-6½" L. (Eq.)	⅝" DIA. ASTM A307 GRADE A HEX BOLT x 2½" (Eq.)	F-10 FASTENER (Eq.)	⅝" DIA. GRADE A325 BOLTS						1" DIA. GRADE A325 BOLTS			
					SAR-10E (Eq.)	SAR-10F (Eq.)		GP-1 (Eq.)	GP-2 (Eq.)	GP-3 (Eq.)	GP-4 (Eq.)	GP-5 (Eq.)	GP-6 (Eq.)	x 4¼" (Eq.)	x 4" (Eq.)	x 3¾" (Eq.)		x 3½" (Eq.)	x 3¼" (Eq.)	x 3" (Eq.)	x 2¾" (Eq.)	x 4" (Eq.)	x 3½" (Eq.)	x 3" (Eq.)			
					90'-0"	1	2	1	2	12	12	48	2	16	16	2		2	2	20	4	264	264	80	64	128	568

NOTE:
L = LONGITUDINAL DIMENSION, OUT TO OUT OF GIRDERS

LIFTING WEIGHT OF TPG SPAN						
SPAN LENGTH (LB.)	COMMON GIRDER (TON)		SIDE GIRDER (TON)		PHASE 1 ASSEMBLED SPAN (LB.) (TON)	
90'-0"	82,675	41.3	83,090	41.5	335,775	167.9

- NOTES:
- COMMON GIRDER WEIGHT DOES NOT INCLUDE CONNECTION ANGLE CA-1.
 - SIDE GIRDER WEIGHT DOES NOT INCLUDE CONNECTION ANGLE CA-1.
 - ASSEMBLED SPAN WEIGHT SHOWN IS FOR STEEL ONLY (INCLUDING GRATING, NOT INCLUDING BEARINGS).
 - ASSEMBLED SPAN WEIGHT INCLUDES ONE COMMON GIRDER, ONE SIDE GIRDER AND FLOOR SYSTEM FOR ONE BAY ONLY.
 - TOTAL WEIGHT OF STRUCTURAL STEEL FOR TPG SPAN = 626,000 LB.

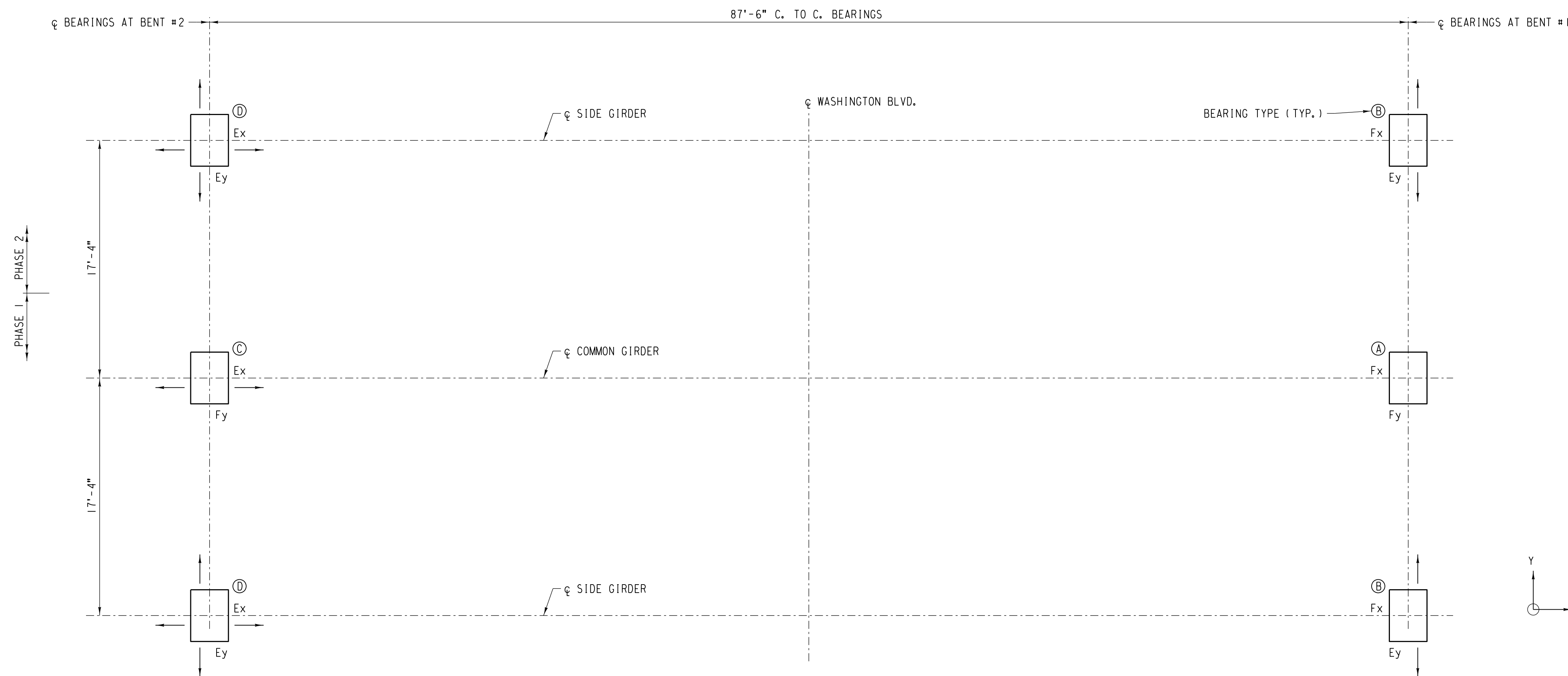
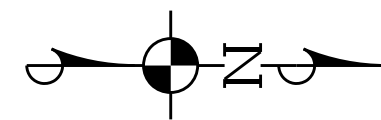
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	/	/	
NO.	DATE	REVISIONS	
COMPLETION STATUS:			
FINAL		05/28/2021	
STATUS		DATE	
			
APPROVED FOR UNION PACIFIC RAILROAD BY:			
MATTHEW BECKER		05/28/2021	
CONSULTANT ENGINEER		DATE	
PROJECT ID:	WORK ORDER:	C/E NUMBER:	
	31876	122525	

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N LONGITUDE: 87.69144°W	
	DSN/CHK BY:	UNION PACIFIC RAILROAD Office of Director Structures Design	
	DRAWN/CHK BY:		
	UPRR ENGINEER:	LOCATION & DESCRIPTION:	
	SHT NO:	BRIDGE 0.93 ROCKWELL SUBDIVISION	
E37 of E43	1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)		
SHEET TITLE:		TPG STRESS, MATERIAL LIST AND LIFTING WEIGHT TABLES	

FILE NAME: C:\user\$krmin\ez\$desk\p\0000931_b1.dgn

TO CANAL ST. (CHICAGO)
(TIMETABLE SOUTH)

TO KEDZIE (CHICAGO)
(TIMETABLE NORTH)

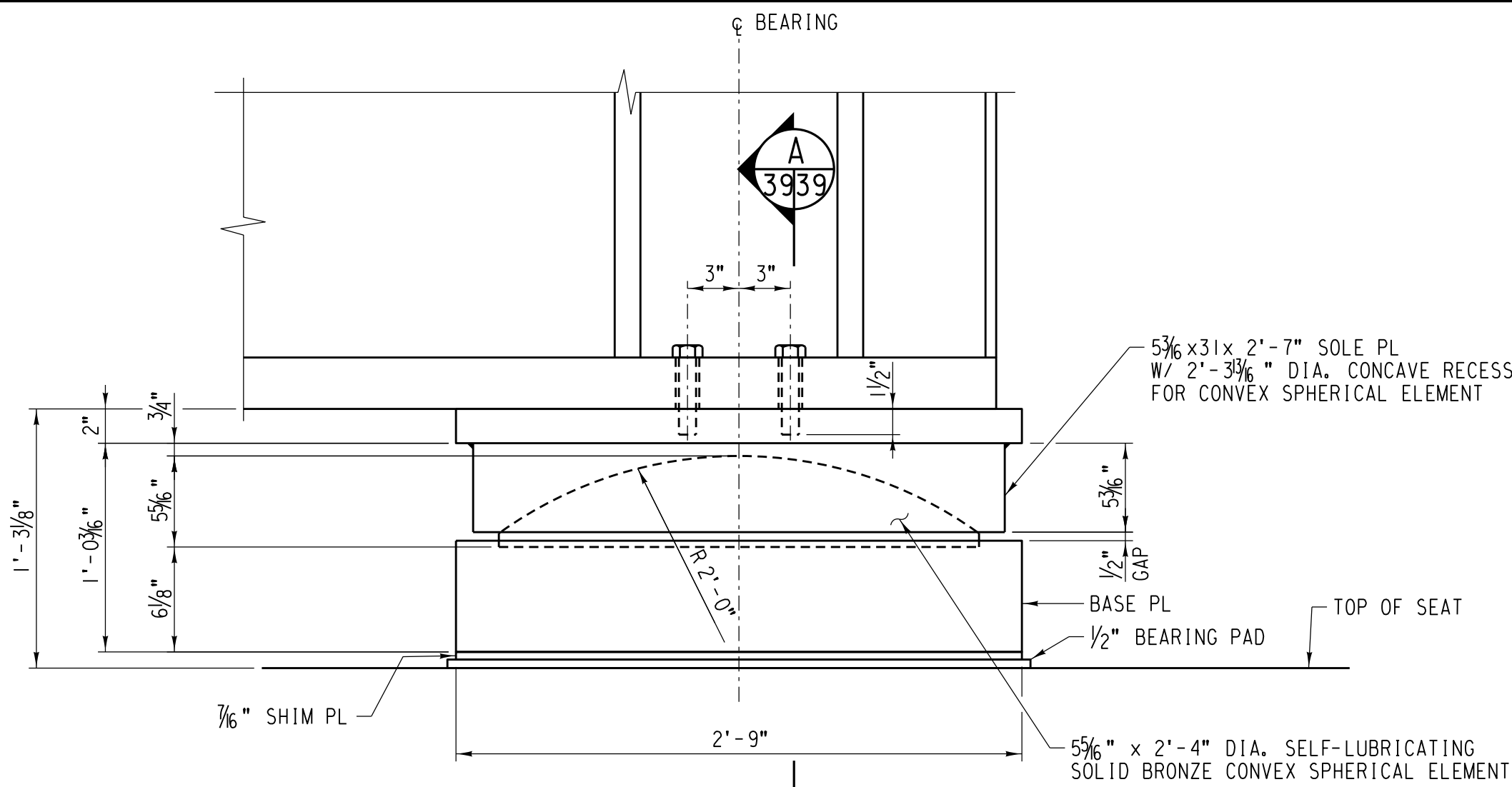


BEARING KEY PLAN
SCALE: $\frac{3}{16}'' = 1'-0''$

NOTE:
WORK THIS SHEET WITH SHEET NOS. 39-42.

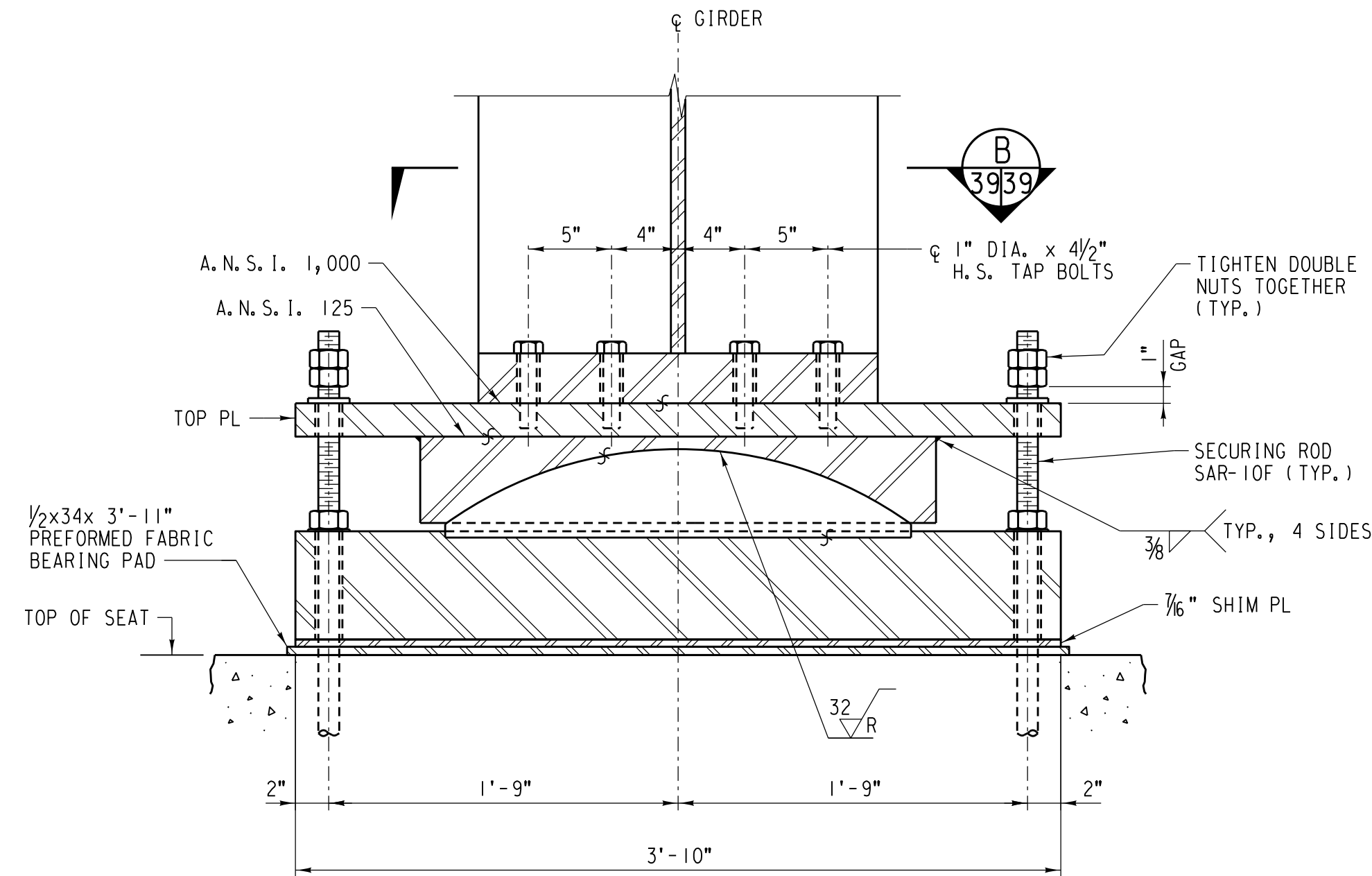
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	/	/	
NO.	DATE	REVISIONS	
COMPLETION STATUS:			
FINAL		<u>05/28/2021</u>	
STATUS		DATE	
 benesch			
APPROVED FOR UNION PACIFIC RAILROAD BY:			
MATTHEW BECKER		<u>05/28/2021</u>	
CONSULTANT ENGINEER		DATE	
PROJECT ID:	WORK ORDER:	C/E NUMBER:	
	31876	122525	

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design	
	DRAWN/CHK BY: RR /MFB		
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
	SHT NO.: E38 of E43		
	SHEET TITLE: TPG BEARING LAYOUT		



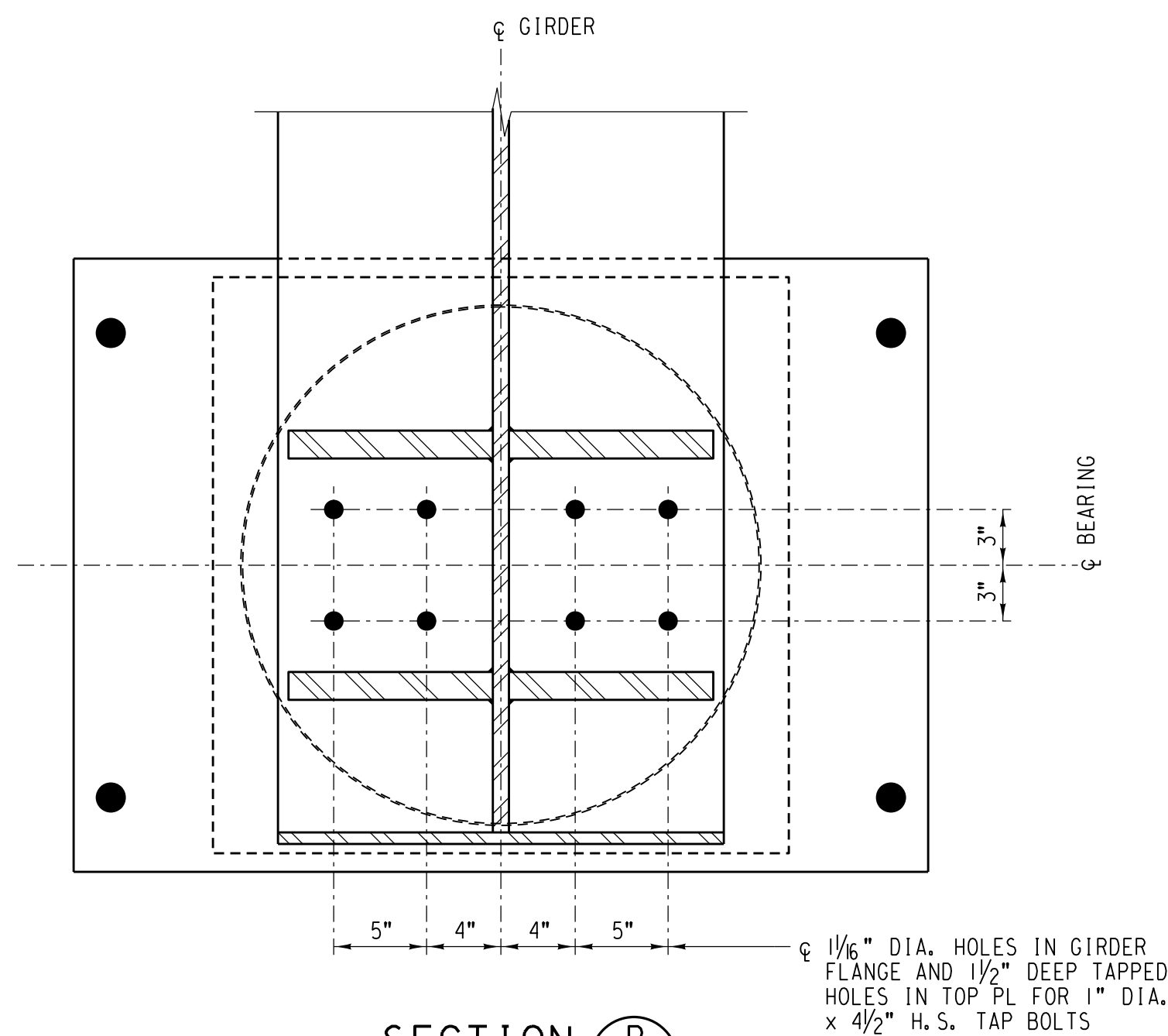
BEARING TYPE A ELEVATION

SCALE: 1/2"=1'-0"
(SECURING RODS AND FLOORBEAMS NOT SHOWN FOR CLARITY)



SECTION A

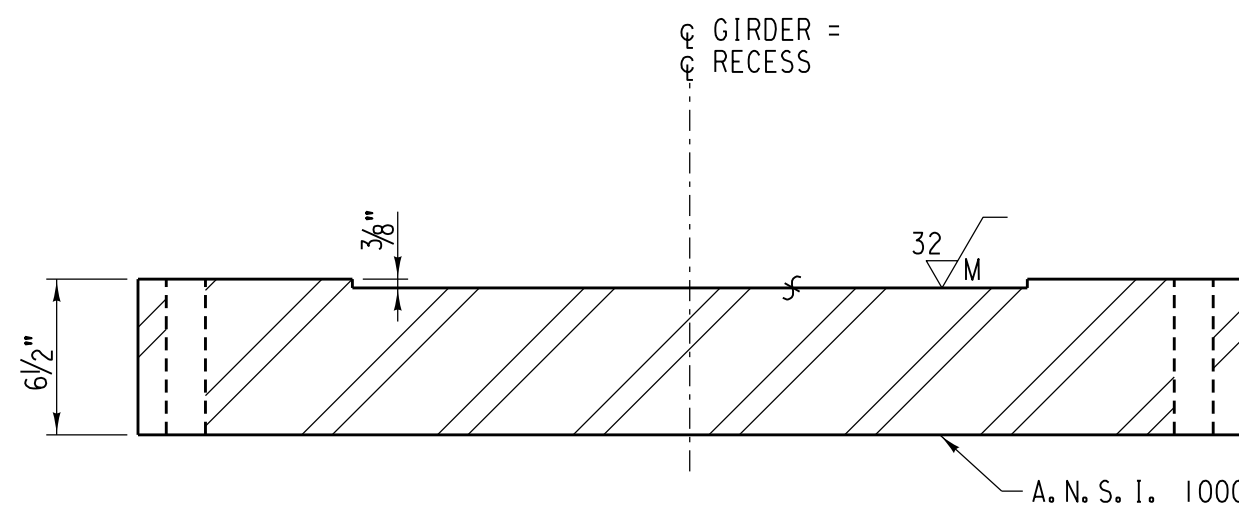
SCALE: 1/2"=1'-0"



SECTION B

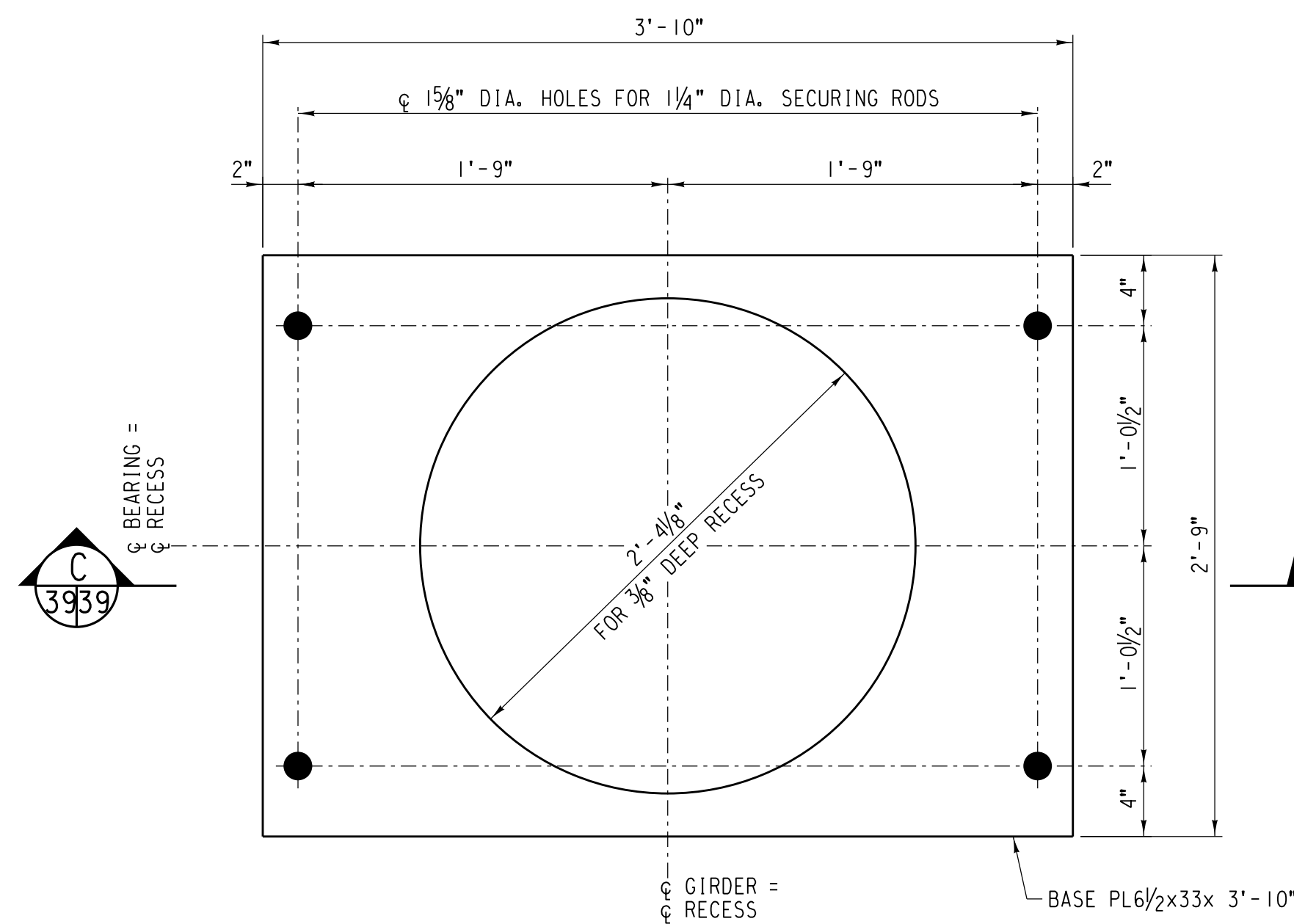
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(END FLOORBEAM AND INTERIOR FLOORBEAM NOT SHOWN FOR CLARITY)



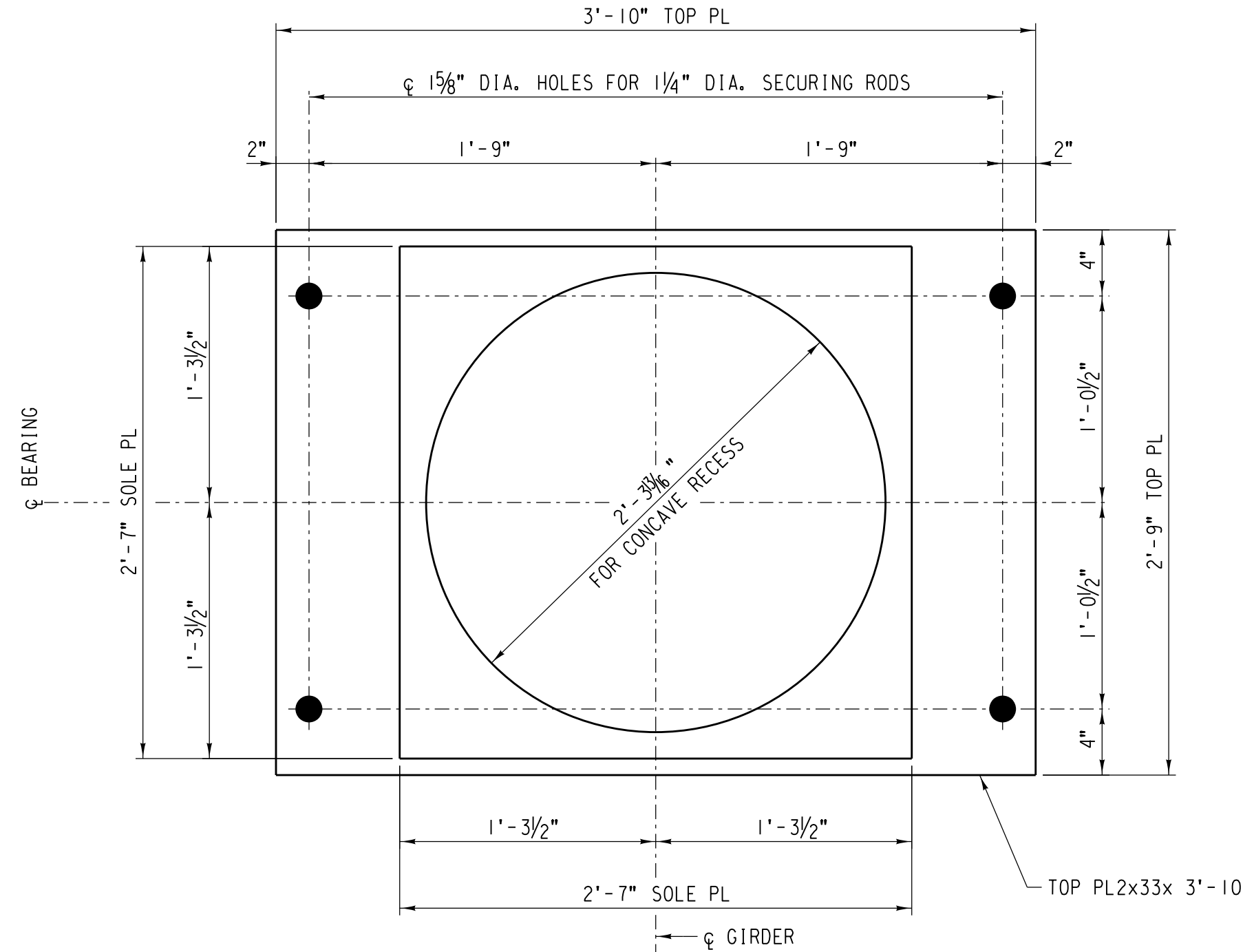
SECTION C

SCALE: 1/2"=1'-0"



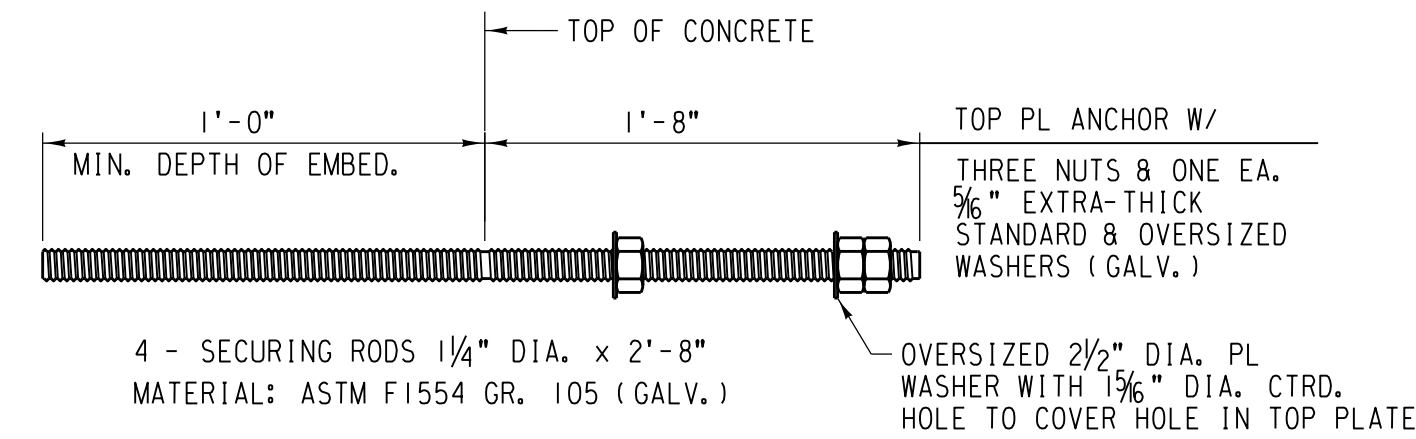
BASE PLATE DETAIL

SCALE: 1/2"=1'-0"



TOP & SOLE PLATE DETAILS

SCALE: 1/2"=1'-0"
(UNDERSIDE SHOWN)



SAR-10F SECURING ANCHOR ROD

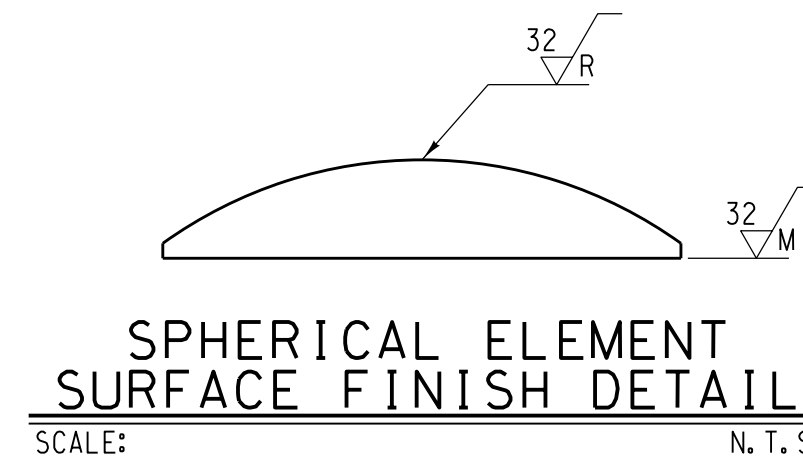
SCALE: NONE

NOTES:

- SECURING RODS SHALL BE MADE FROM HIGH-STRENGTH MATERIAL MEETING THE REQUIREMENTS OF ASTM F1554, GRADE 105. THE RODS SHALL BE SUPPLIED WITH HEAVY HEX NUTS MEETING THE REQUIREMENTS OF ASTM A194 GRADE 2H OR ASTM A563 GRADE DH AND HARDENED WASHERS MEETING THE REQUIREMENTS OF ASTM F436. SECURING RODS AND HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM F2329.
- THE CONTRACTOR SHALL INSTALL THE SECURING RODS INTO THE CONCRETE ABUTMENTS USING NON-SHRINK CEMENTITIOUS GROUT.

NOTES:

- ALL SURFACES MARKED "S" SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.
- BEARING ASSEMBLIES SHALL BE STRESS RELIEVED AFTER WELDING.
- BEARINGS SHALL BE ASSEMBLED COMPLETE IN THE SHOP AND CHECKED FOR FIT AND BEARING OF ALL CONTACT SURFACES, THEN MATCH MARKED FOR INSTALLATION IN THE FIELD.
- STRUCTURAL STEEL PLATE FOR THE BEARINGS SHALL CONFORM TO ASTM A709 GR. 36 OR ASTM A36 SPECIFICATIONS.
- BRONZE ELEMENTS SHALL CONFORM TO ASTM B22, COPPER ALLOY C91100.
- TAP BOLTS SHALL BE 1" DIAMETER HIGH STRENGTH BOLTS CONFORMING WITH ASTM F3125 GRADE A325, TYPE 3.
- END FLOORBEAMS NEAR BEARINGS SHALL HAVE THEIR BOTTOM FLANGES COPED TO AVOID CONFLICT WITH SECURING ANCHOR RODS.



SPHERICAL ELEMENT SURFACE FINISH DETAIL
SCALE: N. T. S.

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION



DESIGNED BY:
FNF/MFB
DRAWN/CHECKED BY:
RR/MFB
UPRR ENGINEER:
DEH/ADS
SHEET NO.:
E39 of E43

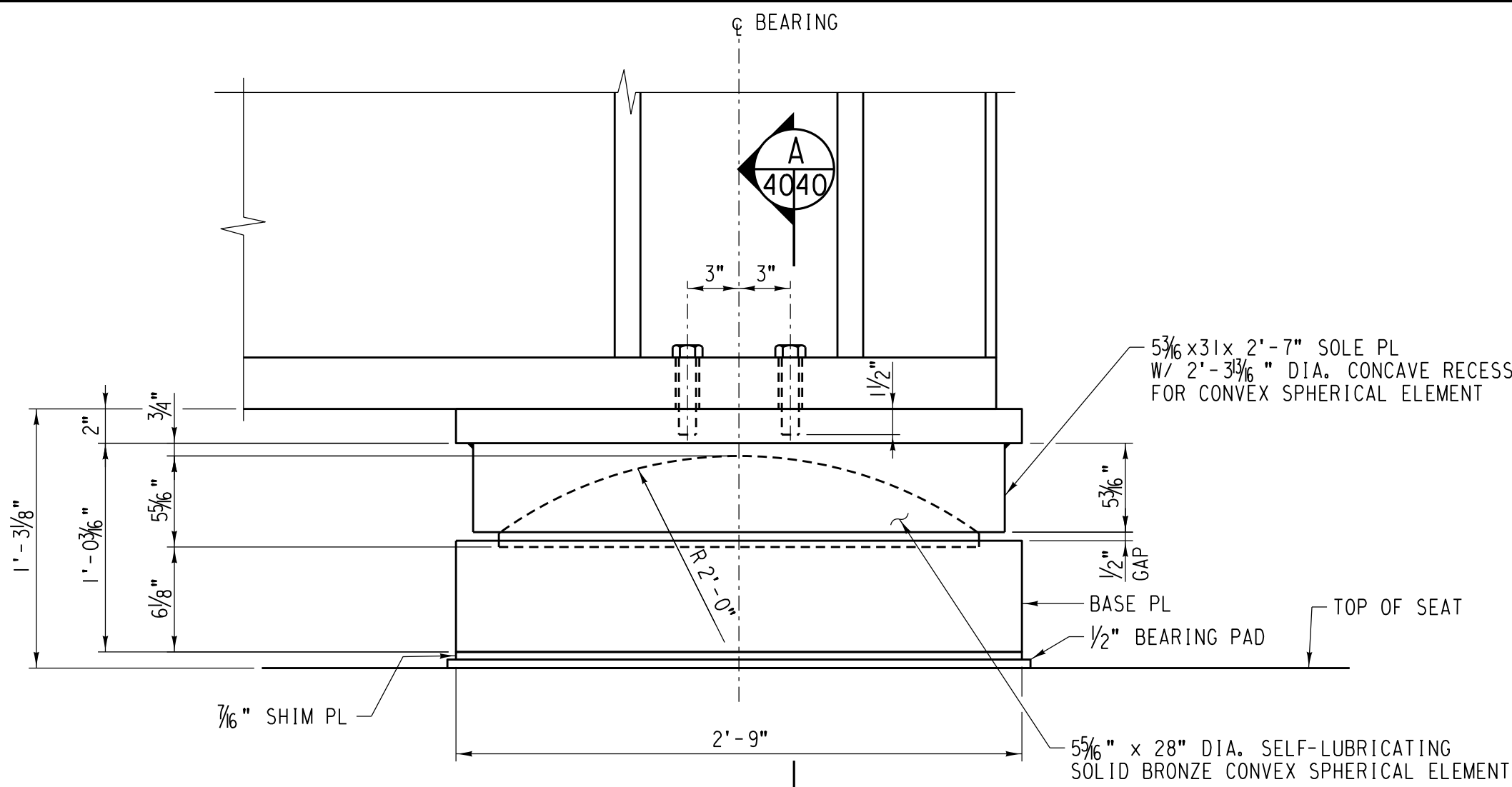
UNION PACIFIC RAILROAD
Office of Director Structures Design

LOCATION & DESCRIPTION: **BRIDGE 0.93 ROCKWELL SUBDIVISION**

1 SPAN TPG x 90'
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)

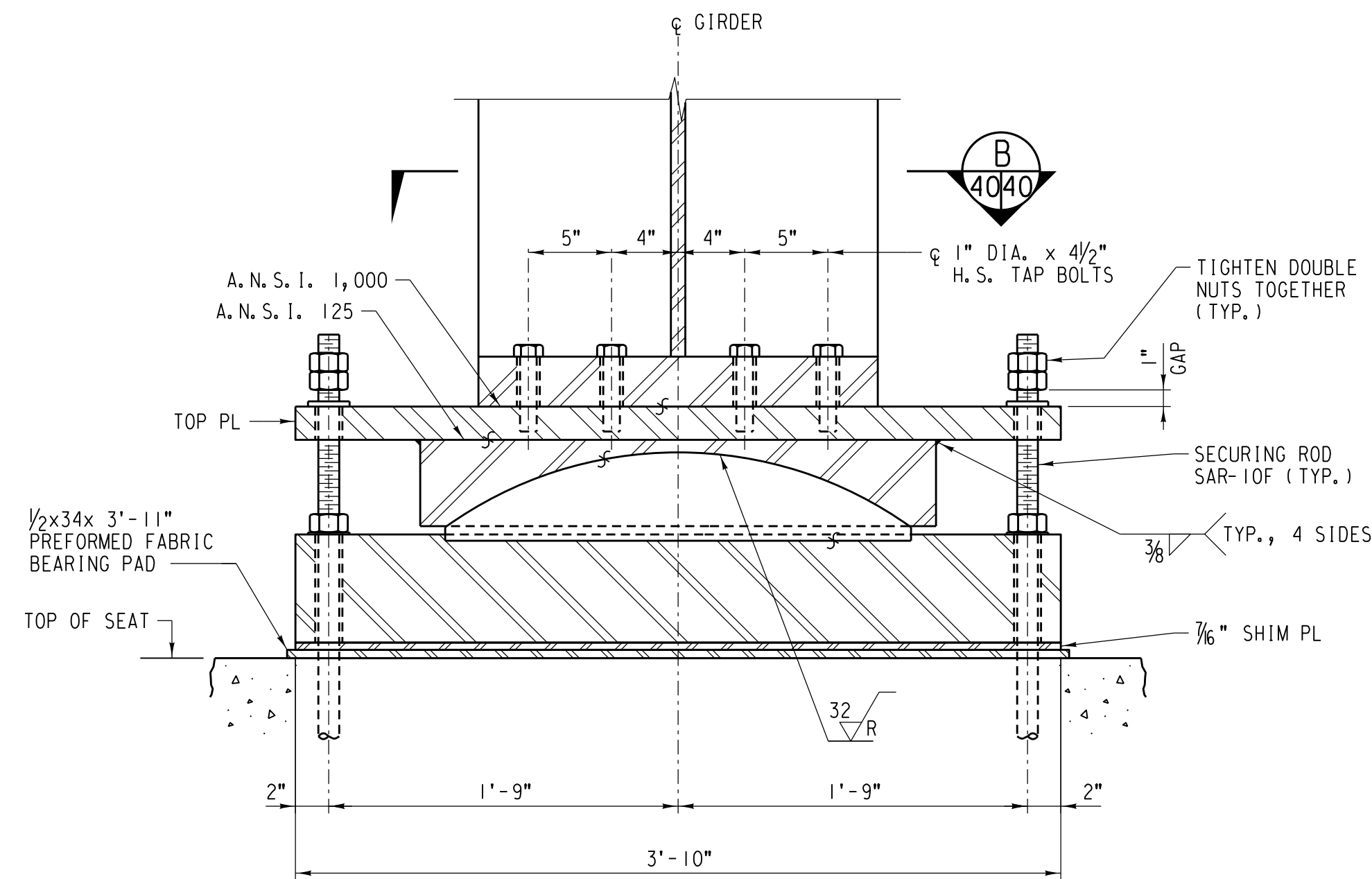
SHEET TITLE: **TPG BEARING DETAILS - TYPE A**

LATITUDE: 41.88670°N LONGITUDE: 87.69144°W



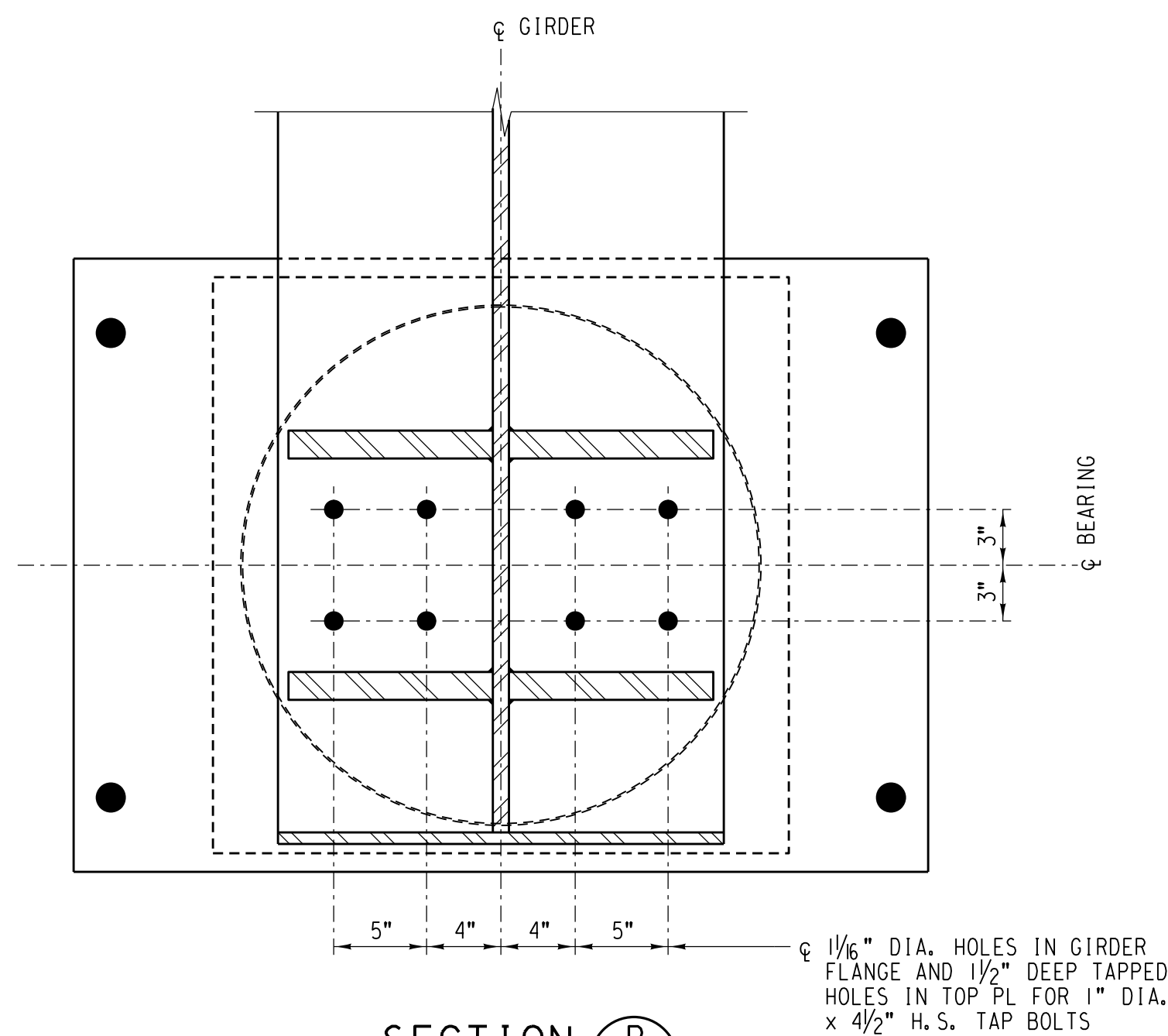
BEARING TYPE B ELEVATION

SCALE: 1/2"=1'-0" (SECURING RODS AND FLOORBEAMS NOT SHOWN FOR CLARITY)



SECTION A

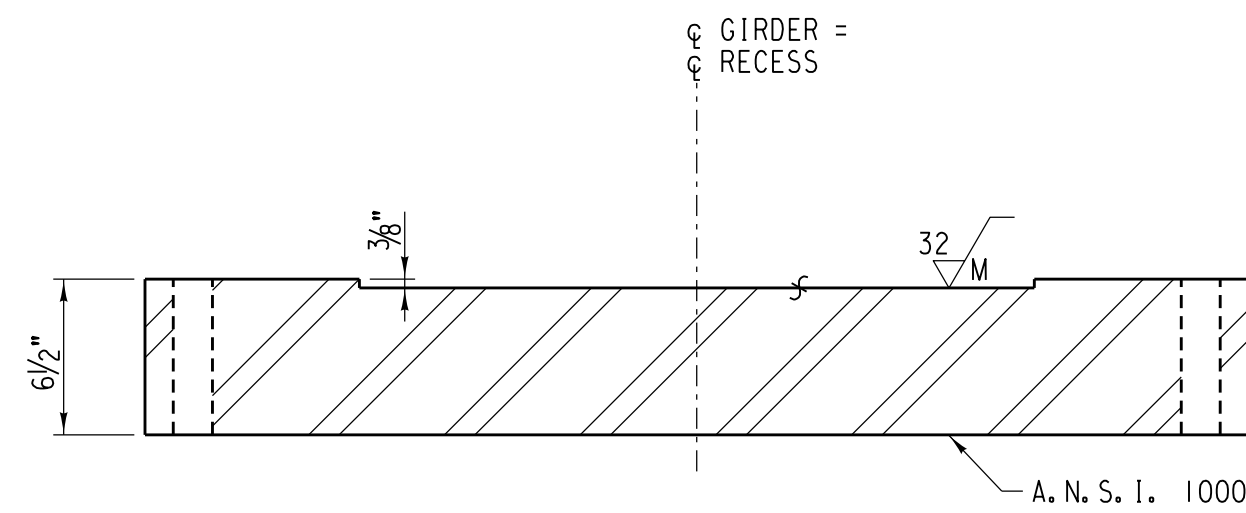
SCALE: 1/2"=1'-0"



SECTION B

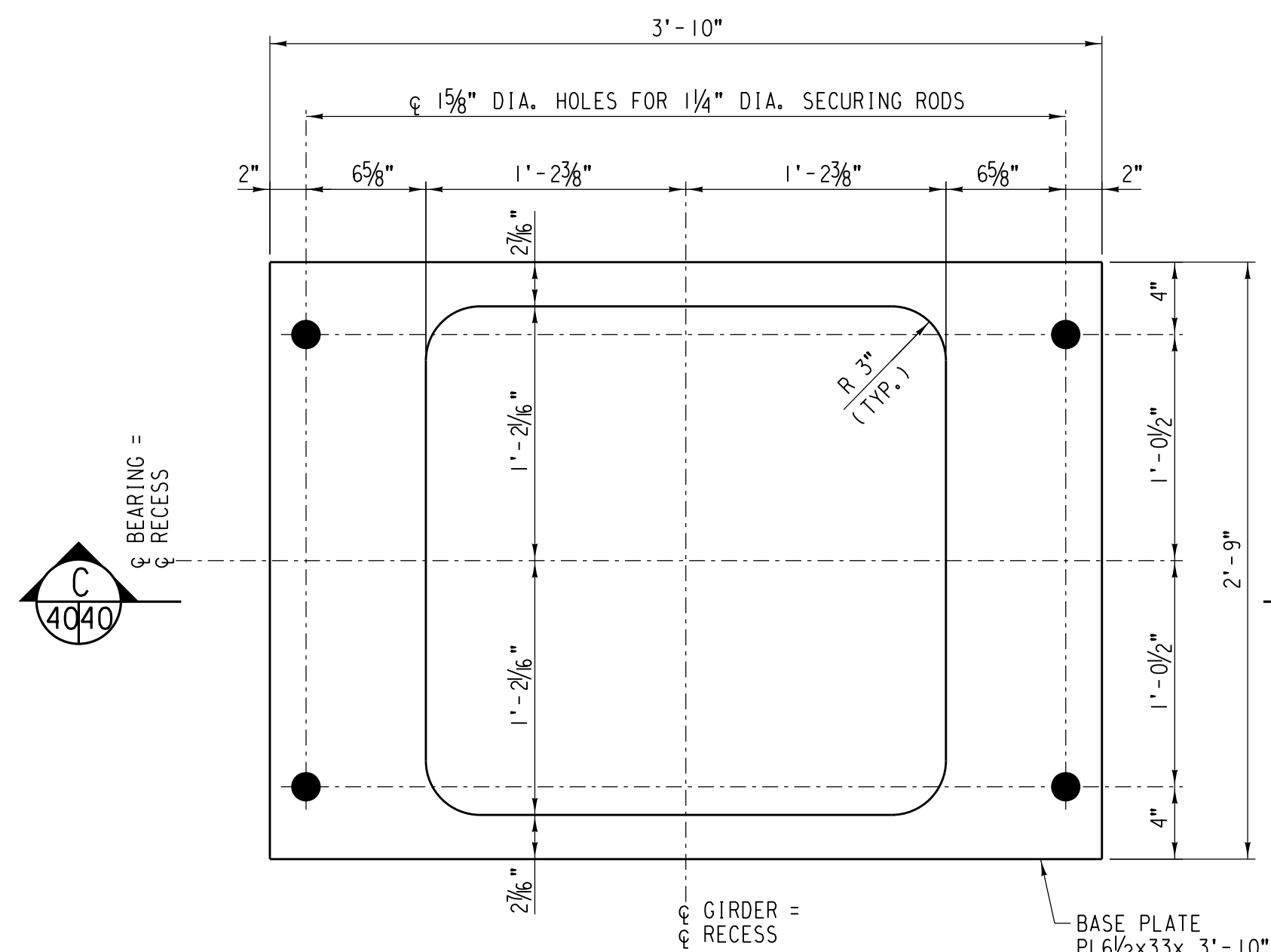
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(END FLOORBEAM AND INTERIOR FLOORBEAM NOT SHOWN FOR CLARITY)



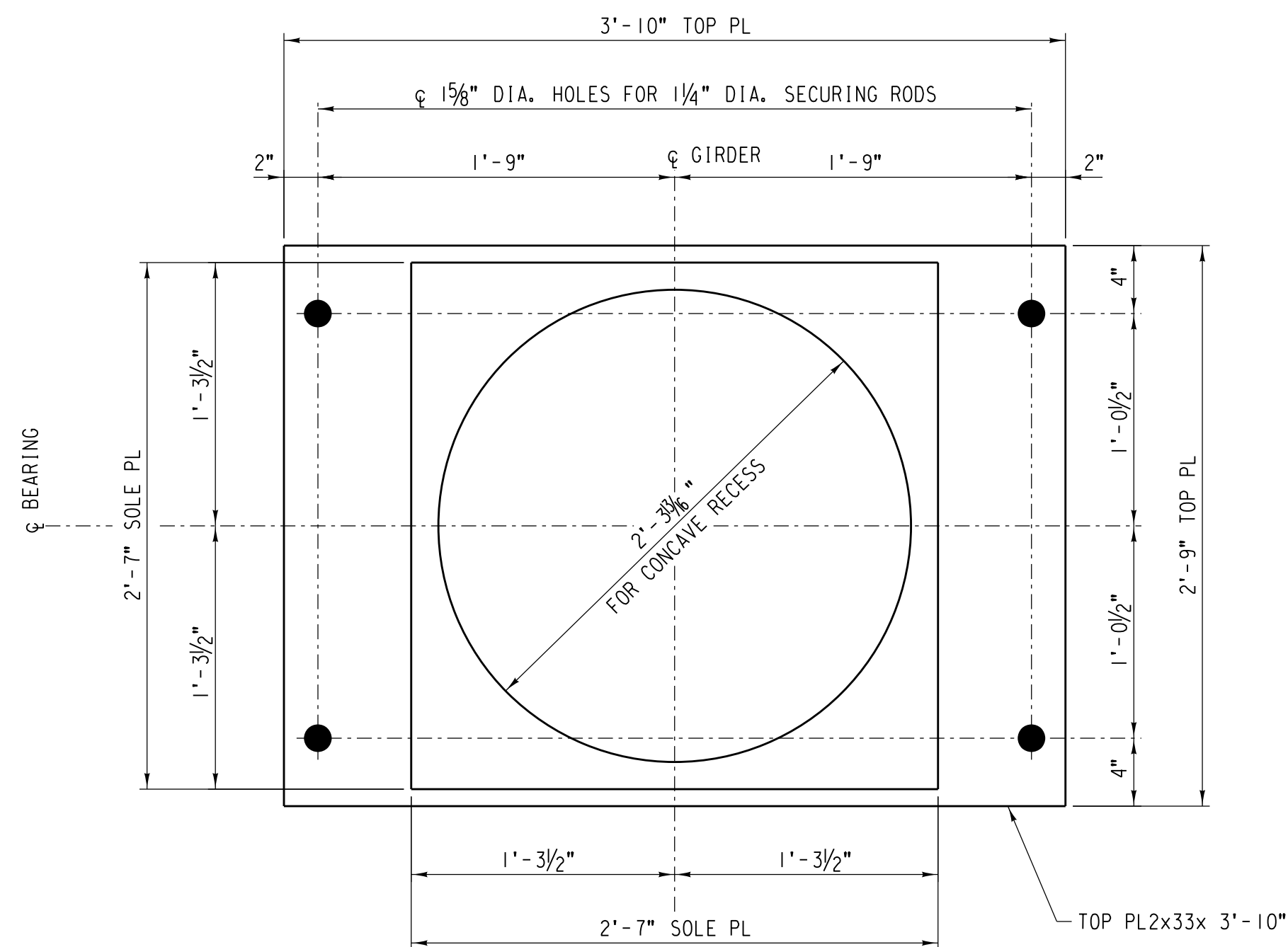
SECTION C

SCALE: 1/2"=1'-0"



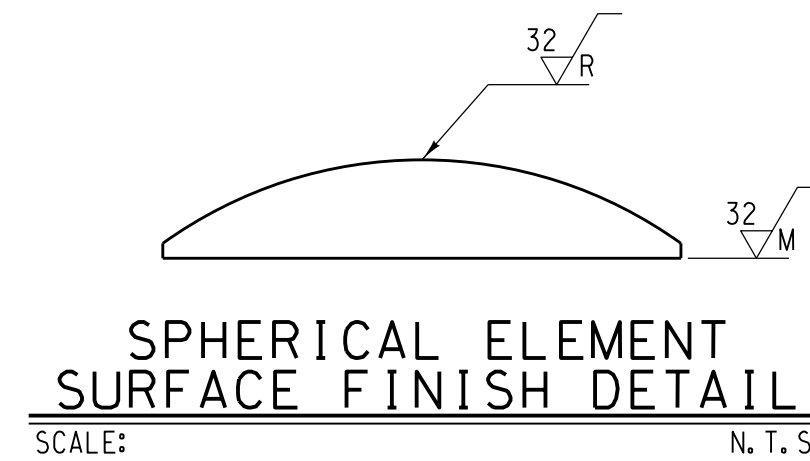
BASE PLATE DETAIL

SCALE: 1/2"=1'-0"



TOP & SOLE PLATE DETAILS

SCALE: 1/2"=1'-0" (UNDERSIDE SHOWN)



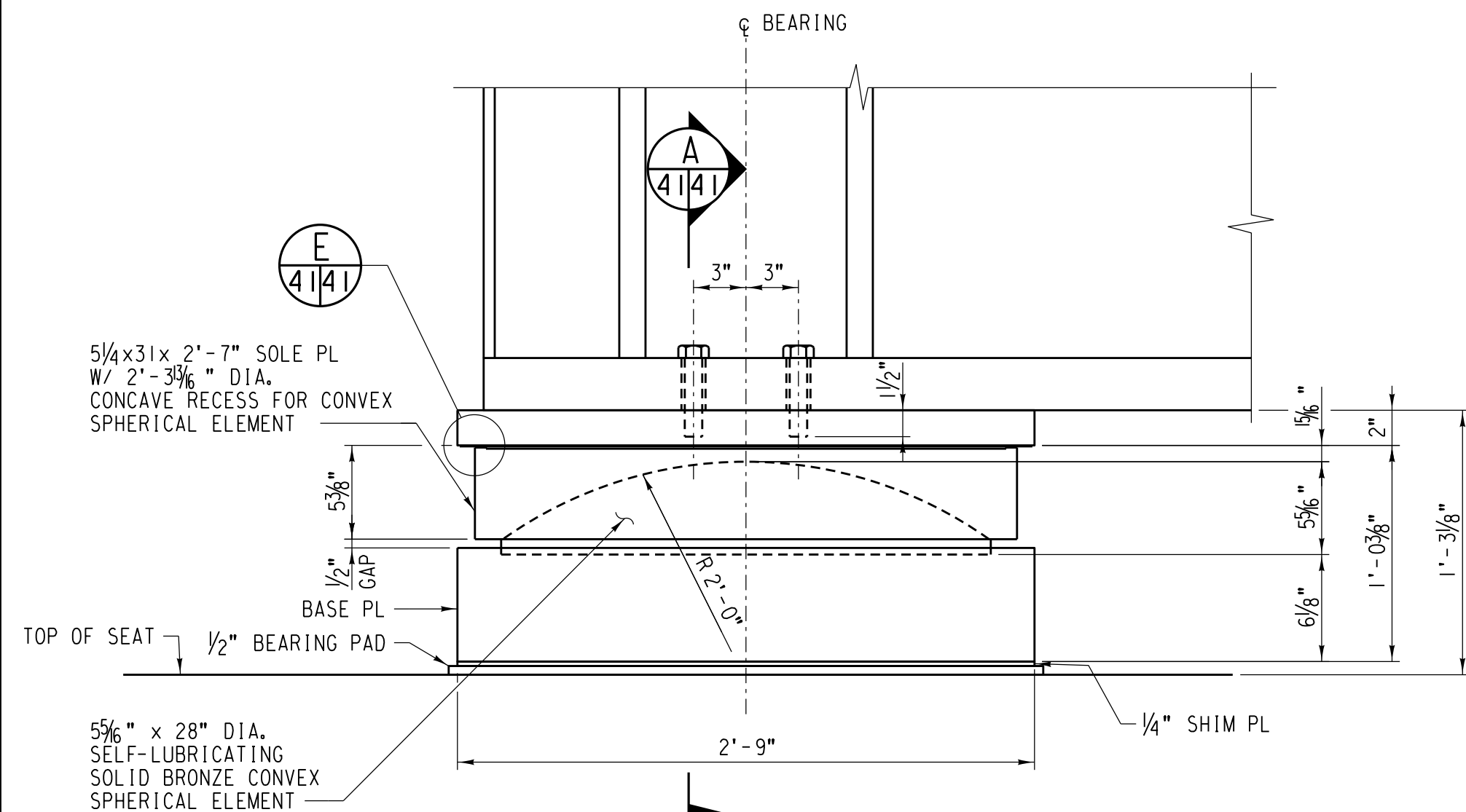
SCALE: N. T. S.

- NOTES:
1. ALL SURFACES MARKED "S" SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.
 2. BEARING ASSEMBLIES SHALL BE STRESS RELIEVED AFTER WELDING.
 3. BEARINGS SHALL BE ASSEMBLED COMPLETE IN THE SHOP AND CHECKED FOR FIT AND BEARING OF ALL CONTACT SURFACES, THEN MATCH MARKED FOR INSTALLATION IN THE FIELD.
 4. STRUCTURAL STEEL PLATE FOR THE BEARINGS SHALL CONFORM TO ASTM A709 GR. 36 OR ASTM A36 SPECIFICATIONS.
 5. BRONZE ELEMENTS SHALL CONFORM TO ASTM B22, COPPER ALLOY C91100.
 6. TAP BOLTS SHALL BE 1" DIAMETER HIGH STRENGTH BOLTS CONFORMING WITH ASTM F3125 GRADE A325, TYPE 3.
 7. END FLOORBEAMS NEAR BEARINGS SHALL HAVE THEIR BOTTOM FLANGES COPED TO AVOID CONFLICT WITH SECURING ANCHOR RODS.

NO.	DATE	REVISIONS
1	05/28/2021	DATE
COMPLETION STATUS:		
FINAL		
STATUS		
benesch		
APPROVED FOR UNION PACIFIC RAILROAD BY:		
MATTHEW BECKER		
CONSULTANT ENGINEER		
DATE		
PROJECT ID:		
WORK ORDER:		
31876		
C/E NUMBER:		
122525		

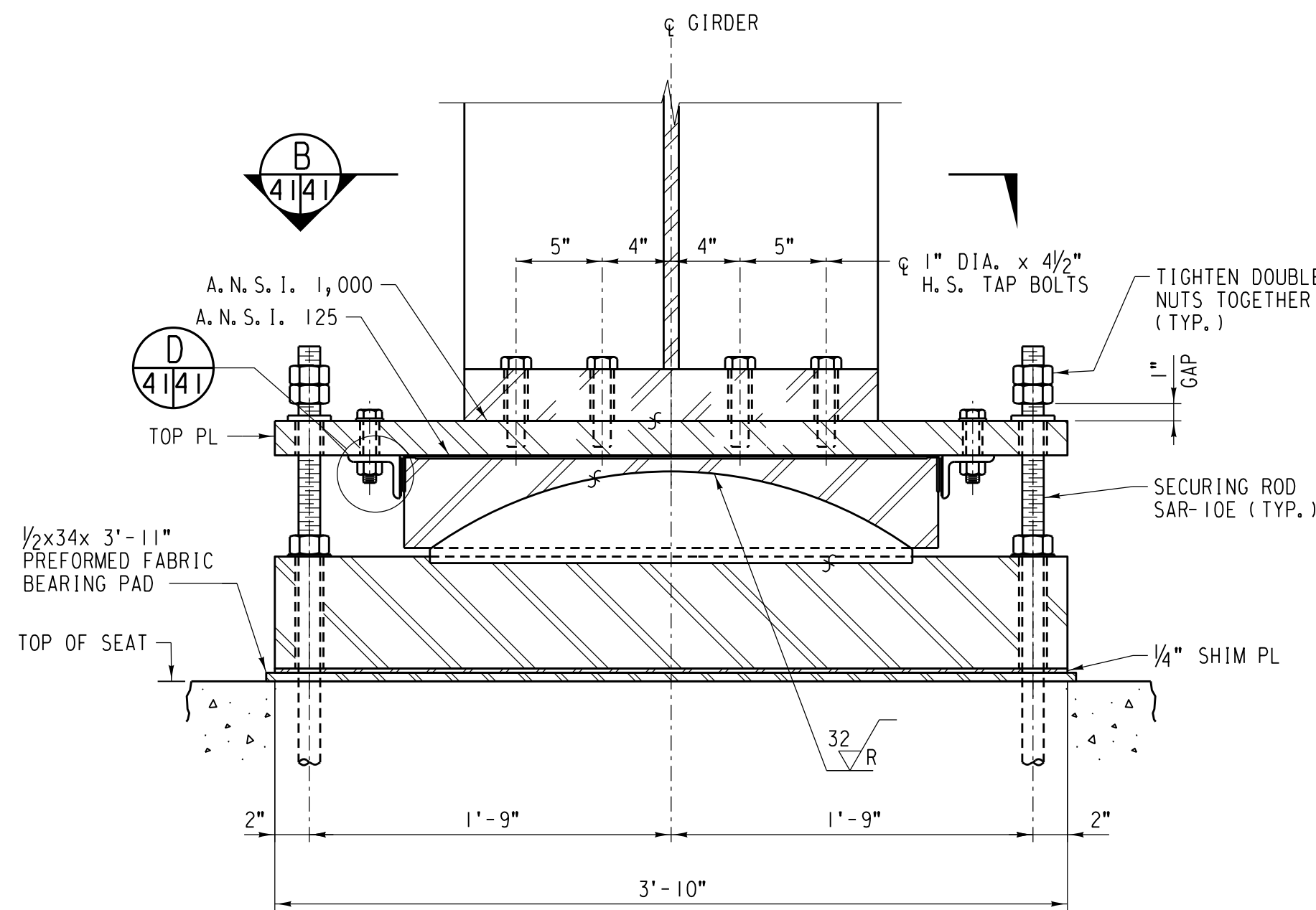
FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
UNION PACIFIC	DSNCHK BY:	UNION PACIFIC RAILROAD	
	FNH/MFB	Office of Director Structures Design	
	DRAWN/CHK BY:	LOCATION & DESCRIPTION:	
	RR /MFB	BRIDGE 0.93 ROCKWELL SUBDIVISION	
UPRR ENGINEER:		1 SPAN TPG x 90'	
DEH/ADS		REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)	
SHT NO.:		SHEET TITLE:	
E40 of E43		TPG BEARING DETAILS - TYPE B	

FILE NAME: C:\user\gfr\cmr\ez\Bask\top\0200931_b11.dgn



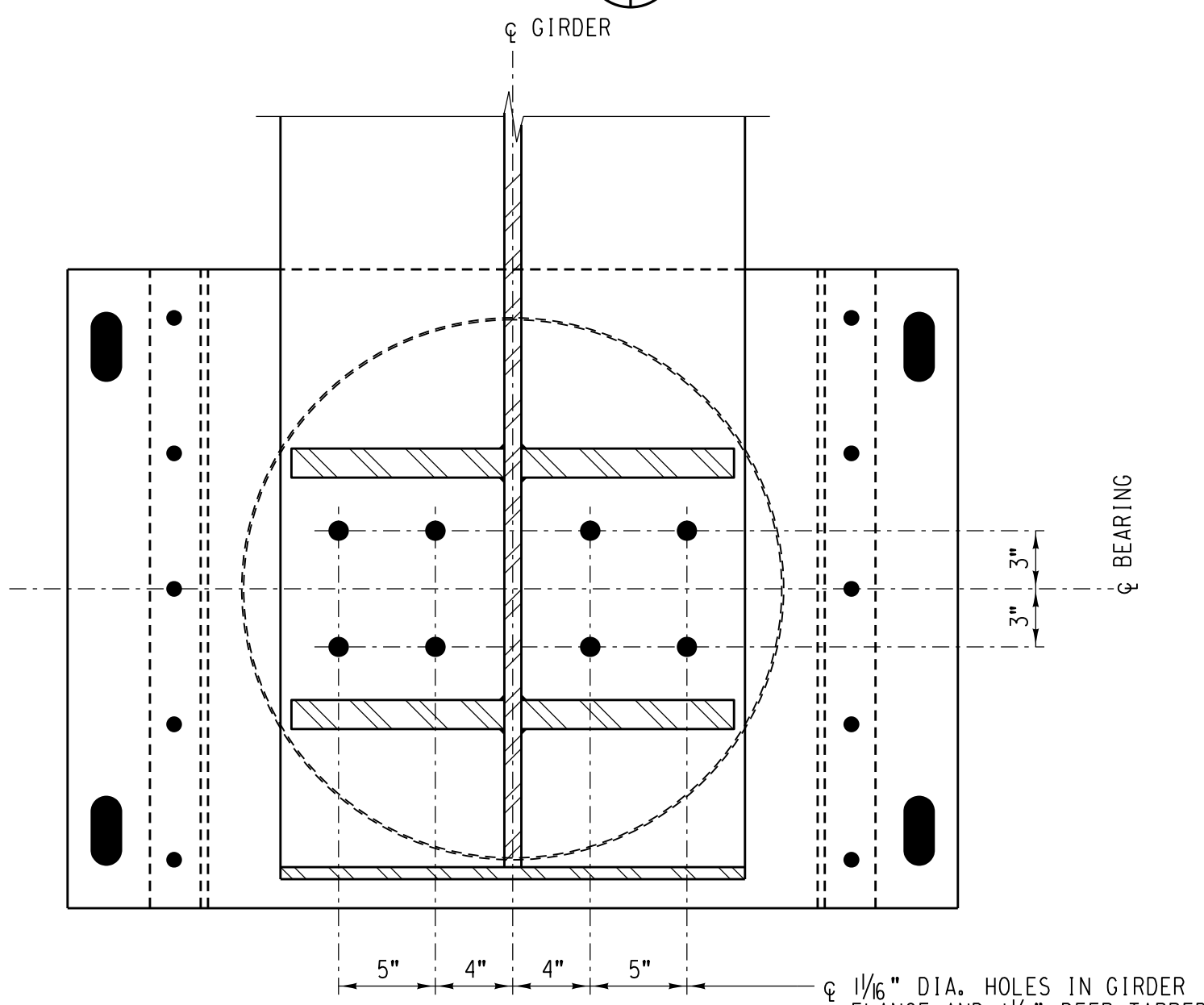
BEARING TYPE C ELEVATION

SCALE: 1/2"=1'-0" (SECURING RODS AND FLOORBEAMS NOT SHOWN FOR CLARITY)



SECTION A

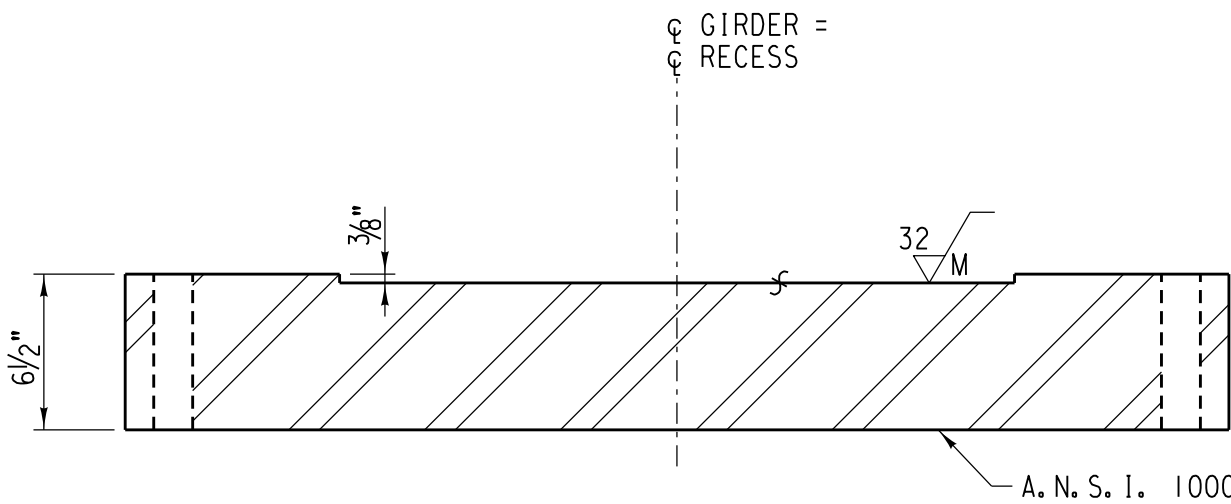
SCALE: 1/2"=1'-0"



SECTION B

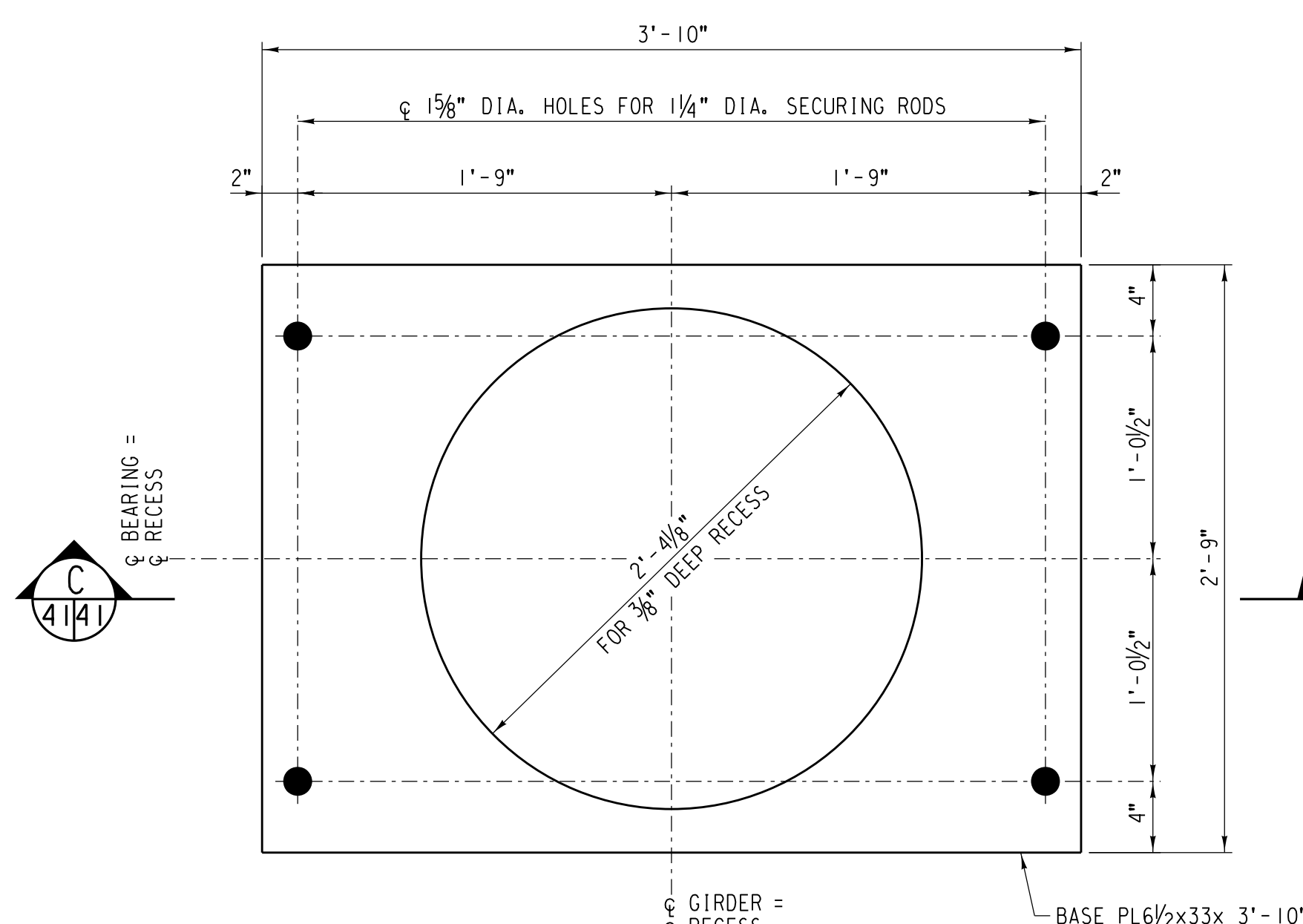
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(END FLOORBEAM AND INTERIOR FLOORBEAM NOT SHOWN FOR CLARITY)



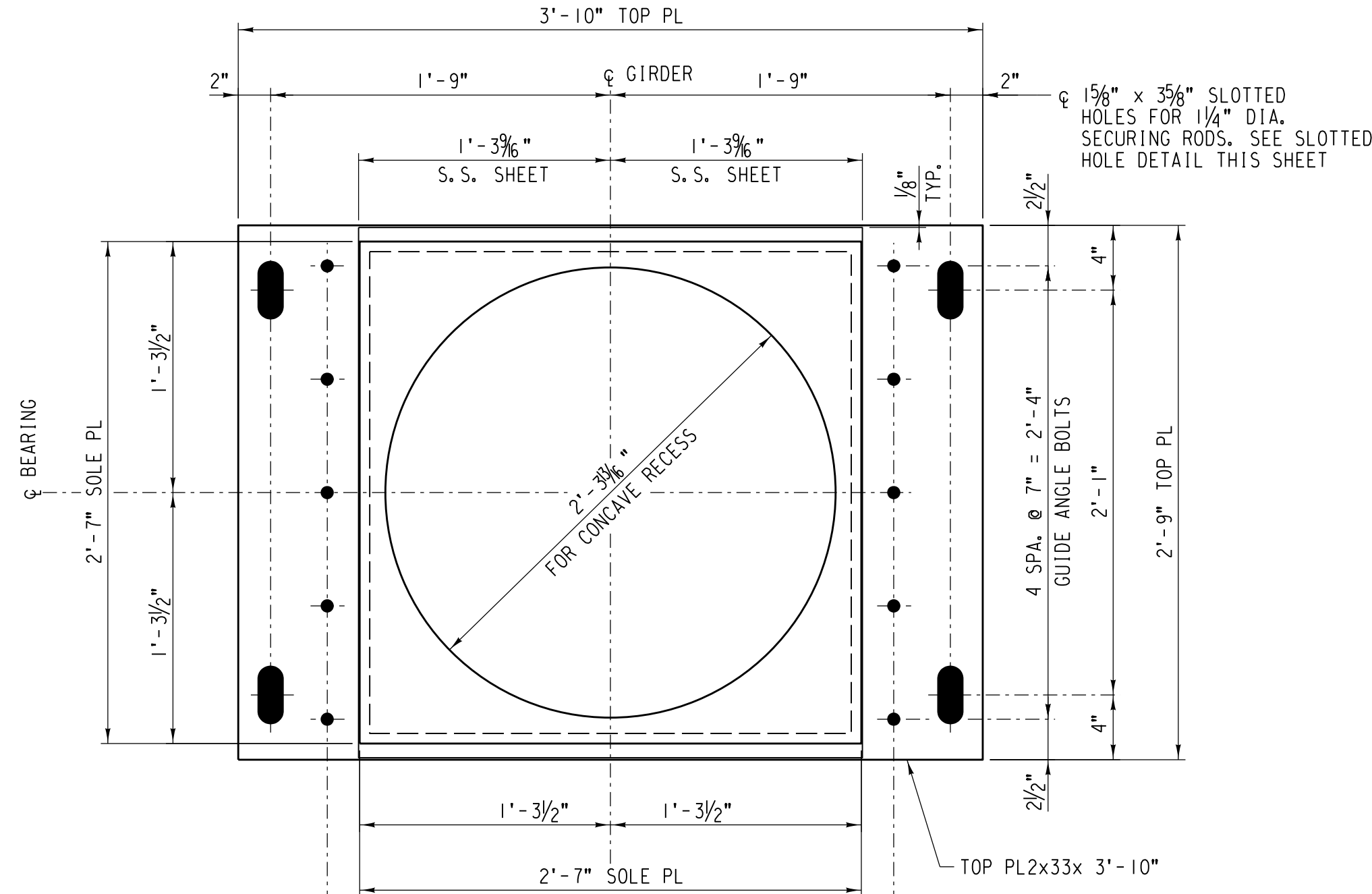
SECTION C

SCALE: 1/2"=1'-0"



BASE PLATE DETAIL

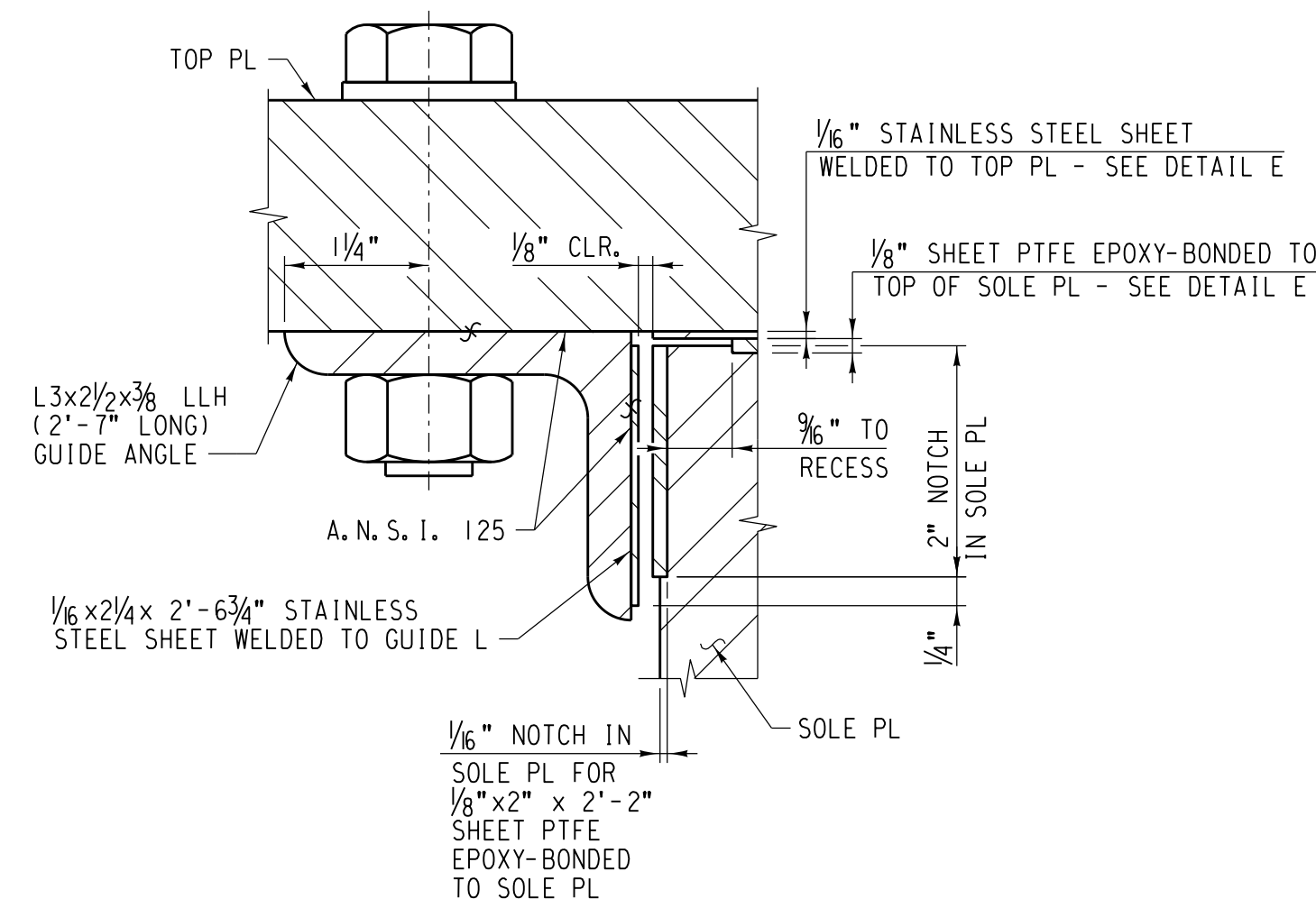
SCALE: 1/2"=1'-0"



TOP & SOLE PLATE DETAILS

SCALE: 1/2"=1'-0"

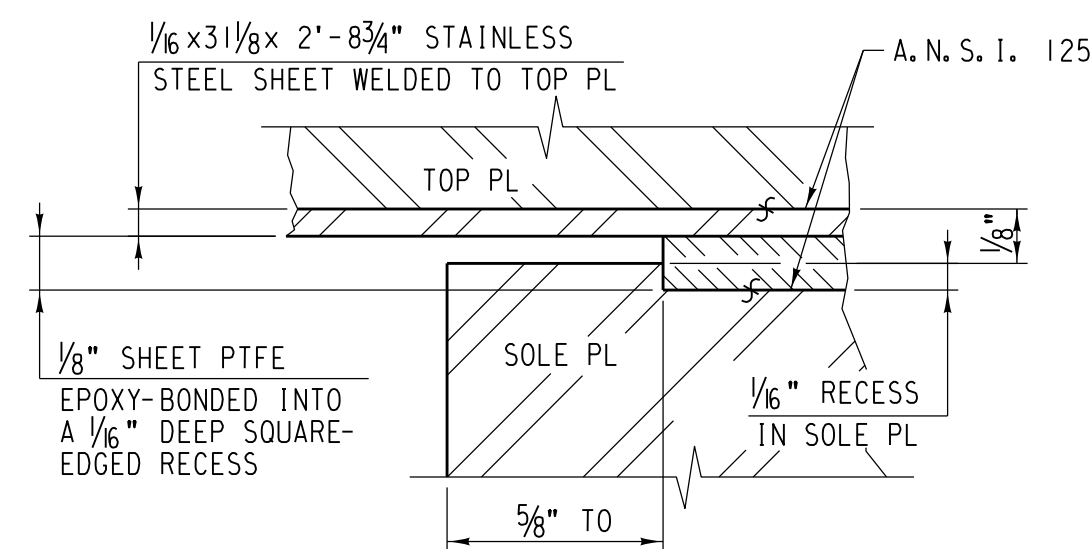
(UNDERSIDE SHOWN)



DETAIL D

SCALE: NONE

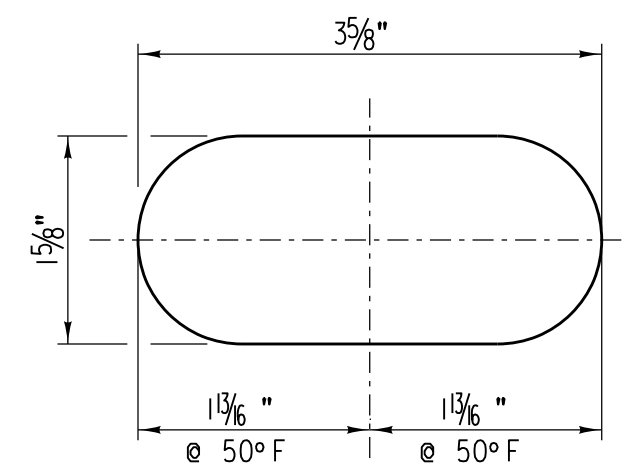
(SHOWN IN SECTION NEAR C OF BEARING)



DETAIL E

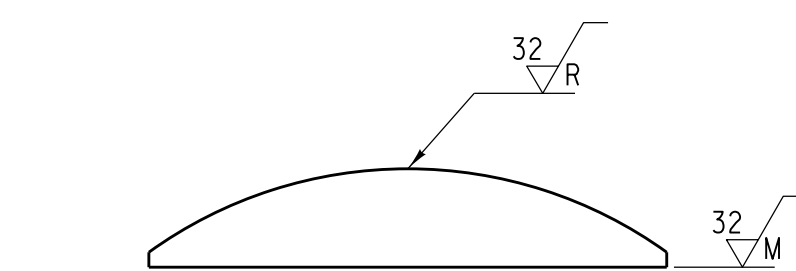
SCALE: NONE

(SHOWN IN SECTION NEAR C OF GIRDER)



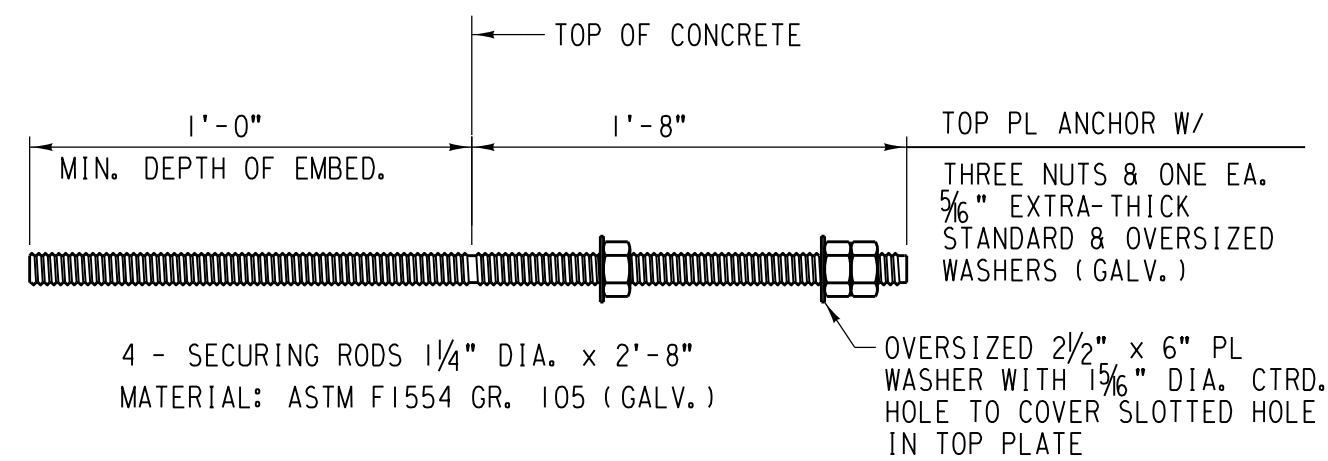
SLOTTED HOLE DETAIL

SCALE: NONE



SPHERICAL ELEMENT SURFACE FINISH DETAIL

SCALE: N. T. S.



SAR-10E SECURING ANCHOR ROD

SCALE: NONE

NOTES:

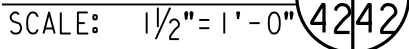
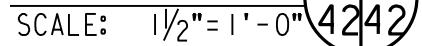
- SECURING RODS SHALL BE MADE FROM HIGH-STRENGTH MATERIAL MEETING THE REQUIREMENTS OF ASTM F1554, GRADE 105. THE RODS SHALL BE SUPPLIED WITH HEAVY HEX NUTS MEETING THE REQUIREMENTS OF ASTM A194 GRADE 2H OR ASTM A563 GRADE DH AND HARDENED WASHERS MEETING THE REQUIREMENTS OF ASTM F436. SECURING RODS AND HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM F2329.
- THE CONTRACTOR SHALL INSTALL THE SECURING RODS INTO THE CONCRETE ABUTMENTS USING NON-SHRINK CEMENTITIOUS GROUT.

NOTES:

- ALL SURFACES MARKED "S" SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.
- BEARINGS SHALL BE ASSEMBLED COMPLETE IN THE SHOP AND CHECKED FOR FIT AND BEARING OF ALL CONTACT SURFACES, THEN MATCH MARKED FOR INSTALLATION IN THE FIELD.
- STAINLESS STEEL SHEETS SHALL CONFORM TO ASTM A167 OR ASTM A240 TYPE 304.
- TYPE 309L ELECTRODES SHALL BE USED TO WELD THE STAINLESS STEEL SHEETS TO THE TOP PL & GUIDE LS.
- STRUCTURAL STEEL PLATES AND LS FOR THE BEARINGS SHALL CONFORM TO ASTM A709 GR. 36 OR ASTM A36 SPECIFICATIONS.
- BRONZE ELEMENTS SHALL CONFORM TO ASTM B22, COPPER ALLOY C9100.
- TAP BOLTS SHALL BE 1" DIAMETER HIGH STRENGTH BOLTS CONFORMING WITH ASTM F3125 GRADE A325, TYPE 3.
- BOLTS FOR GUIDE ANGLES SHALL BE 1" DIAMETER HIGH STRENGTH BOLTS CONFORMING WITH ASTM F3125 GRADE A325, TYPE 1.
- END FLOORBEAMS NEAR BEARINGS SHALL HAVE THEIR BOTTOM FLANGES COPED TO AVOID CONFLICT WITH SECURING ANCHOR RODS.
- POLYTETRAFLUOROETHYLENE (PTFE) SHEET SHALL BE MANUFACTURED FROM PURE VIRGIN (NOT REPROCESSED) PTFE RESIN. PTFE SHEET SHALL MEET THE APPLICABLE MATERIAL REQUIREMENTS OF AREMA CHAPTER 15, SECTION 5.11. FINISHED PTFE SHEET SHALL BE RESISTANT TO ALL ACIDS, ALKALIS, AND PETROLEUM PRODUCTS, BE NON-FLAMMABLE, AND NON-ABSORBING OF WATER.

NO.	DATE	REVISIONS
1	05/28/2021	DATE
2	05/28/2021	DATE
3	05/28/2021	DATE
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6	05/28/2021	DATE
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100	05/28/2021	DATE

FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N	LONGITUDE: 87.69144°W
	DSNCHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design	
	DRAWN/CHK BY: RR/MFB		
	UPRR ENGINEER: DEH/ADS		
	SHT NO: E41 of E43		
BRIDGE 0.93 ROCKWELL SUBDIVISION		1 SPAN TPG x 90'	
REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)		TPG BEARING DETAILS - TYPE C	



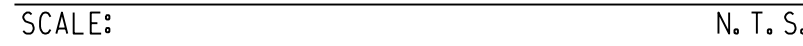
SCALE: $1\frac{1}{2}" = 1' - 0"$ 4242



SCALE: NONE 4242

SCALE: NONE 4242

SCALE: _____ NOM _____



FORMERLY BRIDGE 0.82 ROCKWELL SUBDIVISION		LATITUDE: 41.88670°N		LONGITUDE: 87.69144°W	
	DSN/CHK BY: FNF/MFB	UNION PACIFIC RAILROAD Office of Director Structures Design			
	DRAWN/CHK BY: RR /MFB				
	UPRR ENGINEER: DEH/ADS	LOCATION & DESCRIPTION: BRIDGE 0.93 ROCKWELL SUBDIVISION 1 SPAN TPG x 90' REPLACING 1 SPAN TPGOD x 70' (2 TRACKS)			
	SHT NO.: E42 of E43	SHEET TITLE: TPG BEARING DETAILS - TYPE D			

GENERAL NOTES

GENERAL

- All work requirements shown on the design and not otherwise detailed shall be accomplished as specified in Union Pacific Railroad (UPRR) Specifications and the American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering. In the event of conflicts between specifications, the more restrictive shall apply.
- Construction means and methods shall comply with the All Permits Issued (API) package.
- Field verify all dimensions, stations and elevations prior to start of construction.
- Beams shall be supported by blocking within 1'-6" of ends during storage and transport. Store beams in level position. Beams shall be stacked no more than 3 high.
- Contact the Union Pacific "Call Before You Dig" number 90 days (not less than 60 days) prior to proposed construction start date. Prior to construction, confirm that all necessary relocations have been completed. The CBYD number is: 1-800-336-9193.
- Location of known utilities is approximate. Location shall be verified prior to construction. Notify Call 811 "Call Before you Dig" number at least 48 hours prior to construction.

PILE DRIVING

- All piles shall be driven to capacity shown in design plan set.
- If any numbered pile cannot be driven to these capacities, the UPRR Office of Structures Design shall be notified.
- Estimated capacity of driven piles shall be calculated using the Modified ENR formula, with Factor of Safety of 5. Direct questions to the UPRR Office of Structures Design. Pile driving records and estimated capacities shall be submitted to the UPRR Office of Structures Design.
- Vibratory hammers shall not be permitted to drive any portion of any bearing piles.
- Splice pile per standard drawing Plan No. 531110, Sheet No. HI for H-Piles or Plan No. 531120, Sheet No. I for pipe piles. Pile splices shall be located a minimum of 10' below the proposed or existing ground surface, whichever is lower.
- Mark every pile with a dimension indicating the pile depth from cutoff to point of pile. The dimension shall be rounded to the nearest foot. The mark shall be welded on the outside face, low mile post side on the pile flange, approximately 1'-0" below the bottom of the cap, and in numbers of approximately 3" in height. If a pile is not exposed, no mark is required.
- After pile driving is complete, provide pile driving logs to:

UPRR Senior Manager Structures Design
1400 Douglas St., Stop 0910
Omaha, NE 68179

FIELD WELDING

- Welding shall be accomplished with the SMAW or FCAW Process.
- Welding shall be in compliance with the requirements specified in AWS D1.5, except 3/16" fillet welds may be made with a single pass.
- Welding electrodes shall be E7018 for SMAW or E71T-7 for FCAW. For other acceptable electrodes, refer to AWS D1.5.
- Union Pacific Railroad Employees engaged in welding on structures shall have valid certification through Course ES20, Advanced Welding.
- Contract welders shall possess valid AWS qualifications. Welders shall submit a Procedure Qualification Report (PQR) and Weld Procedure Specification (WPS) for each weld type to be performed. Welders shall be able to present documentation verifying that they have performed the specific weld(s) within the prior six months upon request.

GRADING

- Provide and place all fill and subballast material per UPRR Grading Specifications. Perform grading as required to drain and match existing embankments and upstream and downstream channel flowline.
- Perform grading as required for construction of the new structure and replace areas removed and disturbed in the course of construction to a condition equal to or better than existing.

WELL-COMPACTED FILL

- Well-compacted fill shall be well-graded granular soil free of any organic material, stones larger than 3 inches, frozen lumps, debris or excessive moisture. All compaction shall be determined using ASTM D1556 for field test and ASTM D1557 for moisture and density. Fill shall be compacted to 95% of maximum dry density as defined in ASTM D1557 (Modified Proctor). Fill shall be placed in layers not to exceed 12 inches.

GROUT NOTES

NON-SHRINK GROUT

- Non-shrink grout shall conform to the requirements of ASTM C1107.
- Non-shrink grout shall meet the following strength requirements:

1 day: 3,200 psi
7 days: 6,000 psi

EPOXY GROUT

- Epoxy grout shall consist of a 3-component epoxy resin system.

Two liquid epoxy components.
One inert aggregate filler component.

CONTROLLED LOW-STRENGTH MATERIAL (CLSM)

- Controlled Low-Strength Material is a self-compacting, cementitious fill material with an unconfined compressive strength of 50 to 300 psi. The mixture shall consist of water, Portland cement, fly ash, and sound fine or coarse aggregate or both. The mix design shall allow adequate flowability without segregation of aggregates. Hardening time is of prime importance and CLSM should develop 50 psi in about one hour. The maximum layer thickness for CLSM shall be three feet. Additional layers shall not be placed until the CLSM has lost sufficient moisture to be walked on without indenting more than two inches.

CONSTRUCTOR NOTES (WHEN APPLICABLE)

CONSTRUCTOR DEFINITION

Construction By	Term	Refers To
UPRR	Constructor	Manager Bridge Construction
Contractor	Constructor	Contractor

DIVISION OF RESPONSIBILITY

- RAILROAD (Unless Noted Otherwise by MBC)
 - Remove ties, rail and OTM from existing bridge.
 - Provide and install ballast, ties, rail and OTM for proposed bridge.
 - Provide material as shown in the Bill of Material.
 - Provide and install Private Property/No Trespassing sign and bridge marker signs on right side at each end of bridge.
- CONSTRUCTOR
 - Coordinate all construction activities with the Railroad.
 - Before ordering any material, Constructor shall make a detailed field inspection of the site verifying all pertinent dimensions and elevations. Any variations in dimensions or elevations from those shown on the drawings shall be reported immediately to the UPRR Project Manager.
 - Any modifications to this design shall be approved by the UPRR Office of Structures Design prior to construction.
 - Verify the location, relocation, abandonment, and/or temporary support of all utilities affected by the construction of the structure and embankment and coordinate these activities with the appropriate utility companies, agencies and/or authorities.
 - Apply for and obtain all construction permits necessary to perform the work.
 - Bill of Material and Schedules are provided for information only. Constructor shall be responsible for providing all material, not provided by the Railroad, required to complete the work.
 - Perform all work not performed by the Railroad.
 - Provide the Railroad with a detailed construction plan defining the activity, schedule and procedure for each aspect of the work. Construction shall not begin until the construction plan has been approved by the Railroad.
 - Provide all temporary structures (shoring, bracing and/or falsework) required to support and protect the existing embankments and structures affected by the work. Provide the Railroad with details, design and procedure for all temporary structures. All temporary structures shall be designed, signed and sealed by a professional engineer registered in the State that the structure is to be constructed. All temporary structures shall be approved by the UPRR Office of Structures Design prior to beginning construction.
 - Provide temporary guardrail system as directed by UPRR Project Manager. Guardrails on shoring shall include but not be limited to the following:

The top edge height of the top rail shall be 42" +/- 3" above the walking/working surface.

At least one midrail shall be provided, evenly spaced between walking/working surface and top rail.

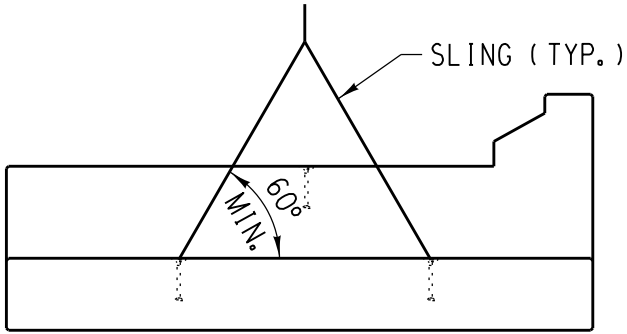
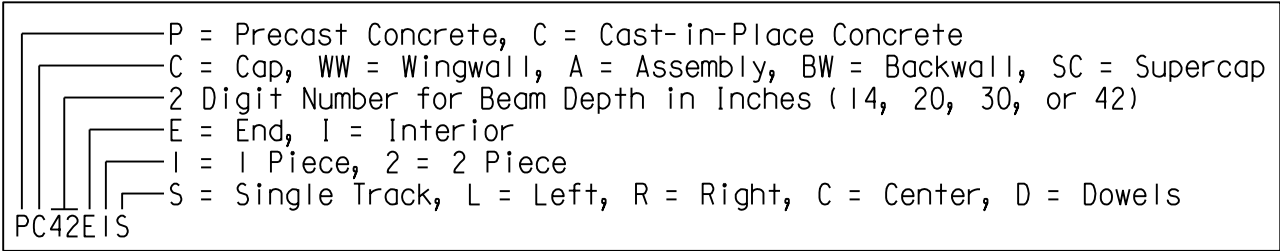
Metal or timber posts or uprights shall be spaced at maximum intervals of 10'-0".

Entire guardrail system, including anchorages, shall be capable of withstanding without failure, a force of 200 lbs. applied in any outward or downward direction at any point.

Guardrail system shall be surfaced to prevent injuries from punctures and lacerations and prevent snagging of clothing. The ends of top rails and midrails shall not extend past the posts or uprights.

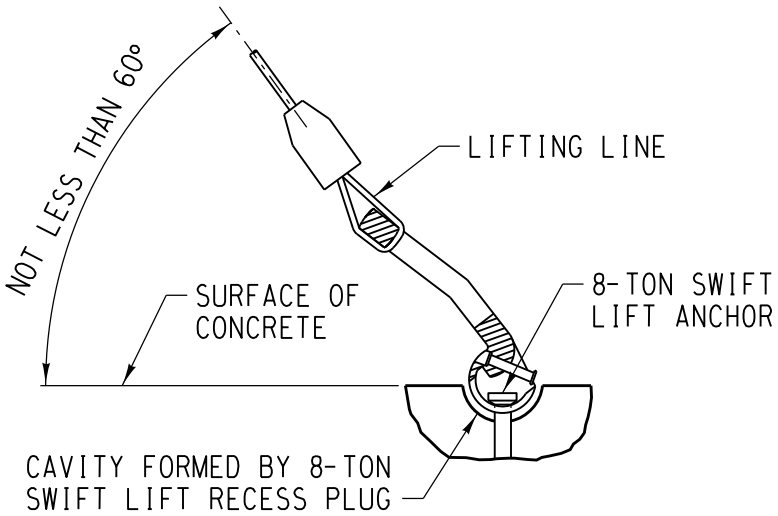
If conditions warrant, i.e. pedestrian traffic/weather, additional protection shall be provided such as screens or mesh to prevent slipping between the midrail and walking/working surface.
 - Direct channel flow as required to perform work.
 - Remove debris and ballst from channel as directed by the Railroad.
 - Accomplish activities within the schedule specified in the approved construction plan.

CONCRETE CAP NAMING CONVENTION KEY



2 POINT PICK DETAIL

SCALE: NONE
SINGLE PIECE END CAP AND LIFTING ANCHORS SHOWN, OTHER CAPS AND BACKWALLS AND LIFTING LOOPS SIMILAR



TYPICAL LIFTING DETAIL

SCALE: NONE
NOTE:
8-TON SWIFT LIFT RECESS PLUGS, ANCHORS AND LIFTING EYES ARE AVAILABLE FROM DAYTON SUPERIOR CORP., 1125 BYERS ROAD., MIAMISBURG, OHIO 45342, TELEPHONE (937) 866-0711. THE MATERIALS FOR THIS LIFTING SYSTEM ARE NOT INCLUDED IN THE BILL OF MATERIAL BUT ARE TO BE ORDERED AS REQUIRED.



BRIDGE STANDARDS

CONCRETE BEAM BRIDGES

GENERAL NOTES

FILE OWNER: UPRR DATE: MAY, 2020

PLAN NO.: 531100 SHEET: T2

Alan Baker

05-04-2020

UPRR - MGR SPECIAL PROJECTS STRUCTURES DESIGN

PRECAST CONCRETE NOTES:

CONCRETE:

1. All concrete materials, placement and workmanship shall be in accordance with Part I, Chapter 8 of the current AREMA Manual for Railway Engineering.
2. Minimum compressive strength - 4000 Lb. per square inch at 28 days.
3. Exposed surfaces shall be formed in a manner which will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered $\frac{3}{4}$ " x $\frac{3}{4}$ ". Top surface to have a smooth finish, free of all float or trowel marks.
4. Concrete shall be proportioned such that the water - cement ratio (by weight) does not exceed 0.45. Concrete must contain a minimum of 6/2 sacks of cement per cubic yard of concrete. Flyash replacement may account for up to 25% of the cement.
5. Cement shall be Type I, Type II or Type III Portland Cement in accordance with ASTM C150 specifications.
6. Aggregates shall be graded in accordance with ASTM C33 specifications. Coarse aggregate shall be no. 67. Fine aggregate shall be natural sand.
7. Air content shall be between 5% and 7% (by volume).
8. Admixtures shall not be used without approval by Engineer.
9. Curing shall be accomplished by wet curing or the application of a Type 2 membrane.
10. The fabricator shall stencil the item name, date manufactured, name of manufacturer and actual lifting weight at location shown.
11. Production procedures for the manufacture of precast members shall be in accordance with the AREMA Manual for Railway Engineering and the Precast/Prestressed Concrete Institute's Manual MNL 116 for Quality Control.
12. Dimensional tolerances governing the manufacture of precast members shall conform to Division VI, Section 6.4 of the Precast/Prestressed Concrete Institute's Manual MNL 116 for Quality Control. Tolerance for location of lifting devices shall be $\pm 1/2$ ".
13. The fabricator will be responsible for loading and properly securing all precast concrete members for shipment. All concrete components shall be made available for inspection by the Railroad at the fabricator's plant prior to shipment, at the Railroad's discretion.

REINFORCING STEEL:

1. Reinforcing steel shall be deformed, new billet bars per current ASTM A615 Specifications and to meet grade 60 requirements.
2. Fabrication of reinforcing steel shall be per Chapter 7 of the CRSI Manual of standard practice. Dimensions of bending details are out to out of bar.
3. Reinforcing steel is to be blocked to proper location and securely wired against displacement. Tie wires shall be installed at every other bar intersection so that at least 50% of the intersections are tied. Tack welding of reinforcing is prohibited. Minimum concrete cover on reinforcement not otherwise noted shall meet current AREMA Manual for Railway Engineering requirements.

EMBEDDED STEEL:

1. Steel plate shall conform to ASTM A36 OR A709-Grade 36 Specifications. Studs shall be C1015, C1017 or C1020 cold drawn steel which conform to ASTM A108 Specifications.
2. Deformed bar anchors shall conform to ASTM A706 specifications. Welding of deformed bar anchors shall conform to AWS D1.4. Welding shall be performed by certified welder.
3. Where galvanizing is not indicated, material shall be plain.

LIFTING ANCHORS:

1. Swift lift anchors shall be Dayton Superior P-52 anchors or approved alternate with a minimum safe working load sufficient for the weight of the precast element including form removal. The safe working load shall provide a minimum safety factor of 4.

MISCELLANEOUS HARDWARE:

1. 8" T-Bar Anchors as manufactured by Meadow Burke Company, or approved alternate.

BEARING PAD SPECIFICATIONS:

- | | |
|---|------------|
| 1. Bearing pads shall be Random Oriented Fiber elastomeric material comprised of high-quality ozone-resistant virgin elastomer and synthetic fibers. Pads shall conform to the following minimum material properties: | |
| Hardness (Shore A, ASTM D2240) | 80 ± 5 |
| Tensile Strength, psi (ASTM D412, Die C) | 1000 ± 100 |
| Ultimate Elongation, minimum % | 40 |
| Heat Aging (ASTM D573, 70 Hrs. @ noted temperature): | |
| Durometer, 212 °F, maximum point change | ± 10 |
| Tensile Strength, 158 °F, maximum % change | ± 25 |
| Ultimate Elongation, 212 °F, maximum % change | ± 25 |
| Compression, minimum ultimate strength, psi | 8000 |
| Apparent Shear Modulus (GA), psi, based on tests conducted at 70 °F to 80 °F under uniform compressive stresses of 500, 1000, and 1500 psi and at applied horizontal shear plus slip strain of 50%. GA is constant in all directions parallel to the bearing plane. | 230 ± 30 |
| 2. Bearing pads shall be Voss Engineering, Inc., "Fiberlast" expansion bearing pads or approved equal. | |
| 3. Cutting of the pads shall be done so that the edges have no tears or other jagged areas. | |
| 4. Permissible tolerances of the pad shall be as stated in Chapter 15, Section 5.12.6 of the 2019 AREMA Manual for Railway Engineering. | |
| 5. The cap fabricator shall fasten the bearing pads to the cap by using the following procedure: clean pads according to manufacturer's recommendations; prime contact surface and glue to cap with Sikadur 31 Hi-Mod Gel (1:1 mix ratio) or approved equal. | |

MISCELLANEOUS STEEL NOTES:

1. Materials, fabrication, workmanship and erection per the current AREMA Manual for Railway Engineering, Chapter 15, Steel Structures.
2. Material shall conform to the following requirements:

Rolled Shapes & Plates	ASTM A36
Pipe	ASTM A53 Gr. B
Bolts	ASTM A307 Gr. A
Elastic Locknut	MLL-DTL-32258
Steel Washer	ASTM F436
3. Grating panels and fasteners shall conform to the following requirements:

Grating Fasteners:	
Saddle Clips	F-10 Galvanized Saddle Clips
Socket Cap Screws	SAE J429, Gr. 8, Zinc Coated
Elastic Locknut	MLL-DTL-32258
Steel Washer	ASTM F436
Nylon Washer	Nylon G16
Grating Panel:	
Material shall comply with NAAMM Standard MBG 531-17	
Size = W-19-4 (1 $\frac{3}{4}$ x $\frac{3}{16}$ Serrated) Steel Galvanized.	
4. Welding requirements:
 - A. All welding shall be with the SAW, SMAW, or FCAW process.
 - B. All welding per AWS D1.1, Structural Welding Code.
 - C. Welders shall possess valid qualifications, including a Procedure Qualification Record (PQR) and Weld Procedure Specification (WPS) for each weld type to be performed as well as documentation verifying that they have performed the specific weld(s) within the prior six months.
5. Miscellaneous steel shall be plain unless noted otherwise.
6. Pieces or assemblies designated as galvanized shall be galvanized after fabrication in accordance with ASTM A123. After galvanizing, all elements shall be free of fins, abrasions, rough or sharp edges, and other surface defects.
7. Bolts and nuts to be zinc plated in accordance with ASTM A153 unless noted otherwise.

CAST- IN-PLACE CONCRETE NOTES:

CONCRETE:

1. All concrete materials, placement, workmanship and testing shall be in accordance with Part I, Chapter 8 of the current AREMA Manual for Railway Engineering.
2. Minimum compressive strength at 28 days shall be as indicated on the design plans.
3. Exposed surfaces shall be formed in a manner which will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered $\frac{3}{4}$ " x $\frac{3}{4}$ ". Top surface to have a smooth finish, free of all float or trowel marks.
4. Concrete shall be proportioned as follows:

Concrete Strength:	3,000 psi	4,000 psi	5,000 psi	6,000 psi
Max. Water/Cement Ratio (by weight):	0.50	0.45	0.42	0.40
Min. Sacks of Cement per Cu. Yd.:	5.5	6.0	6.5	7.0

5. Flyash replacement may account for up to 25% of the cement by substitution.
6. Cement shall be Type I, Type II or Type III Portland Cement in accordance with ASTM C150 specifications.
7. Aggregates shall be graded in accordance with ASTM C33 specifications. Coarse aggregate shall be no. 67. Fine aggregate shall be natural sand.
8. Allowable air content shall be indicated on the design plans based on the following guidelines:
 - Severe Exposure - 5% to 7%
Exposed to wet freeze-thaw, de-icers, or other aggressive agents.
 - Moderate Exposure - 4% to 7%
Exposed to dry freeze-thaw and no de-icers or other aggressive agents.
 - Mild Exposure - 3% to 5%
Not exposed to freezing, de-icers or other aggressive agents.
9. Admixtures shall not be used without approval by Engineer. Where multiple admixtures are used, it is recommended that all admixtures be obtained from the same company.

10. Where exposed to air, curing shall be accomplished by wet curing or membrane curing compound. Membrane curing compound shall conform to ASTM C309, Type 2.
11. Do not use calcium chloride or any admixture containing intentionally added chloride ions. Testing for chloride ions is not required.
12. Apply a structural bonding agent to construction joints or when placing new concrete against existing concrete. Submit bonding agent to the Engineer for approval.

[Note 13 applies only when specifically stated in the Bill of Material descriptions.]
13. DCL-S, as manufactured by W.R. Grace, or approved alternate shall be added at a quantity of 5 gallons per cubic yard. Calcium nitrite solution shall contain 30% solids and shall provide 15.0 lbs. per cubic yard chloride protection. Mix shall also include 7%, by weight of cement, force 10,000 microsilica slurry by W.R. Grace or approved addendum shall be used. Adjust weight of concrete mix water for weight of DCL-S used.

REINFORCING STEEL:

1. Reinforcing steel shall be deformed, new billet bars per current ASTM A615 Specifications and to meet grade 60 requirements.
2. Fabrication of reinforcing steel shall be per Chapter 7 of the CRSI Manual of standard practice. Dimensions of bending details are out to out of bar.
3. Reinforcing steel is to be blocked to proper location and securely wired against displacement. Tie wires shall be installed at every bar intersection so that at least 50% of the intersections are tied. Tack welding of reinforcing is prohibited. Minimum concrete cover over reinforcement not otherwise noted shall meet current AREMA Manual for Railway Engineering requirements.

[Note 4 applies only when specifically stated in the Bill of Material descriptions.]
4. Reinforcing steel shall be epoxy coated per ASTM A775 specifications meeting Annex A1 for epoxy coating.

EMBEDDED STEEL:

1. Steel plate shall conform to ASTM A36 OR A709-Grade 36 Specifications. Studs shall be C1015, C1017 or C1020 cold drawn steel which conform to ASTM A108 Specifications.
2. Deformed bar anchors shall conform to ASTM A706 specifications. Welding of deformed bar anchors shall conform to AWS D1.4. Welding shall be performed by certified welder.
3. Where galvanizing is not indicated, material shall be plain.

REVISIONS			DESIGN BY: HDR	DRAWN BY: HDR	CHECKED BY: AJB
DATE	LTR.	DESCRIPTION	APPROVED:		
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05-04-2020

UPRR - MGR SPECIAL PROJECTS STRUCTURES DESIGN

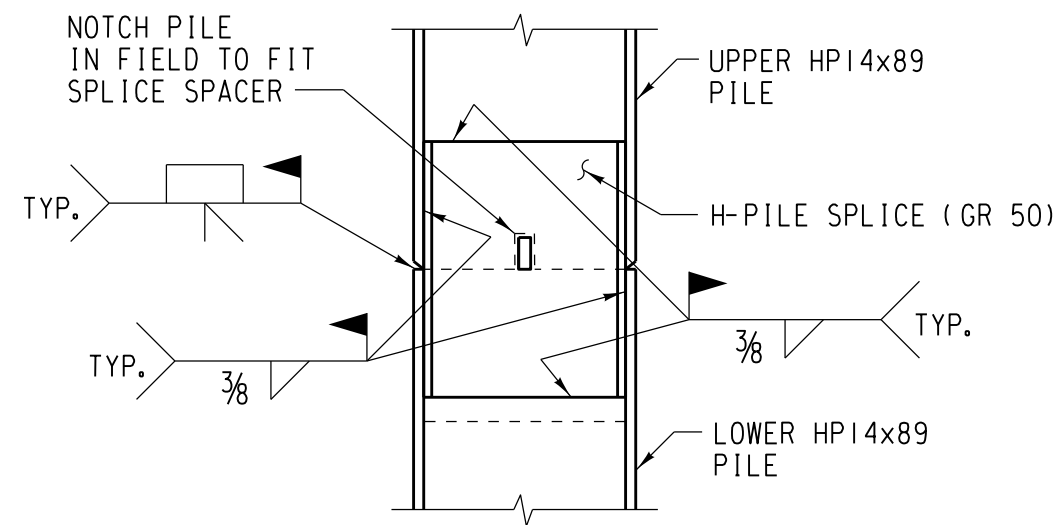


BRIDGE STANDARDS

CONCRETE BEAM BRIDGES

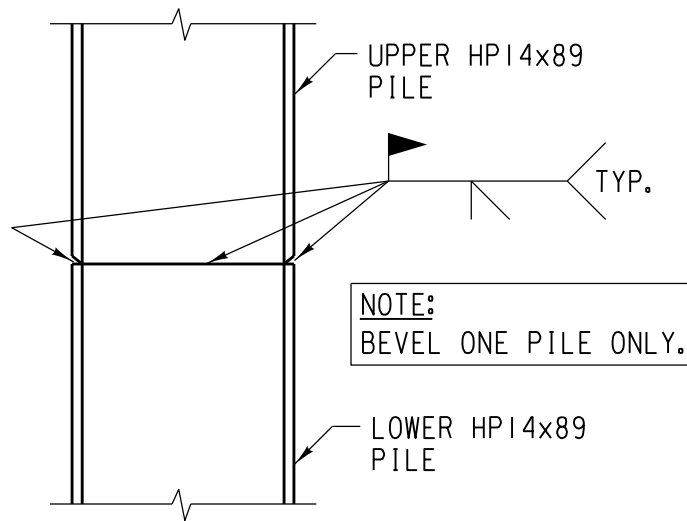
PIECE FABRICATION NOTES

FILE OWNER: UPRR	DATE: MAY, 2020
PLAN NO.: 531100	SHEET: T3

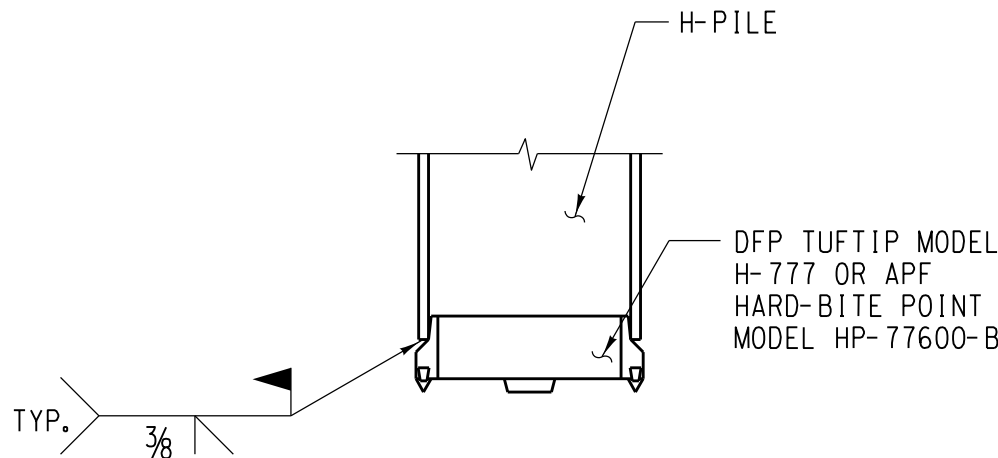


- INSTALLATION INSTRUCTIONS:**
(NOTE: SPLICE IS SHOWN INSTALLED ON UPPER PILE SECTION FIRST. ALTERNATIVELY, SPLICE MAY BE INSTALLED ON BOTTOM SECTION FIRST.)
1. NOTCH THE END OF H-PILE SECTION TO RECEIVE SPLICE FIRST (NOTCH TO ACCOMMODATE THE SPLICE SPACER BAR).
 2. FIT SPLICE OVER NOTCHED END OF H-PILE AND FILLET WELD SPLICE END TO PILE WEB AS SHOWN.
 3. PLACE THE UPPER H-PILE SECTION INTO POSITION ONTO THE LOWER SECTION.
 4. COMPLETE FILLET WELD ALONG SPLICE EDGES.
 5. WELD FLANGE JOINT BETWEEN UPPER AND LOWER PILE SECTIONS.

PILE SPLICE DETAIL
SCALE: 1"=1'-0"

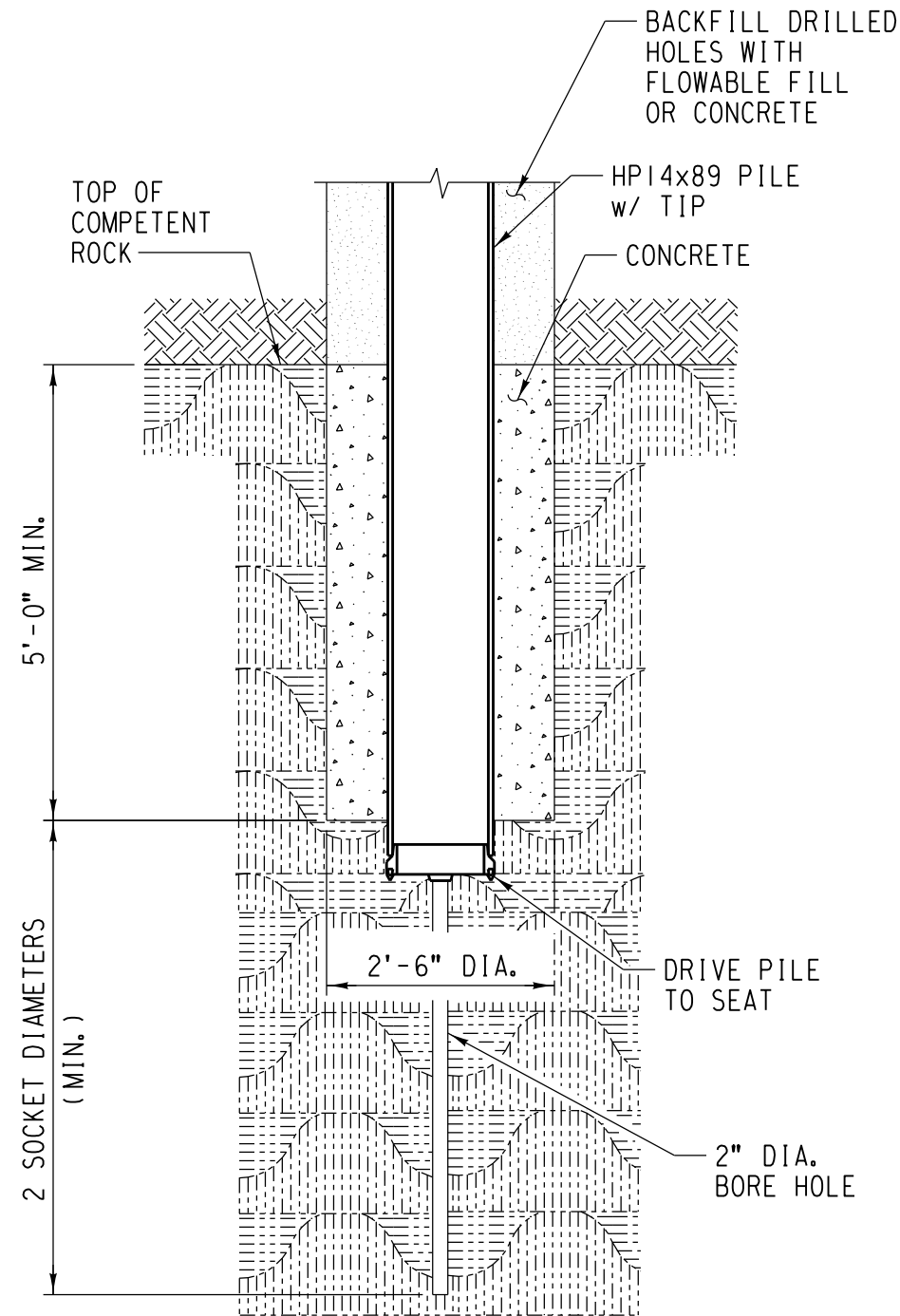


ALTERNATE PILE SPLICE DETAIL
SCALE: 1"=1'-0"



- TIP REINFORCEMENT INSTALLATION INSTRUCTIONS:**
1. FIT POINT ONTO SQUARE CUT PILE.
 2. WELD POINT TO THE PILE IN EITHER FLAT OR VERTICAL POSITION.
 3. FILL THE AREA ACROSS BOTH FLANGES WITH WELD.

PILE TIP DETAIL
SCALE: 1"=1'-0"



PILE SOCKET DETAIL
SCALE: 1/2"=1'-0"
EST. VOLUME OF CONCRETE = 0.9 CU. YD. PER 5' SOCKET DEPTH

- PILE SOCKET NOTES:**
1. SEE BENT DESIGN NOTES TO DETERMINE WHEN PILE SOCKETING IS REQUIRED.
 2. PILING SHALL BE SEATED IN PREDRILLED HOLES IN THE ROCK AND ENCASED IN CONCRETE WITHIN THE BEDROCK (SEE DETAIL).
 3. MINIMUM DEPTH OF PILE SOCKET SHALL BE 5'-0" INTO COMPETENT ROCK.
 4. PILING WITH TIP REINFORCEMENT SHALL BE PLACED INTO ROCK SOCKET AND DRIVEN TO ACHIEVE REQUIRED CAPACITY.
 5. SEATED PILE SHALL BE ENCASED IN ROCK SOCKET WITH CONCRETE.
 6. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE SHALL BE 4000 LB. PER SQUARE INCH AT 28 DAYS.
 7. BORE 2" DIA. HOLE IN CENTER OF PILE SEAT TO A MINIMUM DEPTH OF 2 SOCKET DIAMETERS.

GENERAL NOTES:

Longitudinal bracing between bents required in selected bays on bridges longer than 200 ft and some bridges not composed entirely of concrete spans. See Sheets H5 & H6.

Pile capacities:
For required driven pile capacities, see "Maximum Pile Load" table.

Steel:
Piles - ASTM A572, GR. 50
Pile splices - ASTM A572 Gr. 50
Bracing - ASTM A572

Coating Notes:
Where specified, coal tar coating shall be applied to the external face of piles in accordance with the following guidelines:

Surface Preparation SSPC-SP6/NACE 3
Coating 2 Coats @ 8 Mils For A Total 16 Mil DFT
Masking 3" Mask On Each End
Material Specification Corp of Engineers Formula C-200a & SSPC Paint 16

Welding:
Use shielded metal arc welding (SMAW) or flux core arc welding (FCAW) process per AWS D1.5. Acceptable welding electrodes shall be E7018 for SMAW or E71T-7 for FCAW. For other acceptable electrodes, refer to AWS D1.5.

Splices:
Splices shall be made a sufficient distance above the ground or water (not less than one foot) so that the splice can be observed during driving. The number of splices shall be kept to a minimum. Splicing cut-offs or short pieces to make a main bearing pile is not permitted. The pile shall be driven so that the upper splice is at least 10 feet below the ground surface.

Tip reinforcement:
Pile tip reinforcement shall be used where piles may be damaged by driving through heavy gravel, cobbles, boulders or formations known to contain obstructions. Tip reinforcement shall also be used when the pile will bear on rock.

Driving tolerances:
Variations greater than 1/4 inch per foot from vertical or batter line shall not be allowed. The deviation of the top of piles in a bent shall not exceed one inch from the plan location. Rotation of pile about its centerline shall not exceed 5 degrees from its orientation as shown in plans. Piles not meeting tolerance requirements or out of line as to impair usefulness, or piles that are damaged in driving as to impair structural capacity, shall be pulled and redriven or an additional pile shall be driven to provide added support. If any numbered pile cannot be driven to their capacity, notify the UPRR Office of Structures Design.

BENT DESIGN NOTES:

Selection of pile configuration and maximum heights based on equilibrium superelevation (regardless of actual superelevation installed). Refer to UPRR Track Standards Book, Std. Dwg. 0020 latest revision.

Standard design is valid for minimum pile penetration of 10' if geotechnical investigation demonstrates that piles can be firmly seated in hard rock or shale; otherwise minimum pile penetration of 25' required. If these values cannot be achieved, piles shall be socketed into rock per detail on this sheet. For any other conditions, special design is required.

For span lengths less than 28', use values shown for 28' spans.

Bent shall be driven per requirements of longer span length supported. For example, for a bent supporting 28' and 34' spans, drive per 34' span requirements.

Maximum track offset (from bridge centerline) is 6" with this bent standard.



BRIDGE STANDARDS

CONCRETE BEAM BRIDGES

H-PILE FOUNDATIONS PILE INSTALLATION NOTES AND DETAILS

FILE OWNER: UPRR DATE: MAY, 2020

PLAN NO.: 531110 SHEET: H1

REVISIONS			DESIGN BY: HDR	DRAWN BY: HDR	CHECKED BY: AJB
DATE	LTR.	DESCRIPTION	APPROVED:		
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